



CHABOT-LAS POSITAS COMMUNITY COLLEGE DISTRICT PURCHASING DEPARTMENT

August 12, 2026

Addendum No. 01 INVITATION TO BID.: B26/27-01 Pathways Support Center at Las Positas College

To: All Prospective Bidders

This Addendum One (01) is issued to incorporate the following changes, additions or deletions to the IFB Bid No.: B26/27-01 Pathways Support Center at Las Positas College. Any modifications/changes made by this addendum affect only the portions or paragraphs specifically identified herein; all remaining portions of the IFB Bid No.: B26/27-01 Pathways Support Center at Las Positas College to remain in force. It is the responsibility of all responders to conform to this addendum.

A. ADDITIONS, CHANGES AND/OR CLARIFICATIONS:

Closeout requires a notarized Asbestos and Other Hazardous Materials Certification.

1. Deletion of Certification Requirement. Special Conditions Item 12, "Asbestos and Other Hazardous Materials Certification," and Attachment B to the Special Conditions are deleted in their entirety. The entry "Asbestos and Other Hazardous Materials" is deleted from the list of Contract Documents at Article 5 of the Agreement, and the entry "Attachment B – Asbestos and Other Hazardous Materials Certification" is deleted from the Bidding Requirements Table of Contents. The notarized Asbestos and Other Hazardous Materials Certification is no longer required and is no longer a condition for the District's disbursement of Final Payment.

This deletion is a modification of the Contract Documents made by the District pursuant to Instructions for Bidders Item 10, is effective upon issuance of this Addendum, and results in no adjustment to the Contract Price or the Contract Time. Bidders shall acknowledge this Addendum in their Bid Proposals as required by Instructions for Bidders Item 10.



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2. Obligations Not Affected. The foregoing deletion identified as item 1 does **not** modify, waive or relieve the Contractor of any obligation under General Conditions Article 4.9.7 (Hazardous Materials), including Articles 4.9.7.1, 4.9.7.2 (Prohibition on Use of Asbestos Construction Building Materials) and 4.9.7.3 (Disposal of Hazardous Materials), or under General Conditions Article 4.17 (Encountering of Hazardous Materials), each of which remains in full force and effect.
3. Statement of Bidders Qualifications item 3.4 is hereby replaced with the following: 3.4 As a mandatory minimum qualification, each Bidder shall have completed, as prime contractor, at least two (2) California community college campus construction projects of similar size, scope and complexity to this Project within the five (5) years preceding the Bid Deadline. On a separate attachment, Bidder shall identify each such project, stating the college name, project name, location, contract value, scope of work performed, date of completion, and current contact information including telephone and email for the project owner's representative.

Failure to identify at least two (2) qualifying projects meeting the foregoing requirement, or failure to provide the information required for each, will render the Bid Proposal non-responsive and subject to rejection. This requirement is in addition to, and does not limit, the District's evaluation of Bidder responsibility under Instructions for Bidders, Item 12.6.

B. REQUEST FOR INFORMATION (RFI) QUESTIONS AND RESPONSES:

QUESTION 1: We are interested in bidding on Bid No.: B26/27-01, Pathways Support Center at Las Positas Project and would like to kindly request the Engineer's Estimated Project Cost for our reference

RESPONSE 1: \$900,000.00

QUESTION 2: Does the project fall under PLA or CWTA or MTPA (Master Trade Partner Agreement)?

RESPONSE 2: No.



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QUESTION 3: Is pre-qualification required?

RESPONSE 3: Please refer to section "A", item number 3 regarding the Statement of Bidder Qualifications on this addenda.

QUESTION 4: What is the estimated cost of the project?

RESPONSE 4: See question 1 response

QUESTION 5: A-641 Interior Finish Legend Wall Covering WC1 Custom Art Provided by Architect. Please confirm that the material is supplied by the architect and installed by the contractor.

RESPONSE 5: Refer to spec section 10 26 00. Custom digital images provided by architect, material provided and installed by contractor.

QUESTION 6: S-111 Foundation Plan. Legend shows concrete curb and footing. This appear so be existing. Please confirm no concrete work is required.

RESPONSE 6: No new concrete curbs, footings or trenching.

QUESTION 7: General Conditions Section 6.3 requires that "The Contractor shall obtain Builders Risk insurance covering the full insurable value of the Work from risks of loss, damage or destruction of Work in progress or in place at the Site prior to Final Acceptance including without limitation coverage for losses resulting from the perils of fire, malicious mischief, vandalism, and collapse." The provision governing payment for stored materials instead refers to "the policy of Builder's Risk insurance obtained by the District." Please confirm whether Builder's Risk is to be obtained by the Contractor or by the District, and confirm whether theft of materials or equipment stored at the Site but not yet incorporated into the Work is a covered peril, given that theft is not among the perils listed in Section 6.3.

RESPONSE 7: Builder's Risk insurance is to be obtained by the Contractor. See General Conditions Articles 6.3, 6.4.2 and 6.7, and Special Conditions Article 2.1, "Insurance Requirements for



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Contractor," which sets the Builder's Risk coverage amount at the full value of the Work. The references in General Conditions Articles 8.3.7.2 and 8.3.7.3 to a policy "obtained by the District" are hereby corrected by this Addendum to read "obtained by the Contractor." Please also refer to Special Conditions Article 2.1, "Insurance Requirements **for Contractor**," which sets the Builder's Risk coverage amount at the full value of the Work.

QUESTION 8: General Conditions Section 6.4.1 states that the Contractor "shall be solely and exclusively responsible for the payment of the deductible, if any, under such policy of insurance, without adjustment to the Contract Price on account thereof." Division 1 states that the "Construction Laydown Area will be the contractor's responsibility," and that "Materials, tools, accessories, etc., shall be stored only where directed by CLPCCD" with "Security of stored items shall be Contractor's responsibility." Please identify the laydown and storage location the District will direct on this campus, and confirm the District's expectation for securing stored items outside working hours on an occupied campus.

RESPONSE 8: The quoted laydown provision appears at Special Conditions Item 8, which provides that the Construction Laydown Area is the Contractor's responsibility and shall be confined to within the project boundaries and the area identified and attached here as exhibit #01 which also allows for placement of a single debris bin not to exceed 4 cubic yards. Specific storage locations within the project boundaries will be established at the Pre-Construction Conference per General Conditions Article 4.3.6.1 and Specification Section 01 60 00, 1.3.B. Where the Site does not permit on-site storage, the Contractor shall provide off-site storage and protection per Specification Section 01 60 00, 1.4.C, without adjustment to the Contract Price. As discussed during the pre-bid walk, materials may be stored within the scope area. **The District does not prescribe the means or methods by which the Contractor satisfies its security obligation.**

QUESTION 9: Section 01 23 00 provides for alternates as "An amount proposed by Contractor and stated in its Bid Proposal for certain work defined in the Instruction to Bidders and Bid Proposal Form that may be added to or deducted from the Base Bid



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amount." Please confirm whether any alternates will be issued for this project, and if so, whether the District would consider an alternate for construction site security addressing the Contractor's responsibility for security of stored items.

RESPONSE 9: No Alternates are identified in this project. **The District will not consider contractors site and security obligation as an alternate. Site and security are the contractors responsibility.**

QUESTION 10: Kindly provide the estimated project cost.

Response 10: See question 1 response.

QUESTION 11: Please confirm retention will be withheld at 5%.

RESPONSE 11: Confirmed, retention will be held at 5%.

QUESTION 12: Please confirm there are no SBE/DBE/DVBE participation goals or good-faith-effort documentation requirements for this bid.

RESPONSE 12: Confirmed, there are no SBE/DBE/DVBE participation goals or good-faith-effort documentation requirements for this bid.

QUESTION 13: Please confirm permitted working hours, any quiet-hour or instruction-period noise restrictions, and whether night/weekend work is required or permitted for disruptive activities (demolition, coring, fire alarm testing).

RESPONSE 13: Noise control is always favorable. The City of Livermore work hours are limited to: 7am to 7pm Monday – Friday, Saturday work 9am – 5pm and No Sunday work. Work on campus must fall within the city limitations and must also adhere to the scheduling provisions set forth in the contract documents, including but not limited to General Conditions Article 4.20 which limits construction activities to the District's hours and days set forth in the Special Conditions, except in an emergency. This window identifies the outer limit of allowable working hours - it is not a directive or permission to perform Work throughout the full window,



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and any Work outside normal business hours remains subject to District authorization per GC 4.20 and GC 4.18.5.3.

QUESTION 14: Please identify the exact laydown/staging limits, dumpster and debris-box locations, and the approved construction path of travel and delivery route to Building 1600.

RESPONSE 14: Refer to the response for question 8 above. In regard to the debris-box location see exhibit #01 attached.

QUESTION 15: Is a hazardous materials survey (asbestos/lead) available for Building 1600? Closeout requires a notarized Asbestos and Other Hazardous Materials Certification. Please provide any existing survey reports or confirm the building is free of ACM/LBP in the work area.

RESPONSE 15: Please see section A for response to question.

QUESTION 16: Please confirm the condition of existing chilled beams to be relocated, and which party bears the risk of damage or non-performance of relocated/reused equipment after relocation.

RESPONSE 16: Existing chilled beam conditions shall be verified in the field prior to commencing the Work. Per Specification Section 01 73 29, the Contractor is responsible for examining existing conditions, including elements subject to damage or movement during the Work, and assessing conditions affecting performance after existing work is uncovered. Beginning the Work constitutes acceptance of existing conditions.

The Contractor shall protect existing equipment from damage during relocation and restore assemblies to like-new condition.

QUESTION 17: Please confirm available shutdown windows for the campus hydronic (chilled/heating water) systems for chilled-beam piping tie-ins, and any after-hours requirements for these shutdowns

RESPONSE 17: The selected contractor will coordinate this work with and through the Facilities team, the Maintenance & Operations team and the College.



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QUESTION 18: Please confirm the extent of relocation and whether draining, refilling, and rebalancing of the affected hydronic loop are included in this scope.

RESPONSE 18: Extent of relocation and associated hydronic scope of work is shown on M-111. Existing and new piping is shown diagrammatically to establish design intent. As noted on this sheet, rebalancing will be required on all affected loops. Where it is not possible to isolate piping at the points of connection, draining and refilling will be required to perform the scope of hydronic work.

QUESTION 19: Please confirm the list of OFCI and/or CFCI items.

RESPONSE 19: Please refer to the project drawings and specifications.

QUESTION 20: Please identify the location, capacity, and voltage of the District-provided temporary electrical point of connection, and confirm whether temporary water and restroom facilities are available or contractor-provided.

RESPONSE 20: Contractor is to provide portable restroom facilities. Power must be maintained except by breaker to isolate the scope areas as the building will be in use by staff and students for the duration of the work. Temporary power should be available from one of the adjacent service closets that is not an IDF or MDF.

QUESTION 21: Are record/as-built drawings of Building 1600 available to bidders?

RESPONSE 21: Please see exhibit #02 attached which contains the portion of the as-built drawings we believe are needed to perform the scope of this project. A complete set will be available to the awarded contractor.

QUESTION 22: Please confirm the existing fire alarm system and whether the fire alarm work is a deferred submittal.

RESPONSE 22: There are NO deferred approvals on this project. Fire alarm system is Honeywell. See Specification Section 28 46



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00, Article 2.02.A. Per Article 2.01, fire alarm control units, initiating devices and notification appliances shall match the existing campus system, Basis of Design Honeywell Security & Fire Solutions / Gamewell-FCI, no known equal; the existing Gamewell-FCI E3 fire alarm control unit shall accommodate the new appliances.

QUESTION 23: Sheet G-001 lists deferred submittals as "NONE." Please confirm the vertical stacking overhead folding door and its deck-suspended support framing are fully covered by the DSA-approved documents (Detail ref. S-901) and that no deferred DSA approval is required.

RESPONSE 23: Confirmed.

QUESTION 24: Please confirm the basis-of-design manufacturer for the overhead folding door.

RESPONSE 24: The basis-of-design manufacturer for the overhead folding door is **Renlita Doors North America, LLC**, product "Sovereign" accordion stacking door, per Specification Section 08 36 29, Article 2.1.A.1. CornellCookson, LLC, product "VertiStack Clear," is also listed as an acceptable manufacturer at Article 2.1.A.2. Substitutions shall be submitted under the provisions of Division 01.

All other terms and conditions remain unchanged.

Michael McClung - Buyer, Purchasing and Warehouse Services
Chabot-Las Positas Community College District

Bid 25/26-13 Pathways Support Center EXHIBIT #01



PROJECT DIRECTORY

ARCHITECT OF RECORD

STEINBERG ARCHITECTS
60 PIERCE AVENUE
SAN JOSE, CA 95110
408-295-5446

MECH., ELEC., PLUM. & LEED

TIMMONS DESIGN ENGINEERS
901 MARKET ST, SUITE 480
SAN FRANCISCO, CA 94103
415-957-6788

TELECOM., AUDIO/VISUAL,
SECURITY & ACOUSTICAL
TEECOM DESIGN GROUP
1333 BROADWAY, SUITE 601
OAKLAND, CA 94612
510-337-2800

FOOD SERVICE CONSULTANT

PATRICK STEIN & ASSOCIATES
5669 SNELL ROAD, PMB 235
SAN JOSE, CA 95123
408-360-9881

LANDSCAPE

ROYSTON HANAMOTO ALLEY & ABEY
225 MILLER AVE
MILL VALLEY, CA 94941
415-383-7900

STRUCTURAL

THE CROSBY GROUP
726 MAIN STREET
REDWOOD CITY, CA 94063
650-367-8100

CIVIL

SANDIS
1721 BROADWAY, SUITE 201
OAKLAND, CA 94612
510-873-8866

CODE CONSULTANT

SCHIRMER ENGINEERING
1850 GATEWAY BLVD., SUITE 1030
CONCORD, CA 94520
925-827-5858

ADMINISTRATIVE REQUIREMENTS

CHAPTER 4 PART 1, TITLE 24, C.C.R., ADMINISTRATIVE REQUIREMENTS (PARTIAL LISTING ONLY)

1. A COPY OF PARTS 1 AND 2, TITLE 24, C.C.R. SHALL BE KEPT ON THE JOB SITE AT ALL TIMES.
2. ALL CHANGE ORDERS AND ADDENDA TO BE SIGNED BY THE ARCHITECT AND THE OWNER AND APPROVED BY DSA. CHANGE ORDERS ARE NOT VALID UNTIL APPROVED BY DSA PER SECTION 4-335, PART 1, TITLE 24.
3. ALL TESTS TO CONFORM TO THE REQUIREMENTS OF SECTION 4-335, PART 1, TITLE 24, AND APPROVED T & I SHEET.
4. TESTS OF MATERIALS AND TESTING LABORATORY SHALL BE IN ACCORDANCE WITH SECTION 4-335 OF PART 1, TITLE 24 AND THE DISTRICT SHALL EMPLOY AND PAY THE LABORATORY. COSTS OF RE-TEST MAY BE BACK CHARGED TO THE CONTRACTOR.
5. DSA SHALL BE NOTIFIED AT THE START OF CONSTRUCTION AND PRIOR TO THE PLACEMENT OF CONCRETE PER SECTION 4-331, PART 1, TITLE 24.
6. INSPECTOR SHALL BE APPROVED BY DSA. INSPECTION SHALL BE IN ACCORDANCE WITH SECTION 4-333 (B). THE DUTY OF THE INSPECTOR SHALL BE IN ACCORDANCE WITH SECTION 4-342, PART 1, TITLE 24.
7. SUPERVISION OF CONSTRUCTION BY DSA SHALL BE IN ACCORDANCE WITH THE SECTION 4-334, PART 1, TITLE 24.
8. CONTRACTOR, INSPECTOR, ARCHITECT, AND ENGINEERS SHALL SUBMIT VERIFIED REPORTS (FORM SSS-6) IN ACCORDANCE WITH SECTION 4-336, AND 4-343, PART 1, TITLE 24.
10. THE CONTRACTOR SHALL PERFORM HIS DUTIES IN ACCORDANCE WITH SECTION 4-343, PART 1, TITLE 24.

APPLICABLE CODES AND SUMMARIES

THE INTENT OF THE DRAWINGS AND SPECIFICATIONS IS TO CONSTRUCT REFERENCED PROJECT IN ACCORDANCE WITH CALIFORNIA CODE OF REGULATIONS TITLE 24. SHOULD ANY CONDITION DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH SAID CALIFORNIA CODE OF REGULATIONS TITLE 24, A CHANGE ORDER DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY THE AGENCY HAVING JURISDICTION BEFORE PROCEEDING WITH THE WORK.

CALIFORNIA CODE OF REGULATIONS (CCR)
2010 CALIFORNIA BUILDING CODE - PART 2, TITLE 24, CCR
2010 CALIFORNIA ELECTRICAL CODE - PART 3, TITLE 24, CCR
2010 CALIFORNIA MECHANICAL CODE - PART 4, TITLE 24, CCR
2010 CALIFORNIA PLUMBING CODE - PART 5, TITLE 24, CCR
2010 CALIFORNIA ENERGY CODE - PART 6, TITLE 24, CCR
2010 CALIFORNIA ELEVATOR SAFETY CONSTRUCTION CODE, PART 7, TITLE 24, CCR
2010 CALIFORNIA FIRE CODE - PART 9, TITLE 24, CCR

NFPA 13, STANDARD FOR THE INSTALLATION OF SPRINKLER SYSTEMS, 2010 EDITION
NFPA 72, NATIONAL FIRE ALARM CODE, 2010 EDITION

2004 ADAAG WITH 2007 SUPPLEMENT

AGENCIES

DIVISION OF STATE ARCHITECT - ACCESS COMPLIANCE ("DSA-AC")
ENFORCING AGENCY: DEPT. OF GENERAL SERVICES ("DGS") [CBC 109.1.4.1]

DIVISION OF STATE ARCHITECT - STRUCTURAL SAFETY ("DSA-SS")
ENFORCING AGENCY: DIVISION OF STATE ARCHITECT-STRUCTURAL SAFETY [CBC 109.2.1]

STATE FIRE MARSHALL ("SFM")
ENFORCING AGENCY: STATE FIRE MARSHALL OR THE CITY/COUNTY FIRE DEPARTMENT [CBC111.2]

CURRENT EDITION - ILLUMINATING ENGINEERING SOCIETY LIGHTING HANDBOOK

* At the initial Collaborative Process meeting on Dec. 17, 2009, DSA agreed that this project would comply with the 2010 CBC.

PROJECT DESCRIPTION

Las Positas College Student Services and Administration Building (will be B-1600)

The new Building 1600 at Las Positas College is part of an overall modernization program for the entire campus. This Application (#01-111019) is for the new building and associated surrounding site work. Las Positas College is part of the Chabot-Las Positas Community College District. The project site is not within the Urban Wild Life.

Demolition of three existing buildings will be completed prior to the start of this construction. DSA Form 1 and 1.CP are complete, signed and submitted.

Building 1600 is a new multipurpose building for Student Services, Administration offices, Food Service, and general education classrooms. Located in the newly cleared footprint, this building will replace three existing small modular buildings built in the early 1970's and early 1980's. The new B-1600 is a two-story building. There are three primary entrances to the building. One of these is at a mid-point between floors to support access from the high traffic from the central campus boulevard and quad. The building is a slab-on-grade, moment frame structure enclosed by GFRP panels, metal panels, storefront and curtain wall. The building is fully sprinkled and has acoustic criteria due to program use.

There are three major users in the building: Student Services supporting registration, financial transaction and academic success. The dining room will provide accommodations for day-to-day food services and catered events. All classrooms are located on the ground floor. The Las Positas College Administration organizations are located on the second floor of one wing. The building is serviced by the campus central hydronic system for heating and cooling which feeds a range of HVAC distribution systems: Chilled beams, radiant floor and displacement diffuser, as appropriate for various spaces and functions. Power is provided from the campus loop via a new transformer and switch. The campus existing Recycled Fire Water loop services the fire suppression system. There is detention piping for controlled release of the storm water. The building functions are complemented by several exterior courtyards and plazas. New access pathways and concrete flatwork are provided for accessible circulation in and around the building. The existing exterior and interior sidewalks are modified to provide accessible circulation from parking and public transit stops.

PROJECT INFORMATION

PROJECT LOCATION

STUDENT SERVICES AND ADMINISTRATION BUILDING (B-1600)
LAS POSITAS COLLEGE
3000 CAMPUS HILL DR., LIVERMORE, CA 94588
925-485-5208

CODE SUMMARY

OCCUPANCY CLASSIFICATION:

LAS POSITAS COLLEGE - SHALL BE CLASSIFIED
AS MULTI-OCCUPANCY, TYPE II-B, NON-RATED, WITH
SEPARATED OCCUPANCIES (TABLE 508.3.3)

ALLOWABLE BUILDING HEIGHT & AREA:

	ACTUAL	ALLOWABLE
STORIES:	2	≤ 2-4
HEIGHT:	35'	< 55'
BLDG AREA RATIO:	1.17	< 2
		OKAY

SEE G1.41 FOR DETAIL CALCULATION AND CODE INFORMATION

SHEET INDEX

SHEET NO.	SHEET NAME
GENERAL	
GG.00	COVER
GG.01	PROJECT INFORMATION & SHEET INDEX
G1.20	Campus Plan Accessible Path of Travel
G1.30	Campus Plan Fire Access Diagram
G1.41	CODE/LIFE SAFETY
G1.42	BUILDING SEPARATION ANALYSIS
CIVIL	
CO.1	CIVIL NOTES, LEGEND, AND ABBREVIATIONS
CO.2	SURVEY SHEET (FOR REFERENCE ONLY)
CO.3	FIRE LIFE SAFETY PLAN (FOR REF. ONLY)
CI.0	DEMOLITION PLAN
CI.1	HORIZONTAL CONTROL PLAN
CO.20	GRADING AND DRAINAGE PLAN
CI.30	UTILITY PLAN
CI.40	HYDROMODIFICATION PLAN
CI.41	HYDROMODIFICATION DETAILS
CI.50	EROSION CONTROL PLAN
CO.60	CIVIL CONSTRUCTION DETAILS
CO.61	CIVIL CONSTRUCTION DETAILS
CO.62	CIVIL CONSTRUCTION DETAILS
CO.63	CIVIL CONSTRUCTION DETAILS
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S0.02.2	GENERAL NOTES
S1.01.1	TYPICAL CONCRETE DETAILS
S1.01.2	TYPICAL CONCRETE DETAILS
S1.01.3	TYPICAL STEEL DETAILS
S1.01.4	TYPICAL STEEL DETAILS
S1.01.5	TYPICAL STEEL DETAILS
S1.01.6	TYPICAL STEEL DETAILS
S1.01.7	TYPICAL STEEL DETAILS
S1.01.8	TYPICAL STEEL DETAILS
S1.01.9	TYPICAL INTERIOR METAL STUD DETAILS
S1.01.10	TYPICAL INTERIOR METAL STUD DETAILS
S1.01.11	TYPICAL INTERIOR METAL STUD DETAILS
S1.01.12	TYPICAL EXTERIOR METAL STUD DETAILS
S1.01.13	TYPICAL EXTERIOR METAL STUD DETAILS
S1.01.14	TYPICAL GFRP SCHEMATIC DETAILS
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S2.01.2	FOUNDATION PLAN - WEST WING
S2.11.1	LEVEL 2 FRAMING PLAN - EAST WING
S2.11.2	LEVEL 2 FRAMING PLAN - WEST WING
S2.11.3	ROOF LEVEL FRAMING PLAN - EAST WING
S2.11.4	ROOF LEVEL FRAMING PLAN - WEST WING
S2.11.5	LEVEL 1.5 & UPPER ROOF LEVEL FRAMING PLANS
S2.11.6	ENLARGED BEAM PLANS
S3.01.1	FRAMING ELEVATIONS
S3.01.2	FRAMING ELEVATIONS
S4.02.1	WALL SECTIONS
S6.01.1	FOUNDATION DETAILS
S6.01.2	FOUNDATION DETAILS
S6.01.3	FOUNDATION DETAILS
S6.01.4	FOUNDATION DETAILS
S6.01.5	FOUNDATION DETAILS
S6.02.1	FRAMING DETAILS
S6.02.2	FRAMING DETAILS
S6.02.3	FRAMING DETAILS
S6.02.4	FRAMING DETAILS
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A0.01	SYMBOLS, ABBREVIATIONS & GENERAL NOTES
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A2.00.2	OVERALL FLOOR PLAN - LEVEL 2
A2.00.3	OVERALL ROOF PLAN
A2.01.1	LEVEL 1 - EAST WING
A2.01.2	LEVEL 1 - WEST WING
A2.02.1	LEVEL 2 - EAST WING
A2.02.2	LEVEL 2 - WEST WING
A2.11.1	REFLECTED CEILING PLAN - LEVEL 1 - EAST WING
A2.11.2	REFLECTED CEILING PLAN - LEVEL 1 - WEST WING
A2.12.1	REFLECTED CEILING PLAN - LEVEL 2 - EAST WING
A2.12.2	REFLECTED CEILING PLAN - LEVEL 2 - WEST WING
A2.41.1	OVERALL FINISHES AND SIGNAGE PLAN - LEVEL 1
A2.41.2	OVERALL FINISHES AND SIGNAGE PLAN - LEVEL 2
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A3.02	EXTERIOR ELEVATIONS
A3.03	EXTERIOR ELEVATIONS
A3.11	PARTIAL ELEVATIONS
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A3.22	BUILDING SECTIONS
A3.23	BUILDING SECTIONS
A4.01	WALL SECTIONS
A4.02	WALL SECTIONS
A4.03	WALL SECTIONS
A4.04	WALL SECTIONS
A4.05	WALL SECTIONS
A5.01	INTERIOR ELEVATIONS
A5.02	INTERIOR ELEVATIONS
A5.03	INTERIOR ELEVATIONS
A5.04	INTERIOR ELEVATIONS
A5.05	INTERIOR ELEVATIONS
A5.06	INTERIOR ELEVATIONS
A5.07	INTERIOR ELEVATIONS
A5.11	RESTROOM ENLARGED PLANS AND ELEVATIONS
A5.12	RESTROOM ENLARGED PLANS AND ELEVATIONS
A5.13	RESTROOM ENLARGED PLANS AND ELEVATIONS
A5.21	ADA REQUIREMENTS AND ACCESSORIES DETAILS
A6.01	VERTICAL CIRCULATION - STAIRS
A6.02	VERTICAL CIRCULATION - STAIRS
A6.03	VERTICAL CIRCULATION - ELEVATORS AND EXTERIOR RAMP
A6.11	VERTICAL CIRCULATION DETAILS - STAIRS
A6.12	VERTICAL CIRCULATION DETAILS - STAIRS
A6.13	VERTICAL CIRCULATION DETAILS - ELEVATORS
A7.01	SCHEDULE - DOOR
A7.11	SCHEDULE - EXTERIOR WINDOWS
A7.12	SCHEDULE - EXTERIOR WINDOWS
A7.13	SCHEDULE - EXTERIOR WINDOWS
A7.14	SCHEDULE - INTERIOR WINDOWS AND STOREFRONT
A7.15	SCHEDULE - INTERIOR WINDOWS AND STOREFRONT
A7.21	SCHEDULE - FINISHES
A7.22	FLOOR FINISH PATTERNS
A7.31	SCHEDULE - PARTITION AND WALL TYPES
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A8.02	DETAILS - EXTERIOR - ROOF

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A8.05	DETAILS - EXTERIOR - BELOW GRADE DRAINAGE AND WATERPROOFING
A8.11	DETAILS - EXTERIOR - METAL WALL PANELS
A8.12	DETAILS - EXTERIOR - GFRP PANELS
A8.13	DETAILS - EXTERIOR - GFRP PANELS
A8.21	DETAILS - EXTERIOR - WINDOWS - CURTAIN WALLS
A8.22	DETAILS - EXTERIOR - WINDOWS - CURTAIN WALLS
A8.23	DETAILS - EXTERIOR - WINDOWS - CURTAIN WALLS
A8.24	DETAILS - EXTERIOR - WINDOWS - STOREFRONT
A8.25	DETAILS - EXTERIOR - WINDOWS - CURTAIN WALL AND STOREFRONTS
A8.31	DETAILS - EXTERIOR - DOORS
A8.41	DETAILS - INTERIOR - CEILINGS
A8.42	DETAILS - INTERIOR - CEILINGS
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A8.51	DETAILS - INTERIOR - WALLS
A8.52	DETAILS - INTERIOR - WALLS
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A8.54	DETAILS - INTERIOR - WALLS
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A8.91	DETAILS - INTERIOR CASEWORK
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M0.03	MECHANICAL SCHEDULE
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M0.05	T-24 COMPLIANCE FORMS
M0.06	T-24 COMPLIANCE FORMS
M0.07	T-24 COMPLIANCE FORMS
M0.08	T-24 COMPLIANCE FORMS
M0.09	T-24 COMPLIANCE FORMS
M0.10	T-24 COMPLIANCE FORMS
M0.11	T-24 COMPLIANCE FORMS
M0.12	MECHANICAL SCHEDULE
M0.13	MECHANICAL SCHEDULE
M2.01.1	MECHANICAL FLOOR PLAN LEVEL 1 - EAST WING
M2.01.2	MECHANICAL FLOOR PLAN LEVEL 1 - WEST WING
M2.02.1	MECHANICAL FLOOR PLAN LEVEL 2 - EAST WING
M2.02.2	MECHANICAL FLOOR PLAN LEVEL 2 - WEST WING
M2.03.1	MECHANICAL FLOOR PLAN ROOF LEVEL - EAST WING
M2.03.2	MECHANICAL FLOOR PLAN ROOF PLAN - WEST WING
MECHANICAL PIPING	
MP2.01.1	MECHANICAL PIPING FLOOR PLAN - LEVEL 1 - EAST WING
MP2.01.2	MECHANICAL PIPING FLOOR PLAN - LEVEL 1 - WEST WING
MP2.02.1	MECHANICAL PIPING FLOOR PLAN - LEVEL 2 - EAST WING
MP2.02.2	MECHANICAL PIPING FLOOR PLAN - LEVEL 2 - WEST WING
MP2.03.1	MECHANICAL PIPING ROOF PLAN - EAST WING
MECHANICAL	
M4.01	MECHANICAL ENLARGED PLANS
M4.02	EAST ELEV. DETAIL OF ELEVATOR DISPLACEMENT
M4.03	WEST ELEV. DETAIL OF ELEVATOR DISPLACEMENT
M5.01	MECHANICAL DETAILS
M5.02	MECHANICAL DETAILS
M5.03	MECHANICAL DETAILS
M5.04	MECHANICAL DETAILS
M5.05	MECHANICAL DETAILS
M5.06	MECHANICAL DETAILS
M5.07	MECHANICAL DETAILS
M5.08	MECHANICAL DETAILS
M5.09	MECHANICAL DETAILS
M6.01	MECHANICAL PIPING DIAGRAM
M6.02	MECHANICAL PIPING DIAGRAM
M7.01	MECH EMS CONTROL ARCHITECTURE
M7.02	MECH EMS CONTROL ARCHITECTURE
M7.03	MECHANICAL CONTROL DETAILS
M7.04	MECH CONTROL DIAGRAM AHU-1 & AHU-2
M7.05	MECH CONTROL DIAGRAM MAU-1 & KEF-2
M7.06	MECH CONTROL DIAGRAM AHU-3 & KEF-1
PLUMBING	
P0.01	PLUMBING LEGENDS, NOTES & ABBREVIATIONS
P0.02	PLUMBING SCHEDULE
P2.00.1	PLUMBING FOUNDATION PLAN - EAST WING
P2.00.2	PLUMBING FOUNDATION PLAN - WEST WING
P2.01.1	PLUMBING LEVEL 1 - EAST WING
P2.01.2	PLUMBING LEVEL 1 - WEST WING
P2.02.1	PLUMBING LEVEL 2 - EAST WING
P2.02.2	PLUMBING LEVEL 2 - WEST WING
P2.03.0	PLUMBING OVERALL ROOF PLAN
P4.01	PLUMBING / MECH ROOM LEVEL 1 - EAST WING
P4.02	PLUMBING ENLARGED PLANS
P4.02.1	PLUMBING ENLARGED KITCHEN & SERVERY FLOOR PLAN
P5.01	PLUMBING DETAILS
P5.02	PLUMBING DETAILS
P5.03	PLUMBING DETAILS
P5.04	PLUMBING DETAILS
ELECTRICAL	
E0.01	ELECTRICAL LEGEND, NOTES & ABBREVIATIONS
E0.02	ELECTRICAL SCHEDULE
E0.03	ELECTRICAL SCHEDULE
E0.04	ELECTRICAL SCHEDULE
E0.05	ELECTRICAL SCHEDULE
E0.06	ELECTRICAL SCHEDULE
E0.07	ELECTRICAL SCHEDULE
E0.08	ELECTRICAL SCHEDULE
E0.09	T-24 COMPLIANCE FORMS
E1.01	ELECTRICAL SITE POWER PLAN
E1.02	ELECTRICAL SITE LIGHTING PLAN
E2.01.1	POWER FLOOR PLAN LEVEL 1 - EAST WING
E2.01.2	POWER FLOOR PLAN LEVEL 1 - WEST WING
E2.02.1	POWER FLOOR PLAN LEVEL 2 - EAST WING
E2.02.2	POWER FLOOR PLAN LEVEL 2 - WEST WING
E2.03.1	POWER ROOF PLAN - EAST WING
E2.03.2	POWER ROOF PLAN - WEST WING
E3.01.1	LIGHTING PLAN LEVEL 1 - EAST WING
E3.01.2	LIGHTING PLAN LEVEL 1 - WEST WING
E3.02.1	LIGHTING PLAN LEVEL 2 - EAST WING
E3.02.2	LIGHTING PLAN LEVEL 2 - WEST WING
E4.01.1	ENLARGED ELECTRICAL ROOM PLAN
E4.02.1	ENLARGED KITCHEN PLAN
E4.03.1	LOAD CALCULATIONS
E5.01	ELECTRICAL DETAILS
E5.02	ELECTRICAL DETAILS
E6.01	ELECTRICAL SINGLE LINE DIAGRAM

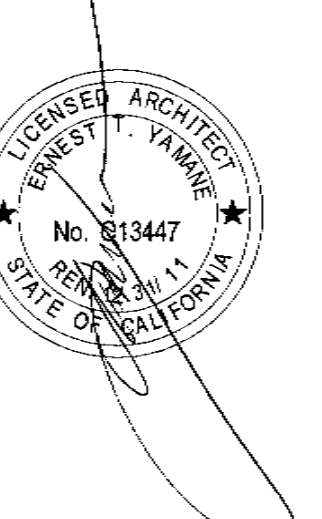
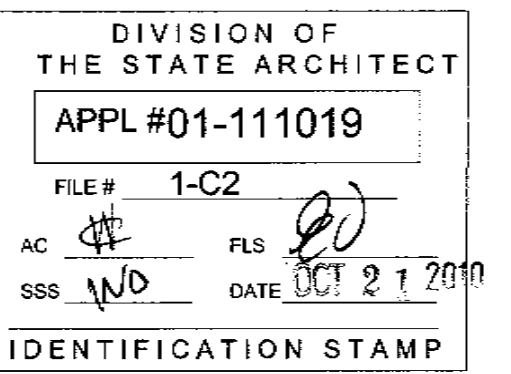
SHEET NO.	SHEET NAME
FIRE ALARM	
FA0.01	FIRE ALARM LEGENDS, NOTES, SYMBOLS AND MOUNTING HT
FA0.02	FIRE ALARM SCHEDULES AND CALCULATIONS
FA0.03	FIRE ALARM SCHEDULES AND CALCULATIONS
FA0.04	FIRE ALARM SCHEDULES AND CALCULATIONS
FA0.05	FIRE ALARM SCHEDULES AND CALCULATIONS
FA0.06	FIRE ALARM SCHEDULES AND CALCULATIONS
FA0.07	FIRE ALARM SCHEDULES AND CALCULATIONS
FA1.01	FIRE ALARM - SITE PLAN
FA2.01.1	FIRE ALARM - LEVEL ONE - EAST WING
FA2.01.2	FIRE ALARM - LEVEL ONE - WEST WING
FA2.02.1	FIRE ALARM - LEVEL TWO - EAST WING
FA2.02.2	FIRE ALARM - LEVEL TWO - WEST WING
FA2.03.1	FIRE ALARM - ROOF PLAN - EAST WING
FA4.01.1	FA ENLARGED FLOOR PLANS
FA6.01A	FIRE ALARM - SINGLE LINE DIAGRAM
FA6.01B	FIRE ALARM - SINGLE LINE DIAGRAM
TECHNOLOGY	
T0.01	SYMBOLS LIST AND DRAWING INDEX
T0.02	SCHEDULE - CABLE AND OUTLET DEVICE
T0.03	SCHEDULE - AUDIOVISUAL BACKBOX
T0.04	SCHEDULE - DOOR AND CAMERA
T0.11	DIAGRAM - BUILDING PATHWAYS
T0.12	DIAGRAM - TELECOM CABLING
T0.13	DIAGRAM - GROUNDING BACKBONE
T0.21	DIAGRAM - AUDIOVISUAL RISER
T0.22	DIAGRAM - AUDIOVISUAL FUNCTIONAL
T0.23	DIAGRAM - AUDIOVISUAL FUNCTIONAL
T0.31	DIAGRAM - SECURITY RISER
T0.32	DIAGRAM - SECURITY ACAMS BLOCK
T0.33	DIAGRAM - SECURITY CCTV BLOCK
T1.01	SITE PLAN - LOW VOLTAGE DEMOLITION
T1.11	SITE PLAN - PATHWAYS
T1.12	SITE PLAN - CABLING
T2.01.1	FLOOR PLAN - LEVEL 1 - EAST WING
T2.01.2	FLOOR PLAN - LEVEL 1 - WEST WING
T2.02.1	FLOOR PLAN - LEVEL 2 - EAST WING
T2.02.2	FLOOR PLAN - LEVEL 2 - WEST WING
T2.11.1	RCP - LEVEL 1 - EAST WING
T2.11.2	RCP - LEVEL 1 - WEST WING
T2.12.1	RCP - LEVEL 2 - EAST WING
T2.12.2	RCP - LEVEL 2 - WEST WING
T2.31.1	SECURITY - LEVEL 1 - EAST WING
T2.31.2	SECURITY - LEVEL 1 - WEST WING
T2.31.3	SECURITY - LEVEL 1.5 - EAST WING
T2.32.1	SECURITY - LEVEL 2 - EAST WING
T2.32.2	SECURITY - LEVEL 2 - WEST WING
T4.11	ROOM PLAN - IDF 1E
T4.12	ROOM PLAN - IDF 1W
T4.13	ROOM PLAN - IDF 2E
T4.14	ROOM PLAN - IDF 2W
T4.31	ROOM PLANS - SECURITY
T5.11	DETAILS - TELECOMMUNICATIONS
T5.12	DETAILS - TELECOMMUNICATIONS
T5.21	DETAILS - AUDIOVISUAL
T5.22	DETAILS - AUDIOVISUAL
T5.31	DETAILS - SECURITY DOOR
T5.32	DETAILS - SECURITY DOOR
T5.33	DETAILS - SECURITY DOOR
T5.34	DETAILS - SECURITY DOOR
T5.35	DETAILS - SECURITY CAMERA
T5.36	DETAILS - SECURITY MISCELLANEOUS
T5.37	DETAILS - SECURITY ADDITIONAL DOORS
LANDSCAPE	
L0.00	COVER SHEET
L1.00	GRADING PLAN
L2.00	LAYOUT PLAN
L2.01	SCORING PLAN
L3.00	CONSTRUCTION PLAN
L3.01	CONSTRUCTION DETAILS
L3.02	CONSTRUCTION DETAILS
L3.03	CONSTRUCTION DETAILS
L3.04	CONSTRUCTION DETAILS
L3.05	CONSTRUCTION DETAILS - ADD/ALT 4
L3.06	CONSTRUCTION DETAILS
L3.07	CONSTRUCTION DETAILS
L4.00	IRRIGATION PLAN
L4.01	IRRIGATION DETAILS
L4.02	IRRIGATION DETAILS
L5.00	PLANTING PLAN
L5.01	PLANTING DETAILS
L5.00	SITE LIGHTING PLAN
FOOD SERVICE	
FS-0	GENERAL NOTES
FS-1.0	EQUIPMENT PLAN
FS-2.0	KITCHEN AND SERVERY ITEMIZED EQUIPMENT
FS-2.1	KITCHEN & SERVERY EQUIPMENT SCHEDULE & LOADS
FS-2.2	KITCHEN & SERVERY BUILDING CONDITIONS
FS-2.3	KITCHEN & SERVERY BUILDING CONDITIONS DETAILS
FS-2.4	KITCHEN & SERVERY PLUMBING POINTS OF CONNECTION
FS-2.5	KITCHEN & SERVERY PLUMBING SCHEDULE & DETAILS
FS-2.6	KITCHEN & SERVERY ELECTRICAL POINTS OF CONNECTION
FS-2.7	ELECTRICAL LOADS & PANEL SCHEDULES
FS-2.8	K

WALL LEGEND

- EXTERIOR ALUMINUM PANEL SYSTEM ON 6" METAL STUD, SEE A8.11
- EXTERIOR GFRG PANEL SYSTEM WITH INTERIOR METAL STUD FLURRINGS, SEE A8.12
- CONCRETE WALL, S.S.D.
- INTERIOR METAL STUD WALL, SEE A7.31
- INTERIOR 1-HR FIRE RATED WALL, SEE 7/A7.31 AND 8/A8.52 AND 1-3/A8.53
- FLOOR DRAIN, S.P.D.

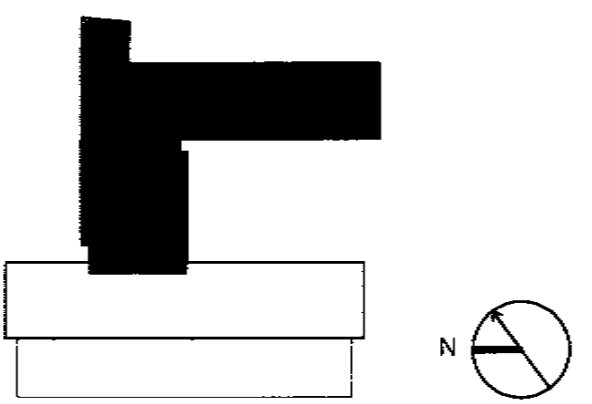
GENERAL NOTES:

- SEE INTERIOR ELEVATIONS FOR EXTENT OF SPECIAL WALL FINISHES.
- SEE INTERIOR ELEVATIONS FOR ACCESSORIES AND EQUIPMENT LOCATIONS.
- ALL ITEMS NEW UNLESS OTHERWISE NOTED.
- FURNITURE SHOWN FOR REFERENCE ONLY.
- ALL DIMENSIONS ARE TO FACE OF FINISH, U.N.O.
- SEE SHEET A5.21 FOR ACCESSIBILITY STANDARDS
- SEE S.S.D. FOR EDGE OF SLAB LOCATIONS AND DIMENSIONS
- LOCATE DOOR OPENING AS SHOWN IN DOOR DIAGRAM A7.01



09	10.14.2010	DSA BACKCHECK
08	07.19.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD - DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% DD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

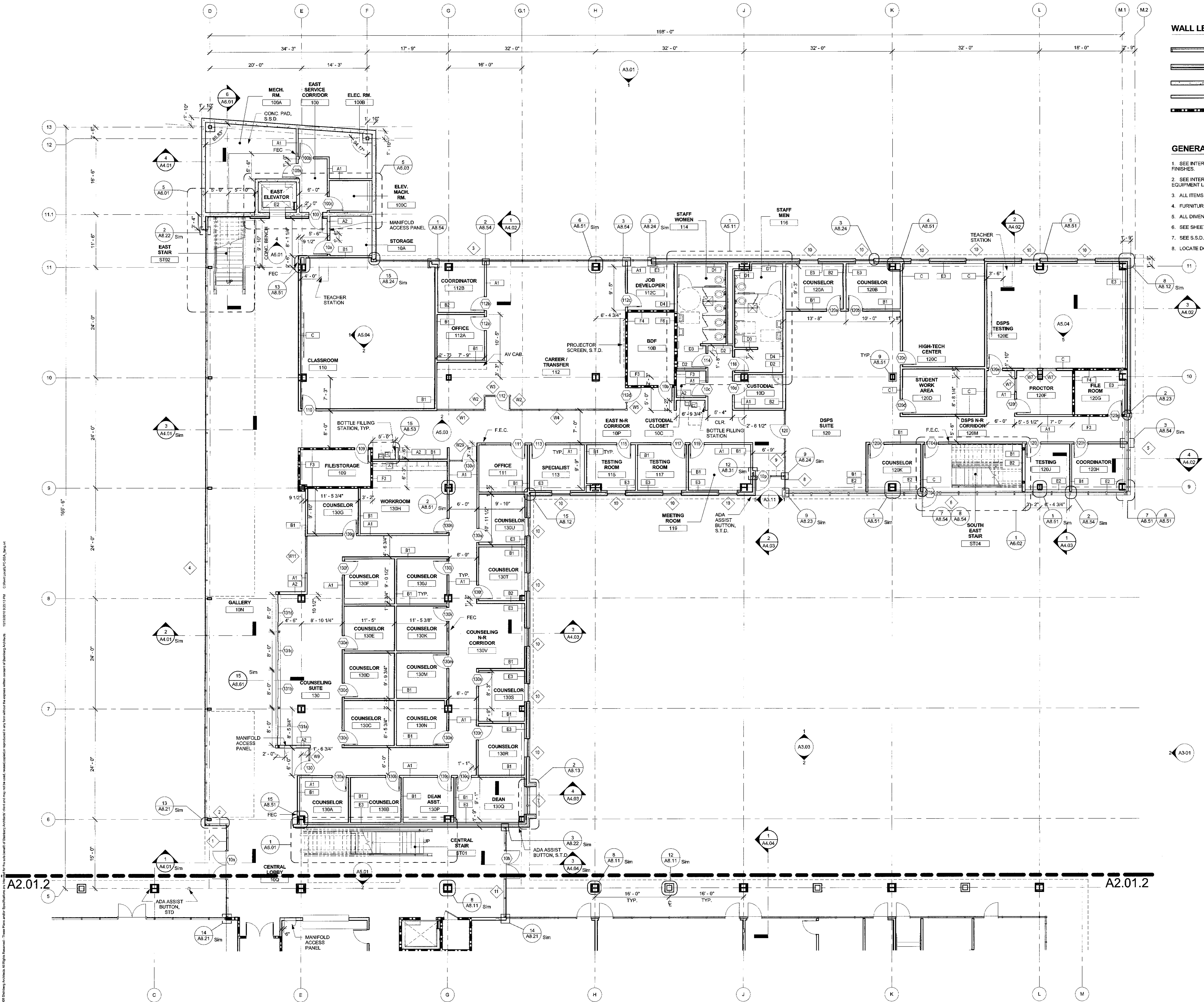
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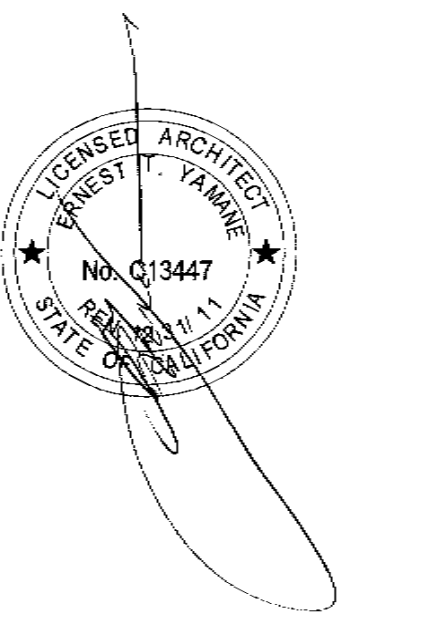


LEVEL 1 - EAST WING

PROJECT #09050.000
DATE 2010, October, 14
SCALE As indicated

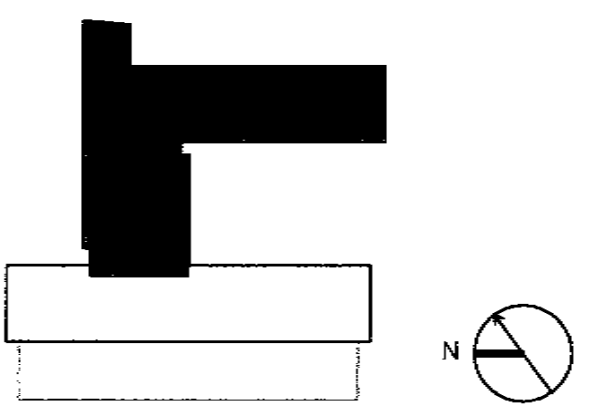
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06	06.03.2010	75% CD - DSA MEETING
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02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

REV NO DATE ISSUE



**Student Services
and Administration
Building**

Las Positas College
3000 Campus Hill Dr.,
Livermore, CA 94551

REFLECTED CEILING PLAN -
LEVEL 1 - EAST WING

PROJECT #09050.000
DATE: 2010 October 14
SCALE: 1/8" = 1'-0"

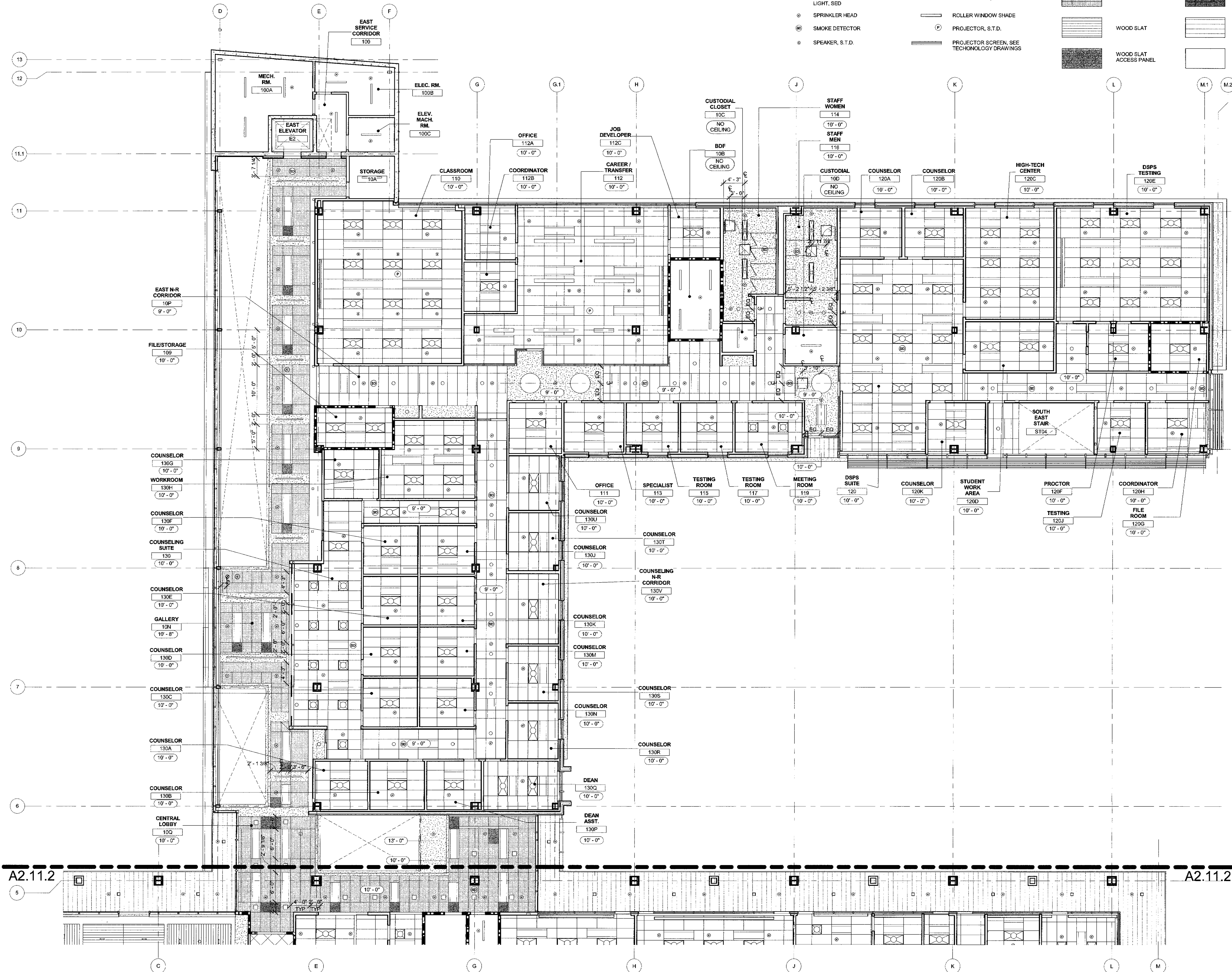
A2.11.1

SYMBOL LEGEND:

	SKYDOME FIXTURE, SED		RETURN AIR REGISTER, SMD
	LINEAR FLOURESCENT LIGHT, SED		EXHAUST AIR REGISTER, SMD
	PENDANT LIGHT, SED		ACCESS HATCH
	EXTERIOR LIGHT FIXTURE, SED		CAMERA
	RECESSED FLOURESCENT LIGHT, SED		CHILLED BEAM, SMD
	SPRINKLER HEAD		ROLLER WINDOW SHADE
	SMOKE DETECTOR		PROJECTOR, S.T.D.
	SPEAKER, S.T.D.		PROJECTOR SCREEN, SEE TECHNOLOGY DRAWINGS

CEILING FINISHES:

	GYPSUM BOARD		2'X2' ACT @ 45 DEGREE ANGLE
	2'X4' ACT TILE		4'X4' MILLENNIUM METAL PANEL
	WOOD PANEL		WOOD PANEL ACCESS PANEL
	WOOD SLAT		METAL SOFFIT
	WOOD SLAT ACCESS PANEL		EXPOSED CEILING



Ceiling Level 1 - East Wing

1

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NUMBERED NOTES:

- 1 NOT USED.
- 2 120V CONNECTION FIRE BELL SHOWN ON ELECTRICAL PLAN. CONTROL MODULE USE TO ACTIVATE THE FIRE ALARM BELL CIRCUIT.
- 3 VISUAL ANNUNCIATION SHALL BE PROVIDED INSIDE ALL ELEVATOR CARS INDICATING THAT THE AUTOMATIC SPRINKLERS, SMOKE DETECTORS OR HEAT DETECTORS IN THE ELEVATOR MACHINE ROOM HAVE ACTIVATED. PROVIDE A RELAY TO TRIGGER A VISUAL LIGHT INDICATING THE ACTIVATION.

SHEET NOTES:

1. THE SINGLE LINE (RISER) DIAGRAM SHALL BE USED AS A REFERENCE GUIDE FOR THE NUMBER OF DEVICES AND APPROXIMATE LOCATIONS. REFER TO THE FLOOR PLANS FOR MORE INFORMATION.
2. THE INSTALLING CONTRACTOR SHALL PROVIDE ALL NECESSARY CONDUIT AND WIRING FOR ALL THE DEVICES SHOWN. MAKE ALL FINAL CONNECTIONS AND PROGRAM THE SYSTEM. THE MAXIMUM WIRE LENGTH (SHOWN ON THE RISER DIAGRAM AND USED FOR VOLTAGE CALCULATIONS) FOR THE NOTIFICATION DEVICES SHALL NOT BE EXCEEDED.
3. ANY ROUTING, IF SHOWN, IS FOR CLARITY ONLY. THE INSTALLING FIRE ALARM CONTRACTOR SHALL DETERMINE THE BEST COURSE OF ROUTING BASED ON THE FIELD CONDITIONS WHILE COMPLYING WITH ALL APPLICABLE GOVERNING CODES.
4. PROVIDE A DEDICATED 20A, 120V CIRCUIT FOR THE FIRE ALARM CONTROL PANEL AND EACH FIRE ALARM EXPANDER PANEL (IF APPLICABLE). NO OTHER DEVICES SHALL BE CONNECTED TO THESE DEDICATED CIRCUITS.
5. CONCEAL ALL FIRE ALARM CONDUITS AND CABLING WHERE APPLICABLE.
6. FIRE ALARM CONTRACTOR SHALL COORDINATE WITH ARCHITECTURAL INTERIOR ELEVATIONS AND SECTION DRAWINGS FOR REFERENCE ON ALL WALL MOUNTED FIRE ALARM DEVICE LOCATIONS. FINAL LOCATIONS SHALL BE CONFIRMED BY THE ARCHITECT AND OWNER PRIOR TO ROUGH-IN. NOTIFY THE ENGINEER OF RECORD ON ANY LOCATION CHANGES REQUIRED.
7. FIRE ALARM CABLES FOR SUPERVISED, POWER-LIMITED CIRCUITS SHALL BE INSTALLED WITHIN METAL CONDUIT SIZED AT A MINIMUM OF 3/4". NO OTHER POWER CIRCUITS SHALL SHARE THIS CONDUIT. REFER TO ARTICLE 760 OF THE NEC FOR MORE INFORMATION.
8. ALL FIRE ALARM WIRING AND CONDUITS SHALL BE DESIGNATED AND MARKED WITH READILY IDENTIFIABLE MARKINGS AND INDICATORS DENOTING FIRE ALARM CIRCUITS.
9. ALL DEVICES MOUNTED BACK TO BACK ON A FIRE RATED WALL SHALL BE MOUNTED WITH A 24" MINIMUM HORIZONTAL OFFSET.
10. LOCATE RELAYS OF ACTIVATION OF FIRE LIFE SAFETY FUNCTIONS WITHIN 3' OF THE CONTROLLED CIRCUITS OR APPLIANCE.
11. MOUNTING OF ALL SMOKE DETECTORS TO BE COORDINATED, PER CEILING CONDITIONS, IN THE FIELD BY THE CONTRACTOR.



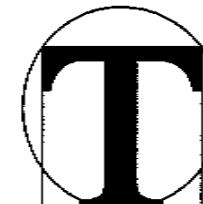
Fire Alarm - Level 1 - East Wing

1

CLAY
Chabot Las Positas Community College District
5100 Franklin Dr.
Pleasanton, CA 94588

ARCHITECT

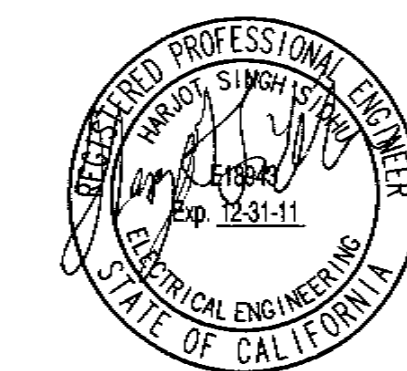
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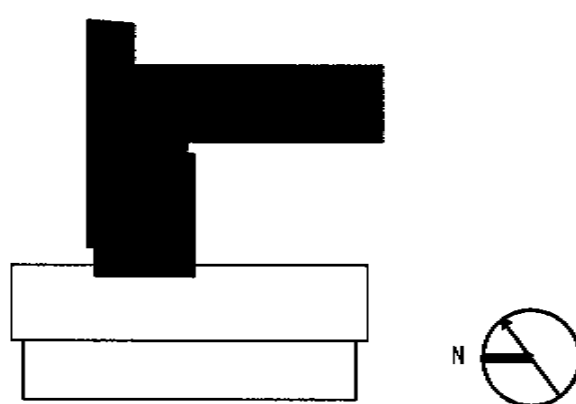
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901 Market Street, Suite 400
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Phone: 415.398.9188 Fax: 415.398.0788
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APPL #	01-111019
REV	1-02
AC	1-02
SS	1-02
IDENTIFICATION STAMP	



09	10/14/2010	DSA BACKCHECK
08	07/19/2010	100% CD SUBMITTAL TO DSA
06	06/03/2010	75% CD-DSA MEETING
05	05/20/2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03/25/2010	100% CD SUBMITTAL TO LPC
03	03/11/2010	80% CD - DSA MEETING
02	12/22/2009	50% SUBMITTAL - DSA
01	11/15/2009	90% SD - DSA MEETING

Student Services and
Administration
Building

Las Positas College

2000 Campus Hill Dr.
Livermore, CA 94551FIRE ALARM
FLOOR PLAN - LEVEL 1 - EAST WING

PROJECT # 09030.001
DATE: 2010 July 19
SCALE: 1/8" = 1'-0"

FA2.01.1

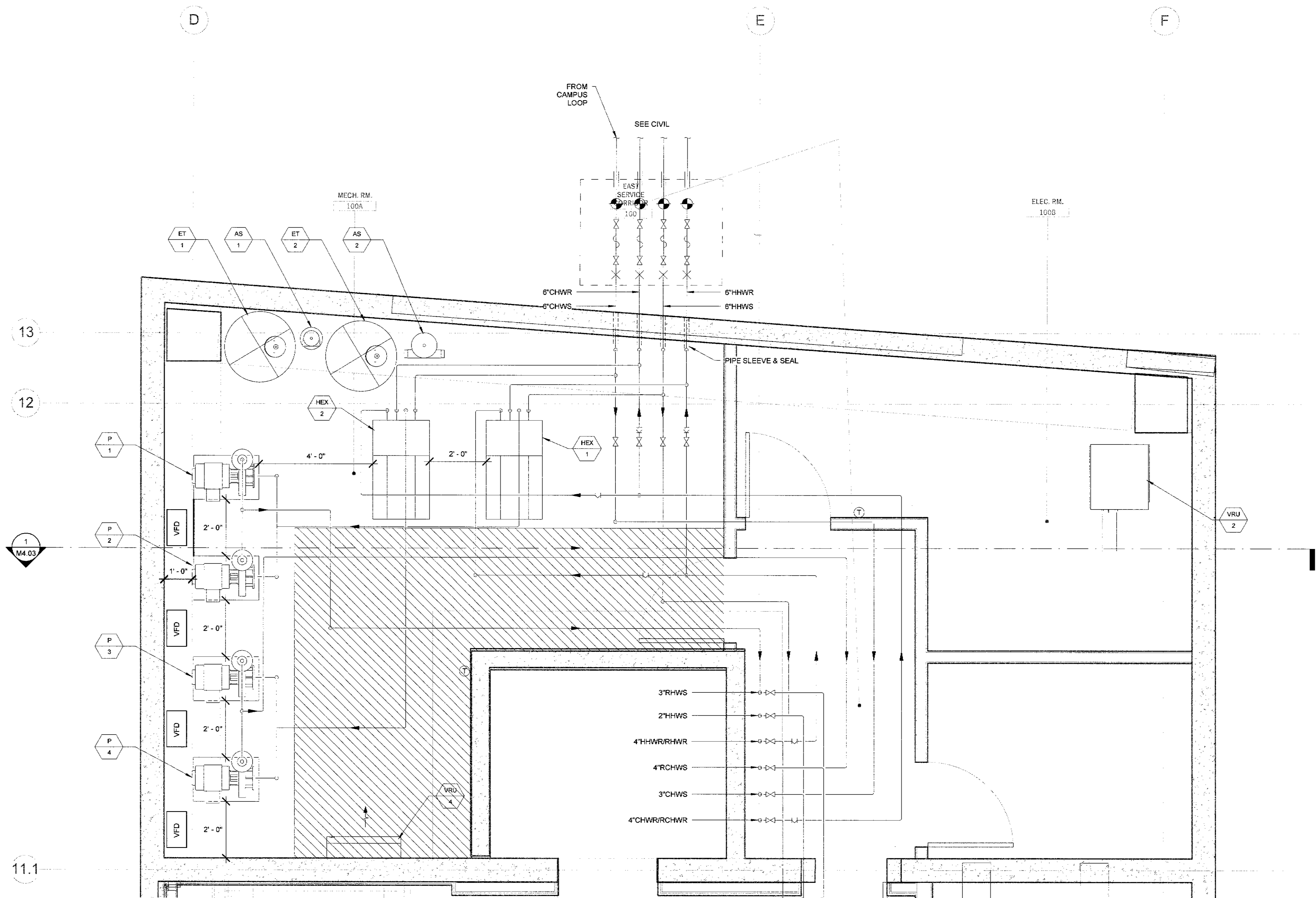
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GENERAL NOTES:

1. NOT ALL FIXTURES & FITTINGS ARE INDICATED ON THIS PLAN. PLEASE ALSO REFER TO M&E-1.
2. KEEP ALL PIPING HIGH TO ALLOW EASY ACCESS AROUND EQUIPMENT. MINIMUM CLEARANCE 7'6" TO UNDERSTAND OF PIPE SUPPORT.
3. PROVIDE ISOLATION VALVE TO ALL EQUIPMENT.
4. PROVIDE 6" EQUIPMENT PAD AS SHOWN ON FLOOR PLAN.

SHEET NOTES:

1. CONTRACTOR TO PAINT MARINE GRAY, PLANT ROOM FLOOR WITH ANTI-SLIP FINISH. STRIPE ACCESS AREA TO PREVENT EQUIPMENT STORAGE.
2. PAINT WALLS & CEILING, GLOSS WHITE.
3. STRIPE FLOOR FOR ACCESS TO VFD'S BETWEEN PUMPS.



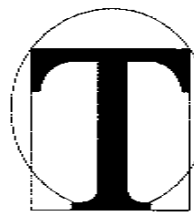
1 Mechanical Room Level 1 East Wing
1/2" = 1'-0"

Steinberg Architects

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5020 Franklin Dr.
Pleasanton, CA 94588

ARCHITECT
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60 Pierce Avenue
San Jose, CA 95110

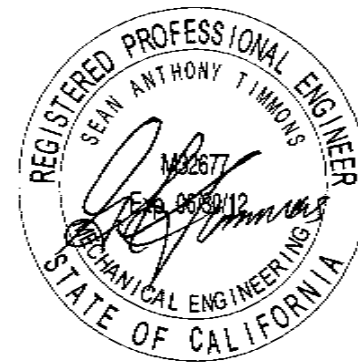
CONSULTANT



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FILE #	1-C2
AC	WJS
SSS	WJS
DATE	OCT 21 2010
IDENTIFICATION STAMP	



09	10.14.2010	DSA BACKCHECK
08	07.19.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD DSA MEETING
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02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

REV	NO	DATE	ISSUE
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Student Services and
Administration
Building

Las Positas College

3000 Campus Hill Dr.,
Livermore, CA 94551

MECHANICAL ENLARGED PLANS

PROJECT #: 09050.000
DATE: 2010 July 12
SCALE: 1/2" = 1'-0"

M4.01

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APPL # 01-111019
FILE # 1-C2
AC # WJ FLS # 2
SSS # WJ DATE 12/12/2009
IDENTIFICATION STAMP



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06	05.03.2010	75% CD-DSA MEETING
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Student Services and
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Las Positas College
3000 Campus Hill Dr.,
Livermore, CA 94551

EAST ELEVATION DETAIL OF
ELEVATOR DISPLACEMENT

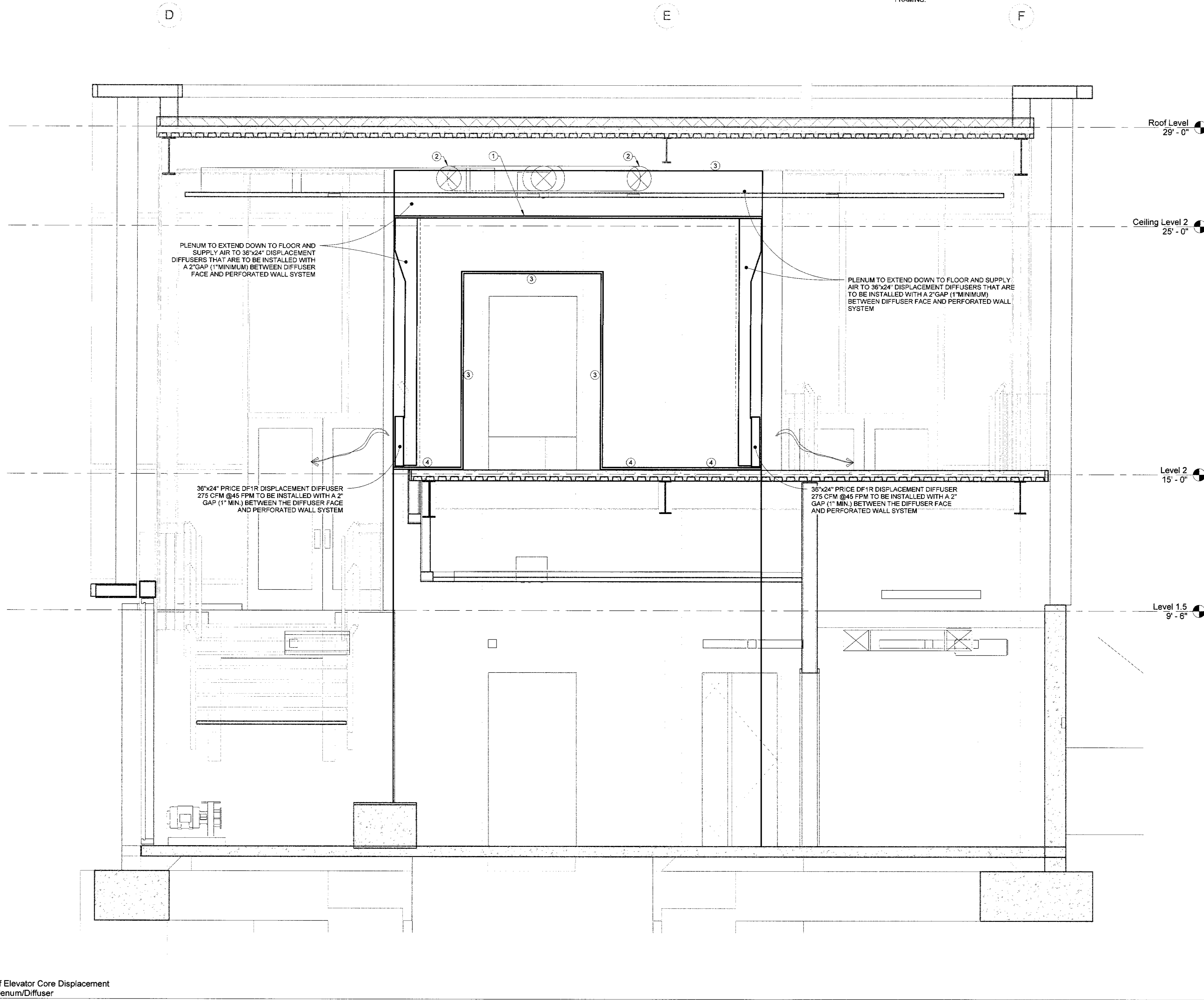
PROJECT #: 09050.000
DATE: 2010.July.12
SCALE: 1/2" = 1'-0"

SHEET NOTES:

1. ELEVATOR CORE NEEDS TO BE AIR TIGHT TO ACCEPT DISPLACEMENT DIFFUSER. SEAL ALL ELEVATOR CORE PENETRATIONS TO AVOID AIR LEAKAGE.
2. SEE ARCHITECT DRAWING FOR ELEVATOR CORE OUTER SKIN DETAIL.

NUMBERED NOTES:

1. 20 GA. GALV. STEEL PLENUM WITH 1" STANDING SEAM BOLTED AT 6" O.C. WITH CHROME HEX HEAD 1/2"x1-1/4" BOLTS AND LOCKING NUTS.
2. CONNECT DUCT TO PLENUM
3. SEAL PLENUM TO ELEVATOR SHAFT WALL WITH FORMED C CHANNEL SAME MATERIAL.
4. SEAL PLENUM TO FLOOR WITH DUCT SEALANT.
5. PROVIDE OPEN STUDS OF SIMILAR GAUGE MATERIAL TO SUPPORT AIR PLENUM OFF ELEVATOR CORE WALL. COORDINATE WITH ELEVATOR CORE FRAMING.

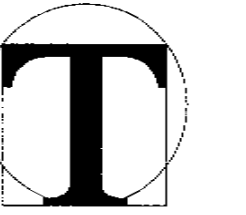


East Detail of Elevator Core Displacement
Ventilation Plenum/Diffuser
1/2" = 1'-0"

client
Chabot Las Positas Community College District
5020 Franklin Dr.
Pleasanton, CA 94588

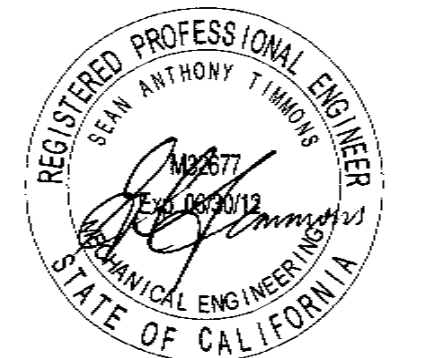
ARCHITECT
Steinberg Architects
60 Pierce Avenue
San Jose, CA 95110

CONSULTANT



TIMMONS DESIGN ENGINEERS, INC.
MEP Engineers & Sustainable Consultants
601 Market Street, Suite 450
San Francisco, CA 94103
Phone: (415) 957-0188 Fax: (415) 957-0798
www.timmonsdesigning.com

DIVISION OF
THE STATE ARCHITECT
APPL # 01-111019
FILE # 1-C2
AC # WJD FLS
SSS WJD DATE OCT 21 2010
IDENTIFICATION STAMP



09 10.14.2010 DSA BACKCHECK
08 07.19.2010 100% CD SUBMITTAL TO DSA
06 06.03.2010 75% CD-DSA MEETING
05 05.20.2010 75% STRUCTURE - DSA
COLLABORATIVE REVIEW
04 03.25.2010 100% DD SUBMITTAL TO LPC
03 03.11.2010 80% DD - DSA MEETING
02 12.12.2009 SD SUBMITTAL - DSA
01 11.19.2009 90% SD - DSA MEETING

REV	NO	DATE	ISSUE

Student Services and
Administration
Building

Las Positas College
3000 Campus Hill Dr.,
Livermore, CA 94551

WEST ELEVATION DETAIL OF
ELEVATOR DISPLACEMENT

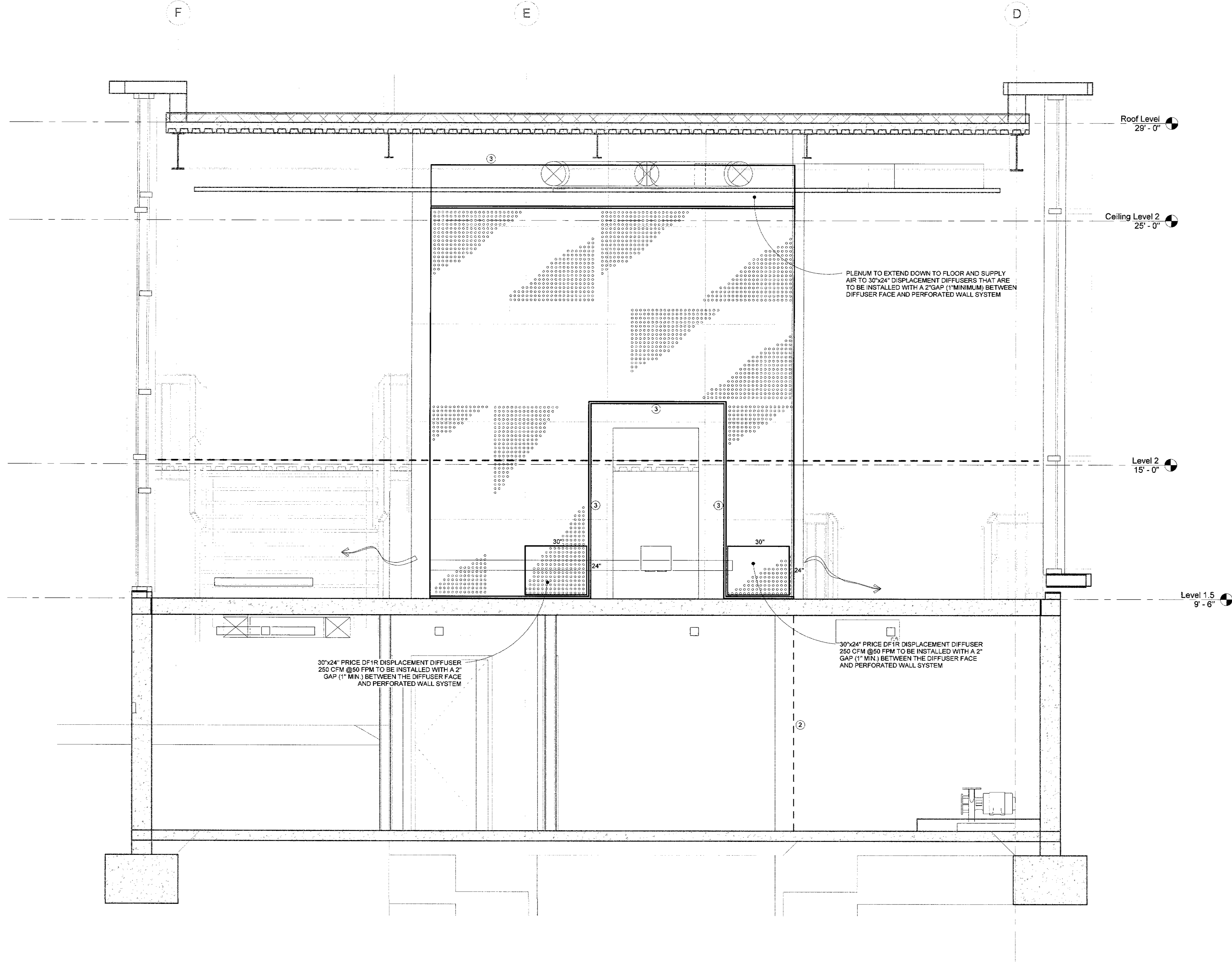
PROJECT #: 09050.000
DATE: 2010.July.12
SCALE: 1/2" = 1'-0"

GENERAL NOTES:

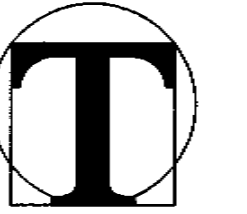
1. ELEVATOR CORE NEEDS TO BE AIR TIGHT TO ACCEPT DISPLACEMENT DIFFUSER. SEAL ALL ELEVATOR CORE PENETRATIONS TO AVOID AIR LEAKAGE.
2. SEE ARCHITECT DRAWING FOR ELEVATOR CORE SKIN DETAIL.

SHEET NOTES:

- ① 20 GA. GALV. STEEL PERFORATED DIFFUSER WITH 1" STANDING SEAM BOLTED AT 6" O.C WITH CHROME HEX HEAD 1/2"x1-1/4" BOLTS AND LOCKING NUTS. DIFFUSER MICRO PERFORATION TO BE 40% FREE AREA.
- ② DIFFUSER RUNS FULL TO FLOOR ON THREE SIDES OF ELEVATOR CORE, WEST, NORTH AND SOUTH.
- ③ SEAL DIFFUSER TO ELEVATOR CORE WALL WITH FORMED C CHANNELS BLANK 20 GA. GALV. STEEL.
- ④ PROVIDE OPEN STUDS OF SIMILAR GA. MATERIAL TO SUPPORT AIR DIFFUSER OFF ELEVATOR CORE WALL. COORDINATE WITH ELEVATOR CORE FRAMING.



West Elevation Detail of Elevator Core
Displacement Vent Plenum/Diffuser
1/2" = 1'-0"



TADAMON DESIGN ENGINEERS, INC.

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DIVISION OF
THE STATE ARCHITECT
APPL # 01-111019
REV # 1-02
AC: [Signature] RL: [Signature]
SS: [Signature] DATE: 11/19/2009
IDENTIFICATION STAMP



09	10.14.2010	DSA BACKCHECK
08	07.13.2009	100% CD SUBMITTAL TO DSA
06	06.03.2009	75% CD/SDA MEETING
05	05.20.2009	75% STRUCTURE - DSA
		COLLABORATIVE REVIEW
04	03.25.2009	100% CD SUBMITTAL TO LDC
03	03.12.2009	80% CD - SDA MEETING
02	12.22.2008	90% SUBMITTAL - DSA
01	11.19.2008	90% SD - SDA MEETING

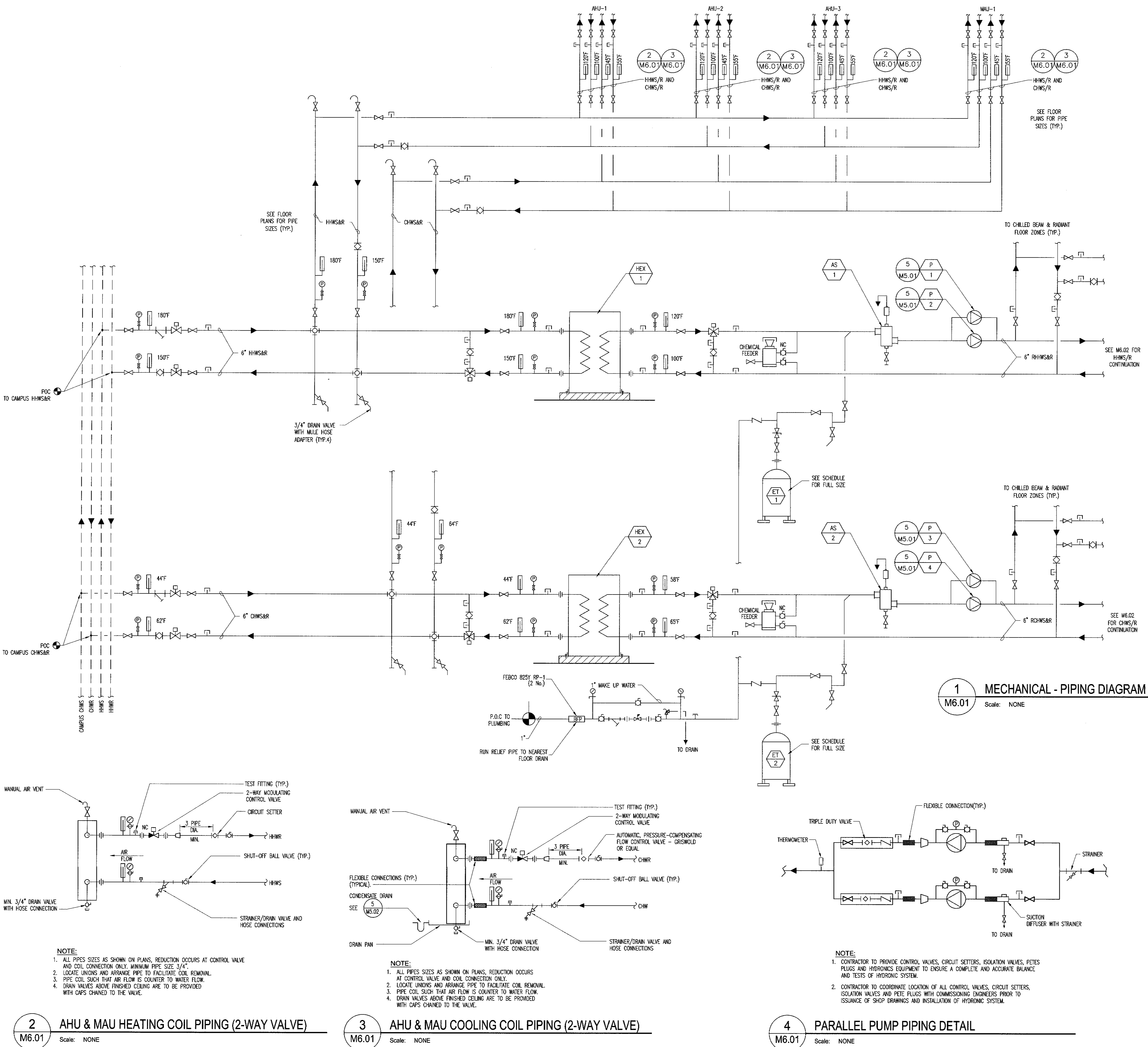
Student Services and
Administration
Building

Las Positas College

3000 Campus Hill Dr.
Livermore, CA 94551

MECHANICAL
PIPING DIAGRAM

PROJECT #: 09050.000
DATE: 2/10/2010
SCALE: None



NOTE:
1. ALL PIPES SIZES AS SHOWN ON PLANS, REDUCTION OCCURS AT CONTROL VALVE AND COIL CONNECTION ONLY. MINIMUM PIPE SIZE 3/4".
2. LOCATE UNIONS AND ARRANGE PIPE TO FACILITATE COIL REMOVAL.
3. PIPE COIL SUCH THAT AIR FLOW IS COUNTER TO WATER FLOW.
4. DRAIN VALVES ABOVE FINISHED CEILING ARE TO BE PROVIDED WITH CAPS CHAINED TO THE VALVE.

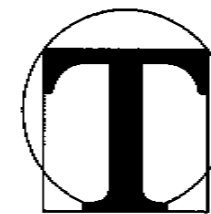
NOTE:
1. ALL PIPES SIZES AS SHOWN ON PLANS, REDUCTION OCCURS AT CONTROL VALVE AND COIL CONNECTION ONLY.
2. LOCATE UNIONS AND ARRANGE PIPE TO FACILITATE COIL REMOVAL.
3. PIPE COIL SUCH THAT AIR FLOW IS COUNTER TO WATER FLOW.
4. DRAIN VALVES ABOVE FINISHED CEILING ARE TO BE PROVIDED WITH CAPS CHAINED TO THE VALVE.

NOTE:
1. CONTRACTOR TO PROVIDE CONTROL VALVES, CIRCUIT SETTERS, ISOLATION VALVES, PETES PLUGS AND HYDRONICS EQUIPMENT TO ENSURE A COMPLETE AND ACCURATE BALANCE AND TESTS OF HYDRONIC SYSTEM.
2. CONTRACTOR TO COORDINATE LOCATION OF ALL CONTROL VALVES, CIRCUIT SETTERS, ISOLATION VALVES AND PETE PLUGS WITH COMMISSIONING ENGINEERS PRIOR TO ISSUANCE OF SHOP DRAWINGS AND INSTALLATION OF HYDRONIC SYSTEM.

Client:
Chico State Community College District
5020 Franklin Dr.
Pleasanton, CA 94588

Architect:

Consultant:



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MEP Engineers & Constructors Consultants
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DIVISION OF THE STATE ARCHITECT	
APPL #	01-111019
FILE #	1-C2
AC	FS
SSS	DATE 10/21/2010
IDENTIFICATION STAMP	



09	10/14/2010	DSA BADCHECK
08	07/25/2010	100% CD SUBMITTAL TO DSA
05	06/03/2010	75% CD-DSA MEETING
05	05/20/2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03/25/2010	100% DE SUBMITTAL TO LPC
03	03/11/2010	80% CD - DSA MEETING
02	12/22/2009	50% SUBMITTAL - DSA
01	11/19/2009	50% SD - DSA MEETING

NO DATE

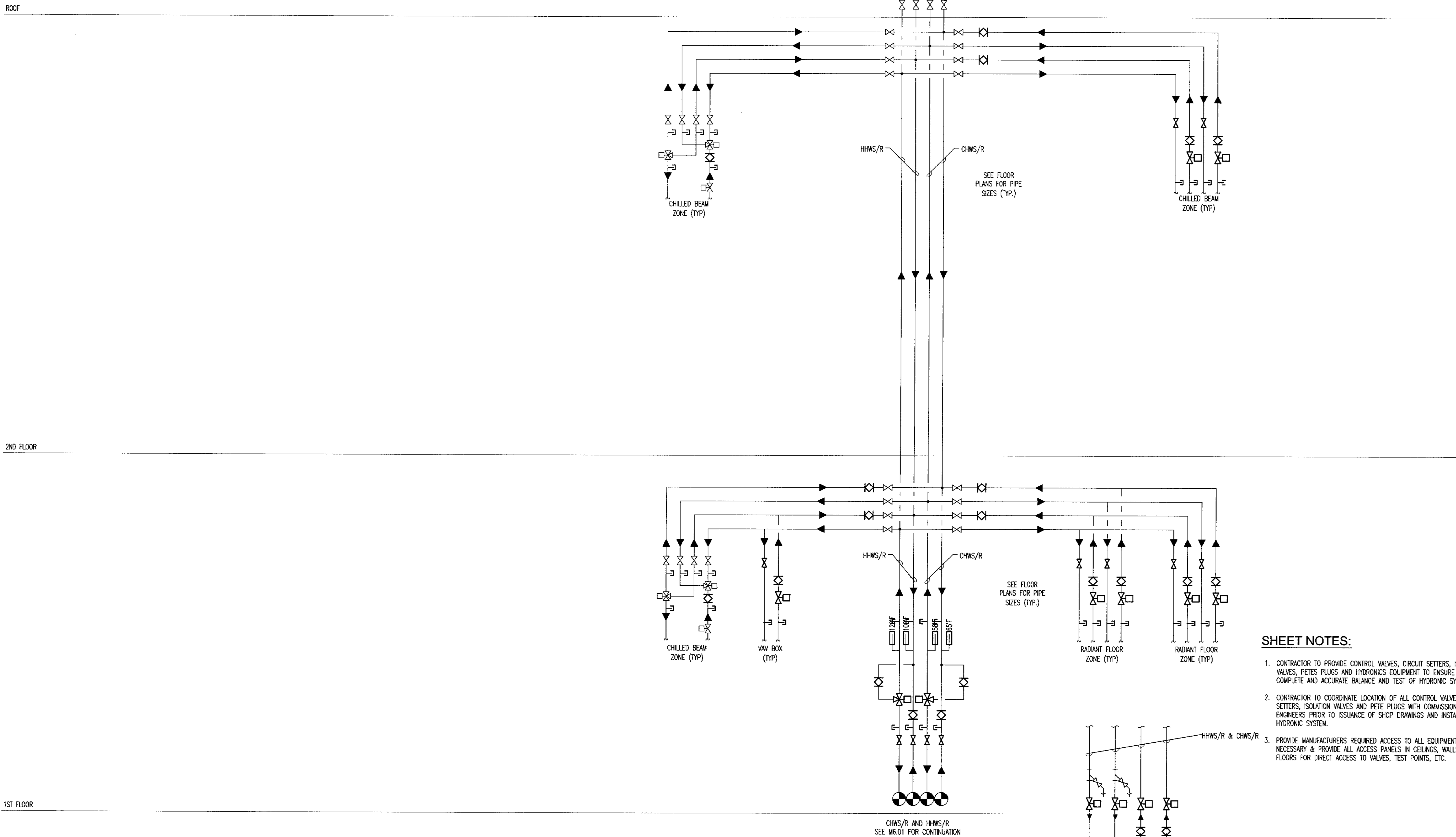
Student Services and
Administration
Building

Las Positas College

3000 Campus Hill Dr.
Livermore, CA 94551

MECHANICAL
PIPING DIAGRAM

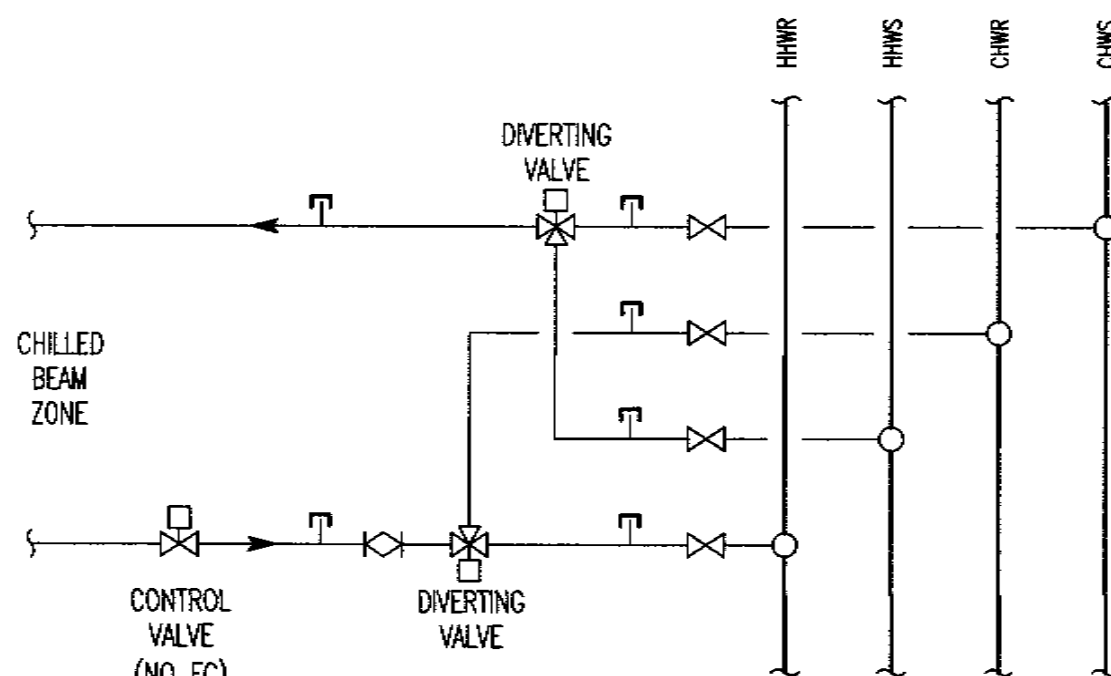
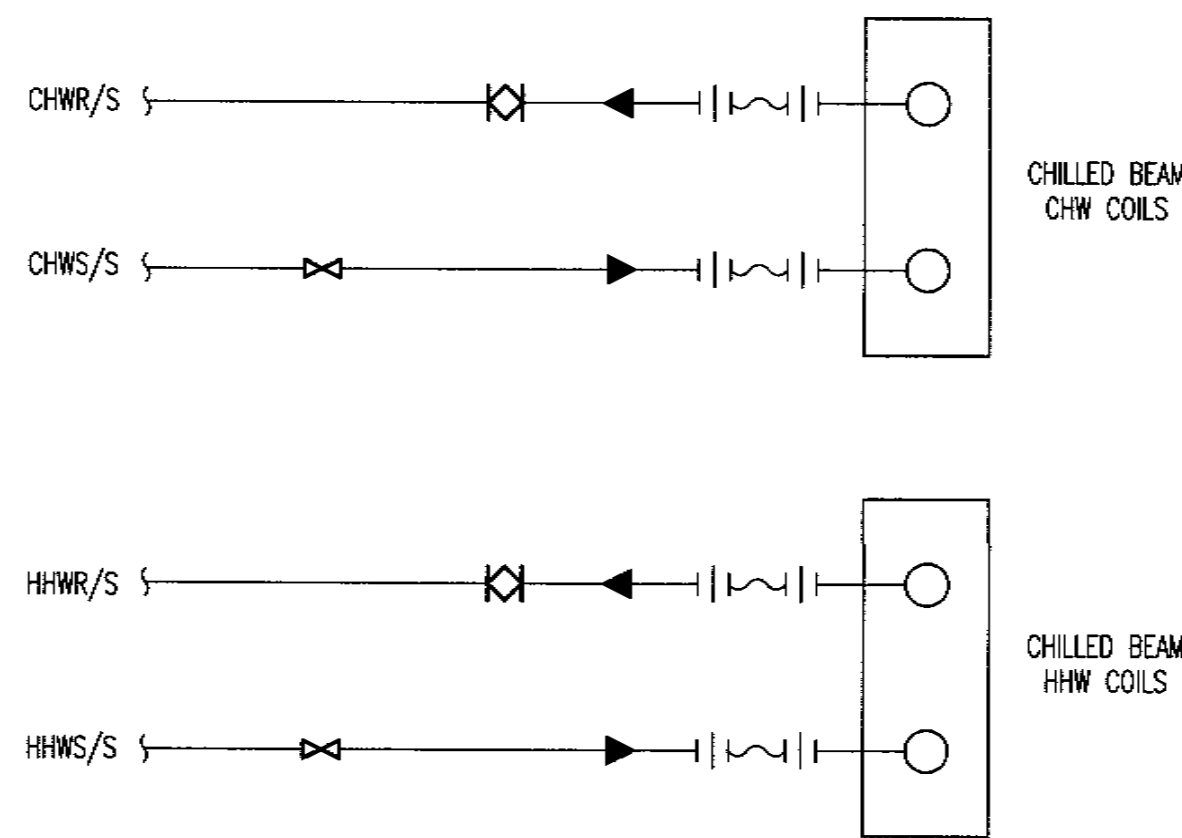
PROJECT #: 09050.000
DATE: 2010.07.19
SCALE: None



SHEET NOTES:

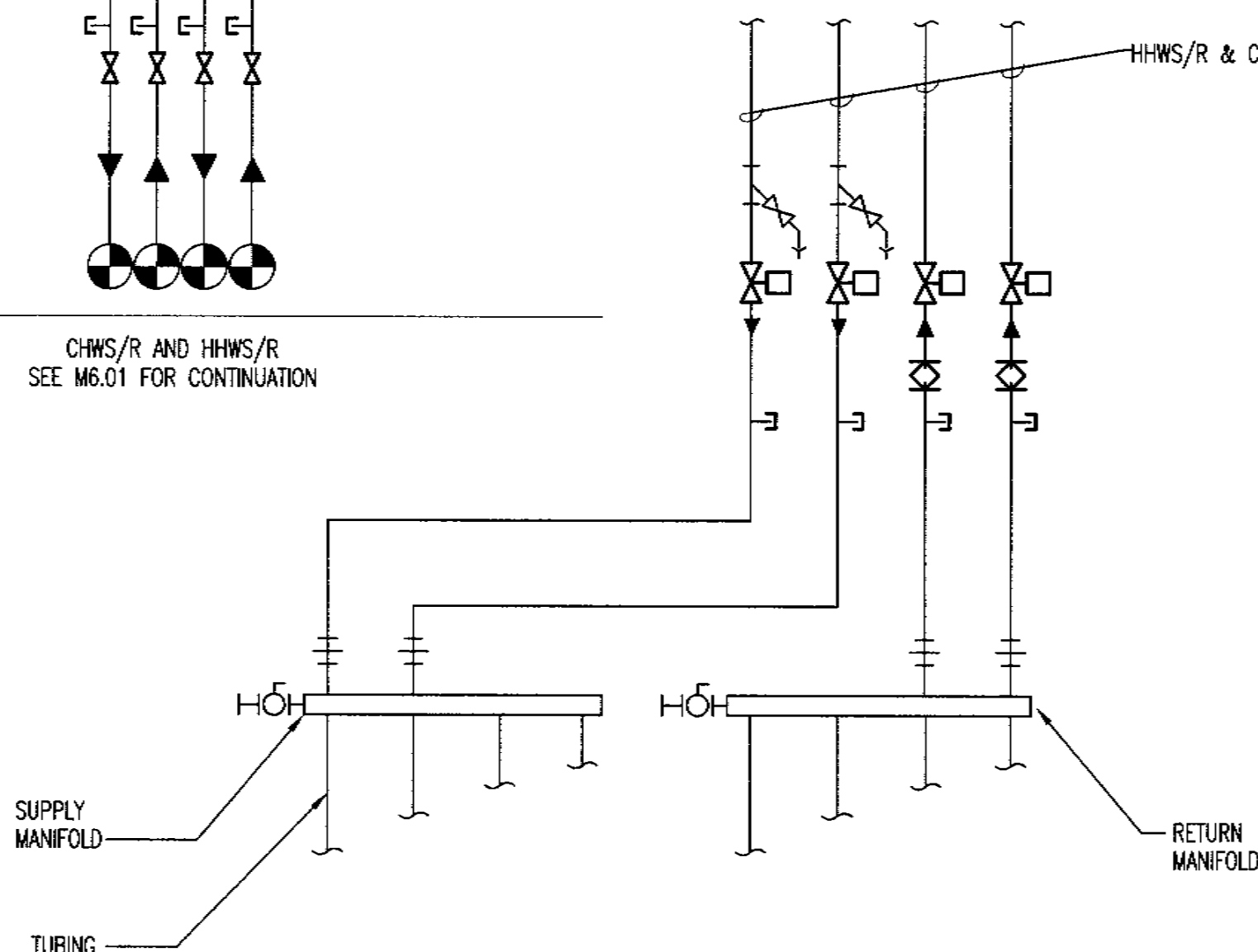
1. CONTRACTOR TO PROVIDE CONTROL VALVES, CIRCUIT SETTERS, ISOLATION VALVES, PETE PLUGS AND HYDRONICS EQUIPMENT TO ENSURE A COMPLETE AND ACCURATE BALANCE AND TEST OF HYDRONIC SYSTEM.
2. CONTRACTOR TO COORDINATE LOCATION OF ALL CONTROL VALVES, CIRCUIT SETTERS, ISOLATION VALVES AND PETE PLUGS WITH COMMISSIONING ENGINEERS PRIOR TO ISSUANCE OF SHOP DRAWINGS AND INSTALLATION OF HYDRONIC SYSTEM.
3. PROVIDE MANUFACTURERS REQUIRED ACCESS TO ALL EQUIPMENT AS NECESSARY & PROVIDE ALL ACCESS PANELS IN CEILINGS, WALLS & FLOORS FOR DIRECT ACCESS TO VALVES, TEST POINTS, ETC.

1 MECHANICAL HHW/CHW PIPING DIAGRAM
M6.02 Scale: NONE



3 STANDARD 4 PIPE - 2 PIPE VALVE CONNECTION DETAIL
M6.02 Scale: NONE

4 RADIANT HEATING/COOLING MANIFOLD DIAGRAM
M6.03 Scale: NONE



- NOTE:
1. COORDINATE MANIFOLD DIMENSIONS AND EXACT LOCATIONS WITH ARCHITECTS.
 2. PROVIDE AIR VENT AT HIGH POINT OF EACH CIRCUIT AS PART OF MANIFOLD ASSEMBLY.
 3. ALL LOOPS CONNECTING TO ONE MANIFOLD SHALL BE THE SAME LENGTH.

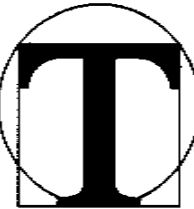
2 CHILLED BEAM PIPING DETAIL PER BEAM
M6.02 Scale: NONE

1ST FLOOR

Client
Chabot Las Positas Community College District
5500 Franklin Dr.
Pleasanton, CA 94588

ARCHITECT

CONSULTANT



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MEP Engineers & Sustainable Consultants
801 Howard Street, Suite 400
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Phone: (415) 957-8780 Fax: (415) 957-8788
www.tmmconsdesigneng.com

GENERAL NOTE:

PROVIDE ALL 120VAC AND 24VOLT WIRING AS REQUIRED FOR COMPLETE AND OPERATING CONTROLS SYSTEM. 120V POWER TO CONTROL DEVICES PROVIDED BY DIVISION 16. PROVIDE ALL REQUIRED TRANSFORMERS FOR CONTROL DEVICES.

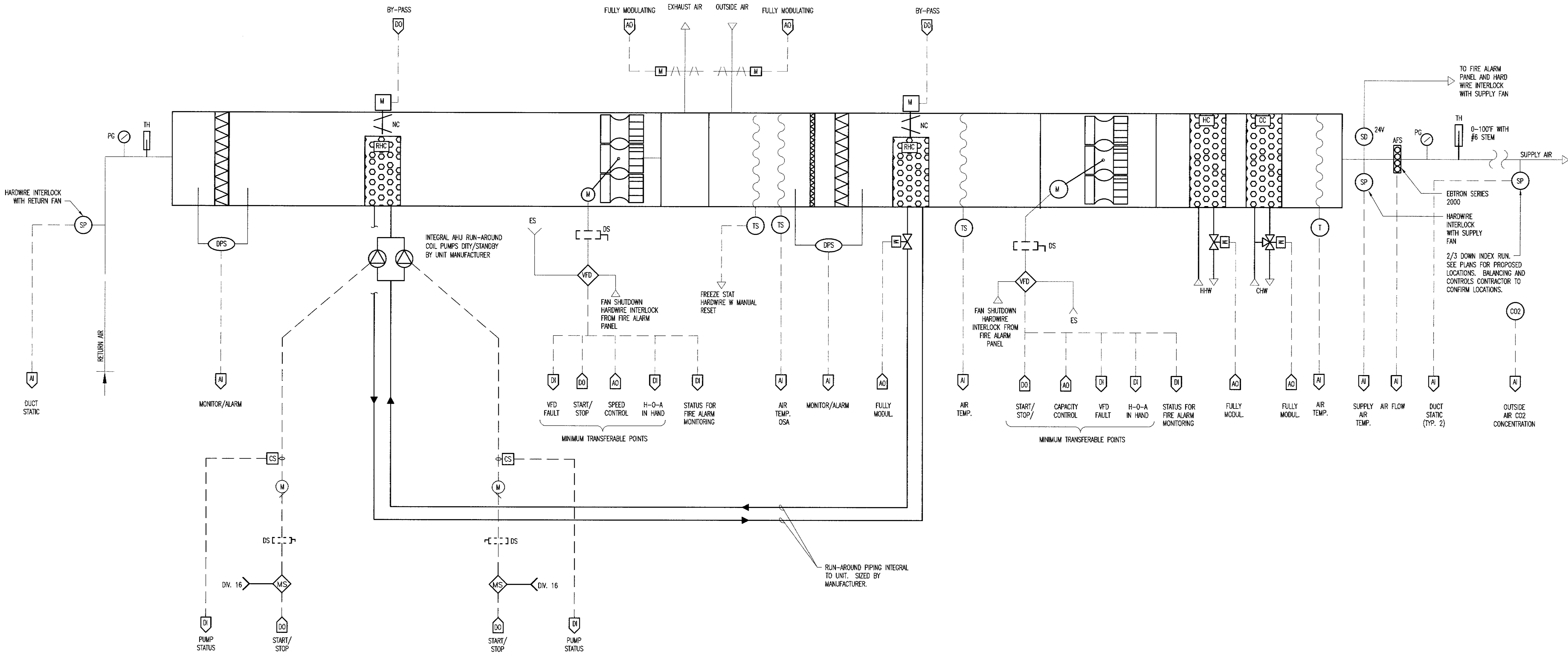
ALL SETPOINTS, CONTROL VARIABLES AND FIGURES SHOWN IN [] BRACKETS ARE TO BE EASILY ADJUSTABLE BY THE BUILDING OPERATOR. SETPOINTS SHALL BE INDIVIDUAL FOR EACH PIECE OF EQUIPMENT, EVEN IF CONTROLS DETAIL/DRAWING IS TYPICAL.

ENSURE THAT NUISANCE ALARMS DO NOT OCCUR DURING PLANT START UP, FIRE CONDITIONS, PLANT MANUAL OVERRIDES OR OTHER IRREGULAR SITUATIONS.

SHUTOFF FOR SMOKE CONTROL:

AIR MOVING SYSTEMS SUPPLYING AIR IN EXCESS OF 2,000 CUBIC FEET PER MINUTE (940L/s) TO ENCLOSED SPACES WITHIN BUILDING SHALL BE EQUIPPED WITH AN AUTOMATIC SHUTOFF. AUTOMATIC SHUTOFF SHALL BE ACCOMPLISHED BY INTERRUPTING THE POWER SOURCE OF THE AIR-MOVING EQUIPMENT UPON DETECTION OF SMOKE IN THE MAIN SUPPLY AIR DUCT SERVED BY SUCH EQUIPMENT. SMOKE DETECTORS WHICH WILL DETECT PRODUCTS OF COMBUSTION OTHER THAN HEAT AND WHICH COMPLY WITH THE CALIFORNIA BUILDING CODE, SHALL BE LABELED BY AN APPROVED AGENCY FOR AIR-DUCT INSTALLATION AND SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. SUCH DEVICES SHALL BE COMPATIBLE WITH THE OPERATING VELOCITIES, PRESSURES, TEMPERATURES AND HUMIDITIES OF THE SYSTEM. WHERE FIRE DETECTION OR ALARM SYSTEMS ARE PROVIDED FOR THE BUILDING, THE SMOKE DETECTORS REQUIRED BY THIS SECTION SHALL BE SUPERVISED BY SUCH SYSTEMS AND SHALL ACTIVATE THE FIRE ALARM SYSTEM.

MINIMUM OUTSIDE AIR:
THE POSITION OF THE ECONOMIZER DAMPERS SHALL BE OVERRIDDEN SO THAT THE CO2 CONCENTRATION IN ANY ZONE IS NOT PERMITTED TO RISE MORE THAN [500] PPM ABOVE THE OUTSIDE AIR CONCENTRATION. DURING NORMAL OCCUPIED HOURS THE ECONOMIZER DAMPERS SHALL NEVER BE PERMITTED TO REDUCE THE OA QUANTITY BELOW 0.15 CFM/SF OR AS REQUIRED BY CODE OR INDICATED ON THE AHU SCHEDULE. THE ACTUAL OA FLOW VOLUME SHALL BE MEASURED BY THE EITRON (OR EQUAL) HOT WIRE FLOW GIRD. ACCURACY MUST BE WITHIN 15%. THE OUTSIDE AIR CO2 CONCENTRATION SHALL BE MEASURED BY A SENSOR LOCATED ADJACENT TO THE OUTSIDE AIR TEMPERATURE SENSOR. ON THE AHU GRAPHIC PROVIDE A REAL TIME INDICATION (AND 30 MINUTE TREND LOG) OF THE DIFFERENCE BETWEEN EACH ZONE'S CO2 CONCENTRATION, RETURN AIR CO2 CONCENTRATION, AND THE OUTSIDE AIR CO2 CONCENTRATION. CO2 SENSORS MUST BE LOCATED IN ALL SPACES WITH OCCUPANCIES GREATER THAN 25 PEOPLE PER 1,000 SQ. FT. (INCLUDES CLASS ROOMS, BLACK BOX, AUDITORIUM, LOBBY/GALLERY, PRACTICE ROOMS).



DIVISION OF
THE STATE ARCHITECT
APPL # 01-111019
FILE # 1-62
AC # 1-62
SSS # WD
DATE 11/10/2009
IDENTIFICATION STAMP



09	10/14/2010	DSA-BRANCH/CHK
08	07/19/2010	100% CD SUBMITTAL TO DSA
06	06/03/2010	75% CD-DSA MEETING
05	05/20/2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03/25/2010	100% DD SUBMITTAL TO LPC
03	03/11/2010	80% DD - DSA MEETING
02	12/12/2009	50% SUBMITTAL - DSA
01	11/10/2009	90% SD - DSA MEETING

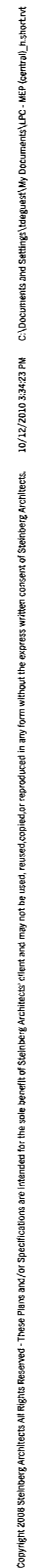
Student Services and Administration Building

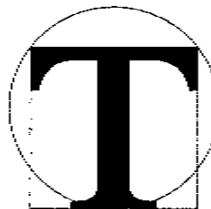
Las Positas College

2000 Camarillo Hill Dr.
Livermore, CA 94551

MECHANICAL CONTROL
DIAGRAMS AHU-1 & AHU-2

PROJECT # 09850.000
DATE 2010.07.19
SCALE None





DIVISION OF
THE STATE ARCHITECT

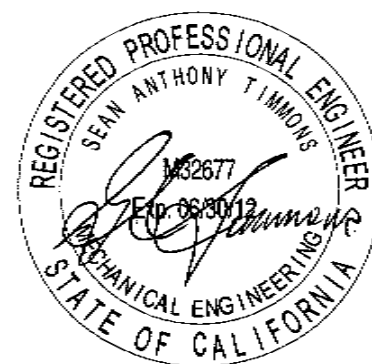
APPL # 01-111019

FILE # 1-C2

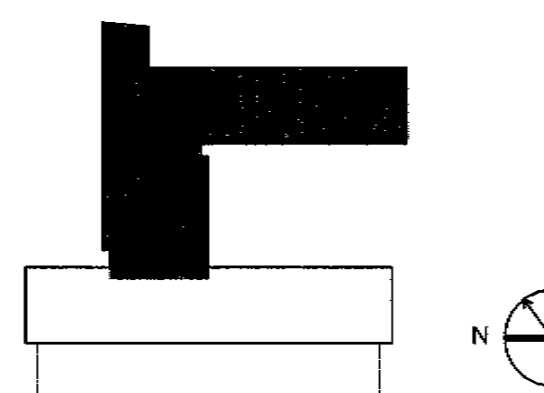
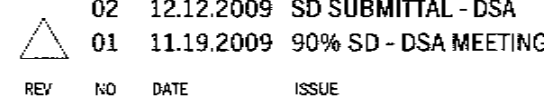
AC *HT* FLS *ED*

SSS *WD* DATE OCT 21 2001

IDENTIFICATION STAMP



09	10.14.2010	DSA BACKCHECK
08	07.19.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD-DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% DD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING



**Student Services and
Administration
Building**

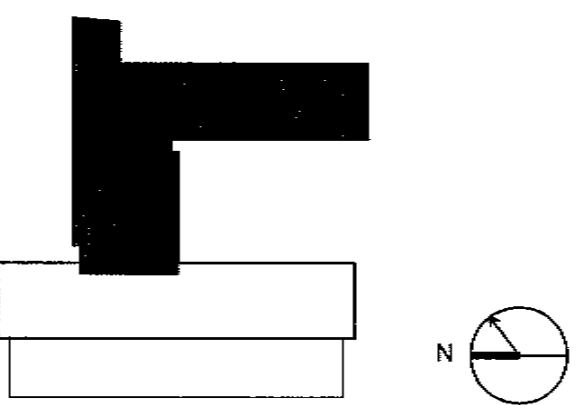
Las Positas College

3000 Campus Hill Dr.
Livermore, CA 94551

MECHANICAL PIPING ROOF
PLAN EAST WING

PROJECT #: 09050.000
DATE: 2010 July 12
SCALE: 1/8" = 1'-0"

MP2.03.1



**Student Services and
Administration
Building**

Las Positas College
3000 Campus Hill Dr.
Livermore, CA 94551

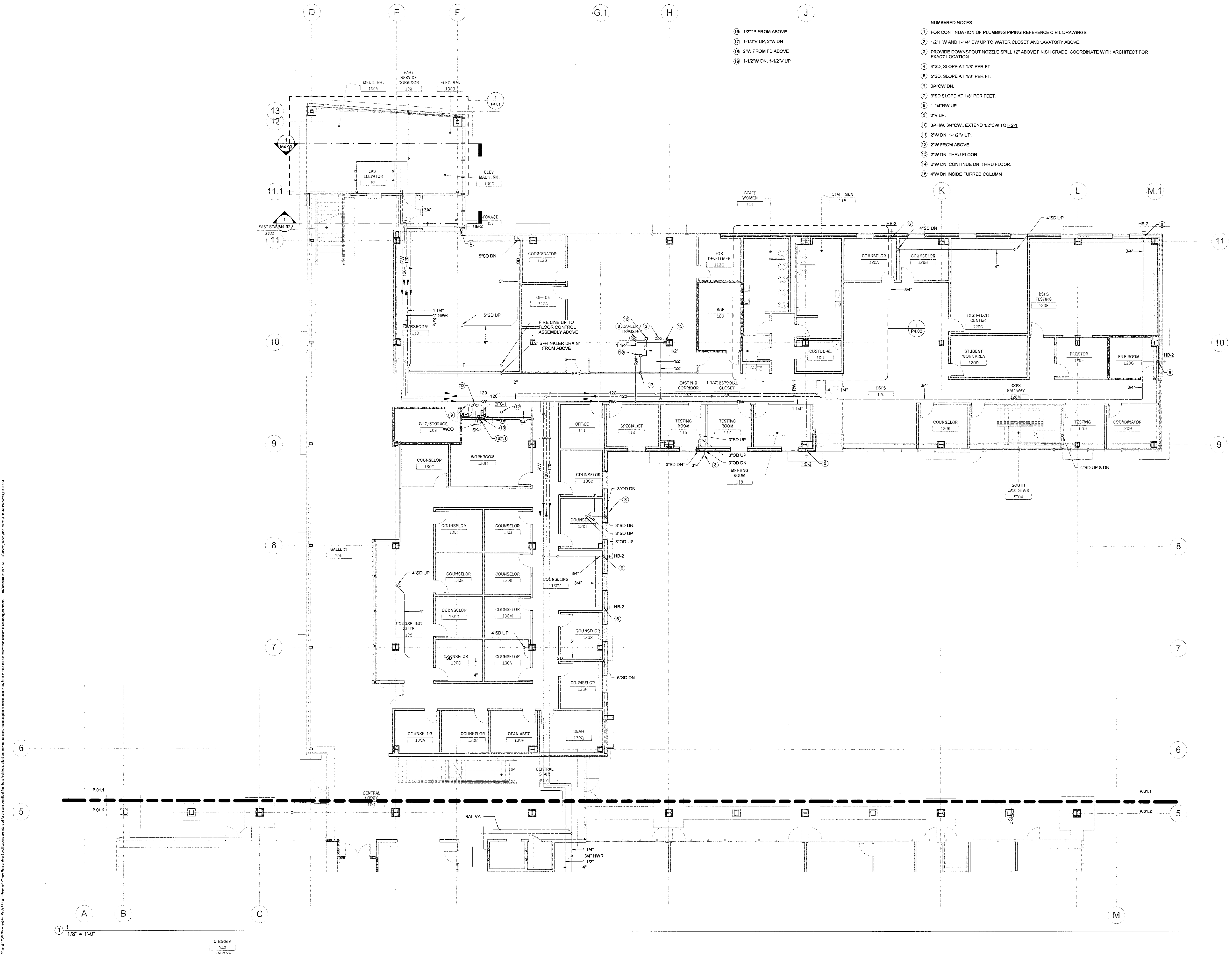
PLUMBING FLOOR PLAN
LEVEL 1 - EAST WING

PROJECT #: 09050.000
DATE: 2010 July 12
SCALE: 1/8" = 1'-0"

P2.01.1

- 16 1/2" TP FROM ABOVE
17 1-1/2" V UP, 2" W DN
18 2" W FROM FD ABOVE
19 1-1/2" W DN, 1-1/2" V UP

- NUMBERED NOTES:
1 FOR CONTINUATION OF PLUMBING PIPING REFERENCE CIVIL DRAWINGS.
2 1/2" HW AND 1-1/4" CW UP TO WATER CLOSET AND LAVATORY ABOVE.
3 PROVIDE DOWNSPOUT NOZZLE SPILL 12" ABOVE FINISH GRADE. COORDINATE WITH ARCHITECT FOR EXACT LOCATION.
4 4" SD, SLOPE AT 1/8" PER FT.
5 5" SD, SLOPE AT 1/8" PER FT.
6 3/4" CW DN.
7 3" SD SLOPE AT 1/8" PER FEET.
8 1-1/4" RW UP.
9 2" V UP.
10 3/4" HW, 3/4" CW, EXTEND 12" CW TO HS-1
11 2" W DN, 1-1/2" V UP.
12 2" W FROM ABOVE.
13 2" W DN, THRU FLOOR.
14 2" W DN, CONTINUE DN, THRU FLOOR.
15 4" W DN INSIDE FURRED COLUMN



@	AT
&	AND
ABV	ABOVE
AB	ANCHOR BOLT(S)
AC	ASPHALTIC CONCRETE
ACI	AMERICAN CONCRETE INSTITUTE
ADDL	ADDITIONAL
ADDNL	ADDITIONAL
AFF	ABOVE FINISH FLOOR
AGG	AGGREGATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ALLM	ALUMINUM
APPROX	APPROXIMATE(LY)
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING OF MATERIALS

	NATIVE SOIL (EARTH)
	COMPACTED SOIL (FINISH GRADE)
	GRAVEL, ROCK FILL
	CONCRETE
	GROUT/SAND/MORTAR/PLASTER
	STEEL CROSS SECTION
	CONCRETE MASONRY UNIT (CMU)
	PLYWOOD
	WOOD FRAMING, INTERRUPTED MEMBER
	WOOD FRAMING, CONTINUOUS THRU MEMBER

GRID LINES

MATCH LINE
SHADED PORTION IS SIDE CONSIDERED

ELEVATION, WORK POINT, CONTROL POINT
OR DATUM POINT

STEP IN ELEVATION

DETAIL NUMBER
SHEET WHERE DRAWN W/ DISCIPLINE PREFIX

ELEVATION NUMBER
SHEET WHERE DRAWN W/ DISCIPLINE PREFIX

FULL BUILDING SECTION NUMBER

SHEET WHERE DRAWN W/ DISCIPLINE PREFIX

WALL SECTION NUMBER

2. DEAD LOADS: THE ACTUAL WEIGHT OF ALL PERMANENT CONSTRUCTION AND FIXED EQUIPMENT

3. LIVE LOADS:

- A. ROOF: _____ 20 PSF (REDUCIBLE)
- B. EXPOSED ROOF MECH AREA: _____ 50 PSF (REDUCIBLE)
- C. TYPICAL FLOOR (OFFICE): _____ 60 PSF (REDUCIBLE)
- D. LOBBY: _____ 100 PSF

4. SEISMIC LOADS/EQUIVALENT LATERAL FORCE PROCEDURE

- A. SEISMIC DESIGN CATEGORY: $SD = D$
- B. OCCUPANCY CATEGORY: $OC = III$
- C. IMPORTANCE FACTOR: $I = 1.5$
- D. SITE CLASSIFICATION: $SC = D$
- E. SITE COEFFICIENTS: $F_a = 1.00$
 $F_v = 1.50$
- F. SPECTRAL RESPONSE ACCELERATIONS: $S_s = 1.523$
 $S_1 = 0.800$
- G. DESIGN SPECTRAL ACCELERATIONS: $SD_s = 1.015$
 $SD_1 = 0.600$
- H. LATERAL SYSTEM: SPECIAL STEEL MOMENT FRAMES
RESPONSE MODIFICATION FACTOR: $R = 8.0$
OVERSTRENGTH FACTOR: $\gamma = 0.30$
DEFLECTION AMP FACTOR: $C_d = 5.5$
REDUNDANCY FACTOR: $C_w = 1.0$
- I. SEISMIC BASE SHEAR: $V = C_w W = 0.159W$

THE STRUCTURE WAS ANALYZED DYNAMICALLY USING THE SITE SPECIFIC RESPONSE SPECTRA DEVELOPED BY THE GEOTECHNICAL ENGINEER. THE RESULTS WERE APPLIED TO THE EQUIVALENT LATERAL FORCE PROCEDURE BASE SHEAR.

5. WIND LOADS:

- A. BASIC WIND SPEED: 85 MPH
- B. EXPOSURE CATEGORY: C
- C. IMPORTANCE FACTOR: 1.15
- D. MWFRS: $Kzt = 1.0$
 $\lambda = 1.29$

6. COMPONENTS AND CLADDING

- A. SHALL MEET SEISMIC DESIGN REQUIREMENTS OF ASCE 7-05, CHAPTER 13, AS MODIFIED BY CBC SECTION 1613 & 1615.
- B. SHALL MEET SEISMIC REQUIREMENTS OF ASCE 7-05, CHAPTER 13, AS MODIFIED BY CBC SECTION 1609 & 1615.

1. DEFLECT UNDER FULL LOAD. A. GLASS FIBER REINFORCED CEMENTITIOUS (GFRC) PANELS
B. CURTAIN WALL SYSTEMS
2. GFRC AND CURTAIN WALLS SHALL BE DESIGNED TO MEET ALL LOADING CRITERIA STIPULATED IN THE CONTRACT DOCUMENTS.
3. THE GFRC AND CURTAIN WALL DESIGN SHALL ACCOMMODATE A MAXIMUM DISPLACEMENT IN ACCORDANCE WITH ASCE 7-05, SECTION 13.3.2 OF $D_{pl} = 4.5$ INCHES.
4. THE DESIGN AND DETAILING OF THE GFRC AND CURTAIN WALLS SHALL MEET ALL REQUIREMENTS OF THE 2010 CBC, AS ADOPTED BY UTA-SSRC. THE GENERAL CONTRACTOR SHALL SUBMIT ALL REQUIRED STRUCTURAL PLANS, SECTIONS, AND CALCULATIONS, STAMPED AND SEALED BY A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA.

3. ALL DRAWINGS AND SPECIFICATIONS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND CORRECTION OF ALL THE DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO THE ATTENTION OF THE CITY ENGINEER PRIOR TO CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR CODE REQUIREMENTS SHALL BE CORRECTED AT THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER OR ARCHITECT.
4. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION FROM COLLAPSE OR DAMAGE. BRACING IS LIMITED TO BRACING AND SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT AND MATERIALS. OBSERVATION VISITS TO THE SITE BY THE CITY ENGINEER, ENGINEERS AND ARCHITECTS ARE REQUIRED AT ALL CRITICAL TIMES.
5. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOFS OR FLOORS. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE THE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
6. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING AND FORMWORK, ETC., AS REQUIRED FOR THE PROTECTING OF LIFE AND PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT. SHORING AND TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL FLOORS, ROOF AND LATERAL BRACING SYSTEMS ARE IN PLACE AND COMPLETE.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND SHORING AND CHECK AND VERIFY ALL DIMENSIONS AS WELL AS ALL HOLES AND OPENINGS IN STRUCTURAL MEMBERS BEFORE FABRICATING SHOT DRILLING. ANY DISCREPANCIES THAT OCCUR, ANY DISCREPANCIES SHALL BE CALLED TO THE ATTENTION OF THE ARCHITECT AND SHALL BE RESOLVED BEFORE PROCEEDING WITH WORK.
8. OMISSIONS, PROMETTS, ETC., IN THE CONTRACT DOCUMENTS, UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS, NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, HOLES, ETC., IN STRUCTURAL MEMBERS NOT SHOWN ON THE STRUCTURAL DRAWINGS. FOR ANY FURTHER RESTRICTIONS ON REQUIREMENTS IN STRUCTURAL ELEMENTS, SEE APPLICABLE SECTIONS BELOW.

NYNO & MOORE, DATED NOVEMBER 3, 2009, PROJECT NO. 401420022 AND SUPPLEMENT LETTER DATED JULY 30, 2010.

2. ALL SITE WORK SHALL BE COMPLETED BUT NOT LIMITED TO: BUILDING PAD PREPARATION, TRENCHING, COMPACTION OF OVEREXCAVATION & FILL, AND BACKFILL OPERATIONS, SHALL COMPLY WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER FOR THE STRUCTURE. THE ENGINEER IS NOT RESPONSIBLE FOR ANY GEOTECHNICAL ASPECTS OF THIS PROJECT.

3. IT IS RECOMMENDED THAT THE GEOTECHNICAL ENGINEER REVIEW THE EXCAVATION PLAN AND THE CONSTRUCTION OF THE STRUCTURE, THE SITE PREPARATION, BUILDING PAD PREPARATIONS AND FOUNDATION EXCAVATIONS.

4. ANY SLOPES SHALL BE CONSTRUCTED AT A MAXIMUM GRADIENT OF 2:1 (HORIZONTAL TO VERTICAL). FILL AND CUT SLOPES SHOULD BE VEGETATED AS POSSIBLE. PROVIDE AN ANNUAL VEGETATION MAINTENANCE REPORT FOR ADDITIONAL INFORMATION. SEE CIVIL DWGS FOR BUILDING PAD EXCAVATIONS.

ALL CONCRETE SHALL BE AS FOLLOWS

A.	FOUNDATIONS, THE BMS, ETC.	150 PSC	Fc = 4000 PSI
B.	SLAB ON GRADE	150 PSC	Fc = 3000 PSI
C.	SUSPENDED SLAB	150 PSC	Fc = 4000 PSI
D.	WALLS	150 PSC	Fc = 4000 PSI
E.	MISC. PADS, CURBS, SIDEWALKS	110 PSC	Fc = 3000 PSI
F.	BEAMS AND COLUMNS	150 PSC	Fc = 4000 PSI
G.	STRUCTURAL PLATE OVER METAL DECK	150 PSC	Fc = 3000 PSI
H.	CLADDING AT STAIRS, PLATFORMS	110 PSC	Fc = 3000 PSI

2. ALL CONCRETE SHALL BE READY-MIX CONCRETE AND SHALL BE BATCHED, MIXED, AND DELIVERED TO THE SITE IN COMPLIANCE WITH ASTM 094.

3. CEMENT: ASTM C150, TYPE I OR II

4. AGGREGATE: ASTM C028

5. THE CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR REVIEW BY THE TESTING LABORATORY AND ENGINEER. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS OF MIXES. DO NOT PLACE CONCRETE PRIOR TO REVIEW OF MIX DESIGNS.

6. NON-SHRINK GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH FC = 7000 PSI.

7. ALL JOINTS ARE TO BE PLACED AT MAIN BARS, TIES, ETC., AND DENOTE CLEAR COVERAGE. THE MINIMUM CONCRETE COVER SHALL BE AS FOLLOWS UNO:

CAN-IN-PLACE CONCRETE (NON-PRESTRESSED)

A.	CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EXPOSURE	3"
B.	CONCRETE EXPOSED TO EARTH OR WEATHER BUT PLACED IN FORMS	2"
C.	#1 THRU #4 BARS, JOINT OR DIRT WIRE, AND SMALLER	2 1/2"
C.	CONCRETE NOT IN CONTACT WITH WEATHER OR IN CONTACT WITH GROUND SLABS, WALLS, OR JOISTS:	
	#1 OR #1 BARS	1 1/2"
	#1 BARS AND SMALLER	1 1/2"
	BEAMS AND COLUMNS	3"
	DIAPHRAGM REINF. STRUTS, HOOPS, TIES, SPIRALS	1 1/2"
	SHELLS AND FOLDED PLATE MEMBERS	3"
	#2 AND LARGER BARS	3 1/2"
	REIN. W/1 OR 2" DIA WIRE & SMALL FR	1 1/2"

3. ALL FORMS SHALL BE PROPERLY CONSTRUCTED, CONFORM TO THE CONTRACT SURFACE AND SHOWN TO BE PROPERLY CONSTRUCTED TO PREVENT LEAKAGE, SUFFICIENTLY RIGID AND STIFF TO MAINTAIN THEIR SHAPE AND ALIGNMENT UNTIL NO LONGER NEEDED TO MAINTAIN THE FORMS AND CONCRETE CURED.
4. FORMS AND SHORING SHALL NOT BE REMOVED UNTIL THE CONCRETE HAS ATTAINED SUFFICIENT STRENGTH TO WITHSTAND ALL LOADS TO BE IMPOSED WITHIN THE ALLOWED STRESS, OR THE DEFLECTION OF THE FORMS.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING AND COORDINATING WITH ALL TRADES THE LOCATION OF ALL ITEMS EMBEDDED IN OR PENETRATING CONCRETE ELEMENTS INCLUDING, BUT NOT LIMITED TO, PIPES, SLEEVES, DUCTS, MISC. GROOVES, BOLTS, ANCHORS, STUDS, GRIDS, ETC.
6. NO PIPES OR DUCTS SHALL BE PLACED IN STRUCTURAL CONCRETE ELEMENTS UNLESS SPECIFICALLY DETAILED.
7. ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING, DOWELS, BOLTS, ANCHORS, PIPES, SLEEVES, DUCTS SHALL BE SECURELY POSITIONED IN THE FORMS BEFORE PLACING THE CONCRETE.
8. MINIMUM EMBEDMENT FOR ANCHOR BOLTS IN CONCRETE SHALL BE 8" ON.
9. THE CONTRACTOR SHALL SUBMIT ALL PROPOSED LOCATIONS OF THE CONSTRUCTION JOINTS FOR REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO STARTING CONSTRUCTION.
10. ALL CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN OR AS APPROVED BY THE ARCHITECT/ENGINEER IN ACCORDANCE TO SECTION 1006.4 OF THE FORMS CONSTRUCTION JOINT DETAIL SHOWN.
11. ALL HARDENED SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED BY BRUSHING OR OTHER MEANS TO REMOVE DUST, CHIPS, OR OTHER FOREIGN MATTER EXPOSING FIRMLY EXPOSED AGGREGATE PRIOR TO PLACING THE ADJACENT CONCRETE.
12. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER AT LEAST 2 DAYS PRIOR TO THE PLACEMENT OF ANY STRUCTURAL CONCRETE TO ALLOW FOR STRUCTURAL OBSERVATION.
13. ALL CONCRETE IS TO BE CURED.
14. ALL CONCRETE IS TO BE CURED PER THE SPECIFICATIONS.

3. THE COLUMBIA BUILDING CODE, CHAPTER 19 AS
ADAPTED BY CSA-S800, "THE MANUAL OF STANDARD PRACTICE
FOR THE WESTERN CONCRETE REINFORCING STEEL INSTITUTE AND
THE BUILDING CODE REQUIREMENTS FOR THE STRUCTURAL
CONCRETE AND CONCRETE COMPOSITES," ACI 318-08

2. REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:

DEFORMED BARS, #3	ASTM A615, GRADE 40
DEFORMED BARS, #4 AND LARGER	ASTM A615, GRADE 60
WELDED REIN, WHEN SPECIFIED BY ENGINEER	ASTM A706
WELDED REIN FABRIC (DW) (SMOOTH WIRE)	ASTM A185
SMOOTH WIRE NOT IN WWF	ASTM A185
DEFORMED WIRE FABRIC (DW) (DEFORMED WIRE)	ASTM A497
DEFORMED WIRE NOT IN WWF	ASTM A497

3. REINFORCING FABRICATION AND PLACING SHALL CONFORM TO THE MANUAL OF
STANDARD PRACTICE OF THE WESTERN CONCRETE REINFORCING STEEL
INSTITUTE, UON.

4. ALL REINFORCING SHALL BE MARKED SO IDENTIFICATION CAN BE MADE WHEN
THE FINAL IN-PLACE INSPECTION IS MADE.

5. REINFORCING SPACING SHOWN ARE MAXIMUM ON CENTER AND ALL REINFORCING IS
CONT. JOINTS, UON.

6. REINFORCING LAP SPICES SHALL BE PER APPLICABLE TABLES ON 651.02. LAP ALL
HORIZONTAL BARS AT CORNERS AND INTERSECTIONS. STAGGER ALL JOINTS, UON
PLACES.

7. SPICES IN ADJACENT BARS SHALL BE NOT CLOSER THAN 2'-0" CLEAR DISTANCE.

8. SPICE CONTINUOUS REINFORCING IN SPANDRALS, GRADE BEAMS, WALL BEAMS,
ETC. AS FOLLOWS:

TOP REINFORCING AT MID-SPAN
BOTTOM REINFORCING AT CENTERLINE SUPPORT

9. REINFORCING DEVELOPMENT LENGTHS SHALL BE PER APPLICABLE TABLES ON
651.02

10. DOWEL ALL VERTICAL REINFORCING IN WALLS AND COLUMNS FROM FOUNDATION
WITH THE SAME SIZE REINFORCING, UON.

11. REINFORCING SHALL NOT BE WELDED UNLESS SPECIFICALLY SHOWN OR
APPROVED BY THE ENGINEER.

12. WELDING OF REINFORCING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS
USING E60XX OR APPROVED EQUIVALENTS. WELDING PROCEDURES SHALL CONFORM
TO THE REQUIREMENTS OF AWS D1.4, LATEST EDITION. REINFORCING TO BE WELDED
SHALL CONFORM TO ASTM A706.

13. REINFORCING SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIALS LIKELY TO
AFFECT ALL REINFORCING BENDS SHALL BE MADE COLD.

14. ALL REINFORCING SHALL BE SECURELY WIRED AND PROPERLY SUPPORTED ABOVE
GROUND AND AWAY FROM THE FORMS.

15. ANCHOR BOLT COUPLERS TO REINFORCING STEEL SHALL BE ERICO "54" LENTON
BOLT COUPLERS.

16. COUPLERS SHALL BE CONNECTED TO REINB BARS IN THE SHOP AND
DELIVERED TO THE JOB SITE.

17. WELDERS SHALL BE CERTIFIED IN COMPRESSION TYPY WITH THE ABILITY TO
DEVELOP 125% OF THE YIELD STRENGTH OF THE BOLT OR REINFORCING,
WHICHEVER IS LESS.

18. CONTRACTOR SHALL PROVIDE 100% MORE COUPLERS THAN REQUIRED FOR

1. ALL JOINTS SHALL BE WELDED TOGETHER TO REMAIN RIGID UNLESS SPECIFICALLY SHOWN.
2. STRUCTURAL PROPERTIES SHALL BE EQUAL TO THOSE OF THE DECKING TYPE SHOWN IN CONNECTIONS WITH REFERENCED ICC REPORTS.
3. THE MINIMUM BARE METAL THICKNESS OF METAL SHALL BE AS SHOWN.
4. DECK SHALL BEAR 2 INCHES MINIMUM ON ALL SUPPORTING STEEL.
5. DECK SHALL BE ACCORDANCE TO ALL SUPPORTING STEEL BY WELDING AS SHOWN AND IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
6. THE SIDE LIPS OF ALL PANELS SHALL BE FASTENED TOGETHER AS SHOWN.
7. WELDING: AWS D1.3
8. UPON COMPLETION OF ERECTION, ALL EXPOSED WELDS SHALL BE TOUCH UP, DE-SLAGGED AND PRIMED.
9. ALL DECKS WITH CONCRETE FILL SHALL BE VENTED.
10. THE CONTRACTOR SHALL COORDINATE OPENINGS IN METAL DECK WITH ALL TRADES, NOT ALL OPENINGS INDICATED ON ARCHITECTURAL DRAWINGS. PROVIDE REINFORCING PER TYPICAL METAL DECK DETAILS AT ALL OPENINGS.
11. WHERE LARGE PREDETERMINED OPENINGS FOR DUCTS AND SIMILAR ELEMENTS PASSING THROUGH DECKING ARE SHOWN, THE CONTRACTOR SHALL PREPARE FABRICATED JOINTS TO FIT JOINT CONDITIONS, WHERE OTHER HOLES OR OPENINGS ARE REQUIRED IN DECKING AFTER ERECTION, SUCH HOLES SHALL BE REINFORCED AS SHOWN.
12. PROVIDE SUPPORT AT COLUMNS PER 1851.014.
13. PROVIDE ALL CLOSURES, END PLATES, PROFILE PLATES AND OTHER ACCESSORIES REQUIRED FOR A COMPLETE INSTALLATION, WELD IN PLACE.
14. SUBMIT SHOP DRAWINGS FOR APPROVAL OF THE ERECTION CONTRACTOR. THE DECKING FOR REVIEW, NO FABRICATION SHALL BEGIN OR MATERIAL DELIVERED TO THE JOINT UNTIL THE ENGINEER HAS COMPLETED THE REVIEW OF THE SHOP DRAWINGS.

AND AMERICAN IRON AND STEEL SPECIFICATION MANUAL, LATEST EDITION.

2. WELDING SHALL CONFORM TO THE AWS SPECIFICATIONS.

3. WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE BEST PRACTICE AND WITHIN THE TOLERANCES SPECIFIED IN THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL.

4. ALL STRUCTURAL STEEL SHALL BE AS FOLLOWS UNLESS

ALL WF, WT SHAPES	A992, GRADE 50
CONNECTION PLATES & MISC. STEEL (UON)	ASTM A36
GUSSET & COLLECTOR PLATES	ASTM A772, GRADE 50
PIPE COLUMNS (TYPE S, SEAMLESS)	ASTM A53, GRADE 50
STRUCTURAL TUBING	ASTM A500, GRADE B
ANGLES, CHANNELS	ASTM A36

5. ALL BOLTS SHALL BE HIGH STRENGTH BOLTS PER ASTM A325 OR A325-SC AS SHOWN, UNLESS

6. ALL BOLTS TO BE USED FOR ERECTION SHALL BE ASTM A325 TYPE WITH THREADS EXCLUDED FROM SHEAR PLANES.

7. ALL ANCHOR BOLTS SHALL COMPLY WITH ASTM F1554, GRADE 55 3" MINIMUM CONCRETE COVER SHALL BE PLACED AROUND ALL ANCHOR BOLTS EXPOSED TO THE WEATHER.

8. WELDING MATERIALS: PER AWS D1.1 & D1.8

A. WELDING ELECTRODES:	E70XX SERIES AS REQUIRED FOR INTENDED USE
B. THE USE OF E70T-4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION.	
C. SEISMIC FORCE RESISTING SYSTEM & DEMAND CRITICAL WELDS SHALL COMPLY WITH CBC 2211.1.2.	

9. A WELDING SEQUENCE SHALL BE PLANNED TO MINIMIZE RESIDUAL STRESSES AND DISTORTIONS OF INDIVIDUAL MEMBERS AND THE BUILDING FRAME.

10. DO NOT WELD ANY STRUCTURAL STEEL MEMBER OR CONNECTION UNLESS IT IS FULLY CALLED OUT IN THE CONTRACT DOCUMENTS.

11. WELD SYMBOLS SHOW FINAL WELD REQUIRED. THE CHOICE TO WELD IN THE FIELD OR IN THE SHOP SHALL BE UP TO THE CONTRACTOR AND SHALL BE

23. MINIMUM FILLET WELDS SHALL BE PER TABLE 2.2.4 OF AISC 360-05.
24. ALL GROOVE & BUTT WELDS TO BE COMPLETE PENETRATION, UON
25. WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) PER THE RECOMMENDATIONS OF THE AMERICAN WELDING SOCIETY (AWS) SHALL BE DEVELOPED BY THE FABRICATOR/ERECTOR AND SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO ANY WELDING OF THE STRUCTURAL STEEL. THE WELDING PROCEDURE SHALL BE APPROVED BY THE TEST PER AISC. A PROCEDURE QUALIFICATION RECORD (PQR) SHALL BE INCLUDED WITH THE WPS IF THE WELDING PROCEDURE OR JOINT IS QUALIFIED BY TESTING. THE ELECTRODE MANUFACTURER'S DATA SHEETS WITH THE WPS SHALL BE IDENTIFIED IN THE WPS IN ADDITION TO THE AWS ELECTRODE CLASSIFICATION NAME. A COPY OF THE ELECTRODE MANUFACTURER'S TECHNICAL DATA SHEETS WITH THE WPS AND WELDING PARAMETERS SHALL BE SUBMITTED WITH THE WPS.
26. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED.
27. CAMBER
 - A. PROVIDE UPWARD CAMBER TO ALL MEMBERS SHOWN TO HAVE CAMBER. AMOUNT MEASURED IN FIELD PRIOR TO INSTALLATION SHALL NOT DEViate MORE THAN ALLOWED.
 - B. BEAMS WITHOUT SPECIFIED CAMBER SHALL BE FABRICATED SO THAT AFTER ERECTION ANY MINOR CAMBER DUE TO ROLLING OR SHOP ASSEMBLY SHALL BE UPWARD.
28. BOLT HOLES SHALL BE STANDARD SIZE IN ACCORDANCE WITH AISC SPECIFICATIONS FOR JOINTS USING ASTM A570 OR A500 BOLTS. UON
29. PENETRATIONS THROUGH STRUCTURAL STEEL MEMBERS ARE NOT ALLOWED EXCEPT AS SPECIFICALLY NOTED.
30. BURNED HOLES ARE NOT ALLOWED.
31. ALL STEFFNER PLATES ARE 3/8" THICK MINIMUM UON.
32. ERECTION CLIPS, TEMPORARY BRACING, ETC., REQUIRED BY THE CONTRACTOR ARE NOT SHOWN
33. ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP COAT OF ZINC CHROMATE PRIMER OR EQUAL. AFTER ERECTION, FIELD CONNECTIONS SHALL BE REPAINTED. DO NOT USE PORTLAND CEMENT OR PORTLAND CEMENT IN CONCRETE. AT HEADED ANCHOR STUD LOCATIONS, OR AREAS TO BE WELDED OR TO RECEIVE FIRE PROOFINGS
34. SHOP FABRICATING WORK SHALL BE DONE IN A SHOP APPROVED BY THE GOVERNING AGENCY. FABRICATOR SHALL SUBMIT PROGRAM OF WELDING INSPECTION FOR REVIEW BY THE ENGINEER
35. SUBMIT SHOP DRAWINGS FOR THE FABRICATION AND ERECTION OF ALL MEMBERS OF STRUCTURAL STEEL WORK. INCLUDE PLANS AND ELEVATIONS AT NOT LESS THAN 1" TO 1/4" SCALE, AND INCLUDE DETAILS OF SECTIONS AT NOT LESS THAN 3" TO 1/4" SCALE.
36. NO FINISH FABRICATING SHALL BE COMMENCED OR MATERIAL DELIVERED TO THE JOB UNTIL THE ENGINEER HAS REVIEWED THE SHOP DRAWINGS

- ANCHOR STUDS AS MANUFACTURED BY NIELSON STEEL WELDING OR APPROVED EQUIVALENT. DBA SHALL BE 1/2" DIA. WITH MINIMUM TENSILE STRENGTH OF 60 KSI, ASTM A308, GRADES 105-1400, WITH A MINIMUM TENSILE STRENGTH OF 60 KSI.
- 2. DEFORMED BAR ANCHOR STUDS (DBA) SHALL BE GRANULAR FLUX FILLED REBAR STUDS AS MANUFACTURED BY NIELSON STEEL WELDING OR APPROVED EQUIVALENT. DBA SHALL BE 1/2" DIA. WITH MINIMUM TENSILE STRENGTH OF 60 KSI, ASTM A308, GRADES 105-1400, WITH A MINIMUM TENSILE STRENGTH OF 60 KSI.
- 3. THE WSA / DBA SHALL BE AUTOMATICALLY END WELDED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. THE WSA SHALL PROVIDE COMPLETE FUSION WITH THE STRUCTURAL STEEL. THERE SHOULD BE NO POROSITY OR EXCESSIVE SPATTER LAGS FOR THE WSA.
- 4. THE WSA / DBA SHALL DECREASE IN LENGTH DURING WELDING. APPROXIMATELY 1" FOR 5/8" DIA. AND UNDER, AND 3/16" FOR OVER 5/8" DIA.
- 5. WELDING SHALL BE DONE ONLY BY QUALIFIED WELDERS APPROVED BY THE PROJECT INSPECTOR.

1. ALL DRILLED-IN EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ (ESR-1917) OR EQUAL.
2. ALL ADHESIVE ANCHORS SPECIFIED SHALL BE HILTI HIT-RE 500-SD (ESR-2322) OR EQUAL.
3. ANCHOR MATERIAL SHALL BE CARBON STEEL FOR INTERIOR USE AND STAINLESS STEEL FOR EXTERIOR USE, UON.
4. THE TENSION TESTING OF THE POST-INSTALLED ANCHORS SHALL BE DONE IN THE PRESENCE OF THE INSPECTOR AND A REPORT OF THE TEST SHALL BE SUBMITTED ACCORDING TO THE

D	C	100% FOR THE ATTACHMENT OF ALL OTHER STRUCTURAL ATTACHMENTS
		IF ANY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME CATEGORY NOT PREVIOUSLY TESTED UNTIL (20) CONSECUTIVE ANCHORS PASS THE TEST REQUIREMENT, THEN RESUME INITIAL TESTING FREQUENCY.
HILTI KWIK BOLT-TZ (ICC-ESR 1917) INSTALLED INTO UNDERSIDE OF LT W/ CONCRETE (3 1/4" MIN) OVER METAL DECK (F_c = 3000 PSI) WITH SPECIAL INSPECTION		
		DIAMETER & TEST LOAD (LBS)
		3/8" 1/2" 5/8" 3/4"
MAX BOLT OFFSET FROM CENTER OF FLUTE		1" 1" 1" 1"
INSTALLED INTO LOWER FLUTE		
MINIMUM EMBEDMENT		2" 3 1/4" 4" 4"
DIRECT PULL-TENSION LOAD (LBS)		1418 2544 4510 4510
INSTALLED INTO UPPER FLUTE		
MINIMUM EMBEDMENT		2" 2" - -
DIRECT PULL-TENSION LOAD (LBS)		1418 1418 - -
HILTI KWIK BOLT-TZ (ICC-ESR 1917) INSTALLED INTO TOP OF LT W/ CONCRETE (3 1/4" MIN) OVER METAL DECK (F_c = 3000 PSI) WITH SPECIAL INSPECTION		
		DIAMETER & TEST LOAD (LBS)
		3/8" 1/2" 5/8" 3/4"
MINIMUM EMBEDMENT		2" 2" - -
MINIMUM SPACING BETWEEN BOLTS		6" 6" - -
MINIMUM EDGE DISTANCE		9" 9" - -
DIRECT PULL-TENSION LOAD (LBS)		1328 1540 - -

ALL STUDS SHALL BE GALVANIZED OR GALVANNEAL BE AS NOTED BELOW.

INTERIOR STUDS: GALVANIZED

INTERIOR STUDS: GALVANIZED

ALL COLD-FORMED METAL FRAMING CONSTRUCTION SHALL BE IN ACCORDANCE WITH AISI SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS;

ALL COLD-FORMED METAL FRAMING SHALL CONFORM WITH THE FOLLOWING:

GALVANIZED STUDS:
12, 14 AND 16 GAUGE

ASTM A653, GRADE 50
(Fy = 50,000 psi)

GALVANIZED STUDS:
18 AND 20 GAUGE

ASTM A653, GRADE 33
(Fy = 33,000 psi)

GALVANIZED TOP TRACK:
12, 14 AND 16 GAUGE

ASTM A653, GRADE 50
(Fy = 50,000 psi)

SUP/TRACK SYSTEM OR EQUAL

INTERIOR VERTICAL PARTITIONS

GALVANIZED TOP TRACK:
18 AND 20 GAUGE

ASTM A653, GRADE 33
(Fy = 33,000 psi)

SUP/TRACK SYSTEMS OR EQUAL

INTERIOR VERTICAL PARTITIONS

GALVANIZED TRACK

ASTM A653, GRADE 33 UNON
(Fy = 33,000 psi)

END CLOSURES, BRIDGING
AND ACCESSORIES

DOUBLE VERTICAL STUDS SHALL BE SITCH WELDED TOGETHER ON BOTH FLANGES WITH 1/8" GROOVE WELDS 4" LONG @ 12" OC. UNON TOP AND BOTTOM TRACKS FOR INTERIOR PARTITIONS SHALL MATCH THE SIZE AND GAGE OF THE WALL STUDS AND SHALL HAVE 1 1/2" FLANGES. UNO SUP/TRACKS WHERE SHOWN, SHALL BE 1/4 GAIN. AND SHALL BE MANUFACTURED BY SUP/TRACK SYSTEMS OR EQUAL.

MINIMUM SECTION PROPERTIES FOR COLD-FORMED FRAMING AT INTERIOR AND EXTERIOR PARTITIONS AND THOSE STUDS REFERENCED ON ARCHITECTURAL DRAWINGS SHALL BE:

STUD / TRACK TYPE	MIN GA (THICKNESS)	EFFECTIVE I in ⁴	EFFECTIVE S in ³
250S137-33	20 GA (.0346")	0.203	0.156
250S137-43	18 GA (.0451")	0.251	0.205
250T150-33	20 GA (.0346")	0.179	0.107
250T150-43	18 GA (.0451")	0.252	0.154
400S137-43	18 GA (.0451")	0.776	0.392
400S137-54	16 GA (.0596")	0.953	0.457
400S162-43	18 GA (.0451")	0.892	0.443
400S162-54	16 GA (.0596")	1.098	0.533
400S200-43	18 GA (.0451")	1.047	0.509
400S200-54	16 GA (.0596")	1.292	0.580
400S200-68	14 GA (.0713")	1.589	0.766
400T200-43	18 GA (.0451")	0.811	0.311
400T200-54	16 GA (.0596")	1.037	0.397
500S137-43	18 GA (.0451")	2.042	0.670
500S137-54	16 GA (.0596")	2.518	0.809
500S162-33	20 GA (.0346")	1.793	0.577
500S162-43	18 GA (.0451")	2.316	0.767
500S162-54	16 GA (.0596")	2.860	0.927
500S162-68	14 GA (.0713")	3.525	1.164
500S200-43	18 GA (.0451")	2.683	0.873
500S200-54	16 GA (.0596")	3.319	1.002
500S200-68	14 GA (.0713")	4.101	1.317
600T150-43	18 GA (.0451")	1.858	0.534
600T150-54	16 GA (.0596")	2.374	0.678
600T200-54	16 GA (.0596")	2.541	0.717
800S200-43	18 GA (.0451")	5.302	1.293
800T200-43	18 GA (.0451")	4.230	0.741
1000S200-54	16 GA (.0596")	11.278	1.805
1000T200-43	18 GA (.0451")	7.442	0.911

THESE VALUES ARE TAKEN FROM THE SSMA CATALOG, ICD #4943P

ALTERNATE PROPRIETARY SECTIONS:

A. JAMBUSTS BY THE STEEL, PRODUCT

B. HEADERS AND SILLS

1. PROCESSLESS STEEL, PRODUCTS BY SCACCO STEEL, STUD MANF COMPANY

2. PRO-X BY BRADY CONSTRUCTION INNOVATIONS (EST-1765)

ALL COLD-FORMED FRAMING TO BE MANUFACTURED BY MEMBERS OF THE STEEL STUD MANUFACTURERS FOR CONSTRUCTION, SSMA (ICD #4943P, ICD #4943P)

CONTRACTOR SHALL COMPLY WITH ALL OF THE MANUFACTURER'S INSTALLATION SPECIFICATIONS AND RECOMMENDATIONS. ALL MATERIAL, STUDS SHALL HAVE STRENGTHENED FLANGES. SEE DRAWINGS FOR SPECIFIC DETAILS ON CONNECTIONS, BRACING, BRIDGING, ETC. CONTRACTOR SHALL PROVIDE ALL ACCESSORIES INCLUDING BUT NOT LIMITED TO: TRACKS, CLIPS, AND OTHER ACCESSORIES REQUIRED FOR COMPLETE AND PROPER INSTALLATIONS, AND AS RECOMMENDED BY THE STEEL MANUFACTURER FOR CONSTRUCTION, SSMA (ICD #4943P, ICD #4943P)

AS A MINIMUM, PROVIDE 2" #10 SMLS AT EACH COLD-FORMED FRAMING MEMBER CONNECTION. UOJ

1. FOR SHEET METAL SCREWS INSTALLED IN COLD-FORMED FRAMING, MINIMUM SPACING BETWEEN FASTENERS IS 1" 1/2" OC AND MINIMUM EDGE DISTANCE IS 3/4" TYPE UOJ.

ALL SHEET METAL SCREWS SHALL PROTRUDE NOT LESS THAN 3 THREADS OR 1/4" MIN THROUGH MATERIAL FRAMING

AS A MINIMUM, ANCHOR TRAC SHALL COMPLY WITH HILT 1.057" DIA X 1/4" PAF (EST-2269) OR 6" DIA X 1/4" PAF APPROVED BY THE PAF SHALL HAVE A MINIMUM 1/2" EMBEDMENT INTO THE CONCRETE AND SHALL BE LOCATED A MINIMUM 3" FROM ANY CONCRETE EDGE. THE PAF SHALL BE PLACED ADJACENT TO EACH WALL STUD AT A MAXIMUM OF 12" ON CENTER. PAF SHALL NOT BE USED IN CONCRETE CURBS.

FOR FASTENERS INSTALLED IN CONCRETE, MINIMUM SPACING BETWEEN FASTENERS IS 4" OC AND MINIMUM EDGE DISTANCE IS 3", TYPE UOJ

FOR HILT 1.057" DIA X 1/4" PAF (EST-2269) INSTALLED IN STEEL, 3" MINIMUM THICKNESS, MINIMUM SPACING BETWEEN FASTENERS SHALL BE 12" AND MINIMUM EDGE DISTANCE IS 1/2". FASTENERS SHALL BE DRIVEN TO A PENETRATION WHERE

HILTI RE 500-SZ (ICC-ESR 2322) INSTALLED INTO NORMAL WT CONCRETE ($f'_c = 3000$ PSI) WITH SPECIAL INSPECTION				
	DIAMETER & TEST LOAD (LBS)			
	3/8"	1/2"	5/8"	3/4"
MINIMUM EMBEDMENT	2 1/2"	3"	3 1/2"	3 1/2"
MINIMUM SPACING BETWEEN BOLTS	2"	2 1/2"	3 1/8"	3 3/4"
LOCATIONS NEAR SLAB EDGE				
MINIMUM EDGE DISTANCE	2"	2 1/2"	3 3/4"	4 1/2"
DIRECT PULL-TENSION LOAD (LBS)	900	1575	2625	3375
LOCATIONS NOT NEAR SLAB EDGE				
MINIMUM EDGE DISTANCE	4 3/4"	6 1/4"	7 3/4"	9"
DIRECT PULL-TENSION LOAD (LBS)	1500	2400	3375	3900

HILTI KWIK BOLT-TZ (ICC-ESR 1917) INSTALLED INTO NORMAL WT CONCRETE ($f'_c = 3000$ PSI) WITH SPECIAL INSPECTION				
	DIAMETER & TEST LOAD (LBS)			
	3/8"	1/2"	5/8"	3/4"
MINIMUM EMBEDMENT	2"	2"	3 1/8"	3 3/4"
MINIMUM SPACING BETWEEN BOLTS	2 1/2"	2 3/4"	3 1/2"	5"
LOCATIONS NEAR SLAB EDGE				
MINIMUM EDGE DISTANCE	2 1/2"	2 3/4"	3 1/4"	4 1/4"
DIRECT PULL-TENSION LOAD (LBS)	2200	2350	3850	7400
LOCATIONS NOT NEAR SLAB EDGE				
MINIMUM EDGE DISTANCE	3"	3 1/2"	4 3/4"	5 3/4"
DIRECT PULL-TENSION LOAD (LBS)	2400	2900	5000	8600

SPECIAL INSPECTION

1. SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 1704A OF THE 2010 CBC AS ADOPTED BY DSA-SSIC IS REQUIRED AND SHALL BE PROVIDED FOR THE FOLLOWING WORK UNDER SUPERVISION OF AN INDEPENDENT SPECIAL INSPECTION TESTING AGENCY EMPLOYED BY THE OWNER, AND DEEMED ACCEPTABLE TO THE JURISDICTION:

A) EARTHWORK

1. PROVIDE SPECIAL INSPECTION IN ACCORDANCE WITH CBC 1704A.7 AND AS INDICATED IN TABLE 1.
2. PRIOR TO THE PLACEMENT OF PREPARED FILLS, THE SPECIAL INSPECTOR SHALL DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE APPROVED GEOTECHNICAL REPORT.
3. DURING PLACEMENT AND COMPACTION OF FILL MATERIALS, THE SPECIAL INSPECTOR SHALL DETERMINE THAT MATERIALS BEING USED AND THE MAXIMUM LIFT THICKNESS COMPLY WITH THE APPROVED GEOTECHNICAL REPORT.
4. THE SPECIAL INSPECTOR SHALL DETERMINE, AT THE APPROVED FREQUENCY, THAT THE IN-PLACE DENSITY AND MOISTURE CONTENT ARE IN ACCORDANCE WITH THE APPROVED GEOTECHNICAL REPORT.

B) CONCRETE

1. PROVIDE SPECIAL INSPECTION IN ACCORDANCE WITH CBC 1704A.4, CBC 1916.1, AND AS INDICATED IN TABLE 2.

C) STRUCTURAL STEEL

1. PROVIDE SPECIAL INSPECTION IN ACCORDANCE WITH CBC 1704A.3, CBC 2211.5, AND AS INDICATED IN TABLE 3.

D) POST-INSTALLED ANCHORS

1. PROVIDE SPECIAL INSPECTION OF POST-INSTALLED ANCHORS PER APPLICABLE ICC EVALUATION REPORT.

E) SPECIAL INSPECTION FOR SEISMIC RESISTANCE IN

1. PROVIDE SPECIAL INSPECTION FOR SEISMIC RESISTANCE IN ACCORDANCE WITH CBC 1707A.
2. COLD-FORMED METAL FRAMING: PROVIDE PERIODIC INSPECTION IN ACCORDANCE WITH CBC 1707A.4 FOR:
 - a. WELDING
 - b. SCREW ATTACHMENTS, BOLTING, ANCHORING, AND OTHER FASTENERS
3. ARCHITECTURAL COMPONENTS: PROVIDE PERIODIC INSPECTION IN ACCORDANCE WITH CBC 1707A.8 FOR:
 - a. ERECTION AND FASTENING OF EXTERIOR CLADDING AND INTERIOR & EXTERIOR NONBEARING WALLS
4. MECHANICAL & ELECTRICAL COMPONENTS: PROVIDE SPECIAL INSPECTION IN ACCORDANCE WITH CBC 1707A.8 FOR:
 - a. ANCHORAGE OF ELECTRICAL EQUIPMENT
 - b. INSTALLATION OF PIPING & ASSOCIATED EQUIPMENT THAT CARRY FLAMMABLE OR COMBUSTIBLE CONTENTS

TABLE 1

CBC TABLE 1704A.7

REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION & INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	--	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	--	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	--	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	--
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	--	X

TABLE 2

CBC TABLE 1704A.4

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION & INSPECTION TASK	CONTINUOUS	PERIODIC	REFERENCED STANDARD	CBC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	--	X	ACI 318: 3.5, 7.1-7.7	1913A.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704A.3, ITEM 5B.	X	--	AWS D1.4 ACI 318: 3.5.2	--
3. INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	X	--	ACI 318: 8.1.3, 21.2.8	1911A.5, 1912A.1
4. INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.	--	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1912A.1
5. VERIFYING USE OF REQUIRED DESIGN MIX.	--	X	ACI 318: Ch. 4, 5.2-5.4	1904A.2.2, 1913A.2, 1913A.3
6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	--	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913A.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	--	ACI 318: 5.9, 5.10	1913A.6, 1913A.7, 1913A.8
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	--	X	ACI 318: 5.11, 5.13	1913A.9
9. INSPECTION OF PRESTRESSED CONCRETE: A. APPLICATION OF PRESTRESSING FORCES. B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM.	--	--	--	--
10. ERECTION OF PRECAST CONCRETE MEMBERS.	--	--	--	--
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	--	--	--	--
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	--	X	ACI 318: 6.1.1	--

TABLE 3
CBC TABLE 1704A.3
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION & INSPECTION TASK	CONTINUOUS	PERIODIC	REFERENCED STANDARD	CBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	--	X	AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS	--
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	--	X	--	--
2. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. SNUG-TIGHT JOINTS.	--	X	AISC 360, SECTION M2.5	1704A.3.3
B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	--	X		
C. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	--		
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:				
A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	--	X	AISC 360, SECTION M5.5	--
B. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	--	X	APPLICABLE ASTM MATERIAL STANDARDS	
C. MANUFACTURER'S CERTIFIED TEST REPORTS.	--	X	--	
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	--	X	AISC 360, SECTION A3.5 AND APPLICABLE AWS AS DOCUMENTS	--
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	--	X	--	--
5. INSPECTION OF WELDING:				
A. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:				
1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	X	--	AWS D1.1	1704A.3.1
2) MULTIPASS FILLET WELDS.	X	--		
3) SINGLE-PASS FILLET WELDS > 5/16"	X	--		
4) PLUG AND SLOT WELDS.	X	--		
5) SINGLE-PASS FILLET WELDS ≤ 5/16"	--	X	AWS D1.3	--
6) FLOOR AND ROOF DECK WELDS.	--	X		
B. REINFORCING STEEL:				
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	--	--	AWS D1.4 ACI 318, SECTION 3.5.2	--
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	--	--		
3) SHEAR REINFORCEMENT.	X	--		
4) OTHER REINFORCEMENT.	--	X		
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE:				
A. DETAILS SUCH AS BRACING AND STIFFENING.	--	X	--	1704A.3.2
B. MEMBER LOCATIONS.	--	X		
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	--	X		

STRUCTURAL OBSERVATION

1. STRUCTURAL OBSERVATION SHALL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD OR DESIGNEE IN ACCORDANCE WITH CBC 1710A.
2. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURAL FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE SPECIAL INSPECTOR OR THE DEPUTY INSPECTOR.
3. THE CONTRACTOR SHALL COORDINATE AND CALL FOR A PRE-CONSTRUCTION MEETING BETWEEN THE ENGINEER OF RECORD OR DESIGNEE, THE CONTRACTOR, THE AFFECTED SUBCONTRACTORS, AND THE BUILDING AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS.
4. PRIOR TO COMMENCEMENT OF OBSERVATIONS THE STRUCTURAL OBSERVER SHALL SUBMIT TO BUILDING OFFICIAL A WRITTEN STATEMENT IDENTIFYING THE FREQUENCY AND EXTENT OF STRUCTURAL OBSERVATIONS.
5. THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT:

CONSTRUCTION STAGES

A) FOUNDATIONS

B) STEEL ERECTION

C) METAL DECKING

D) COLD-FORMED STEEL

ELEMENTS / CONNECTIONS TO BE OBSERVED

REIN / DOWELS / ANCHORS

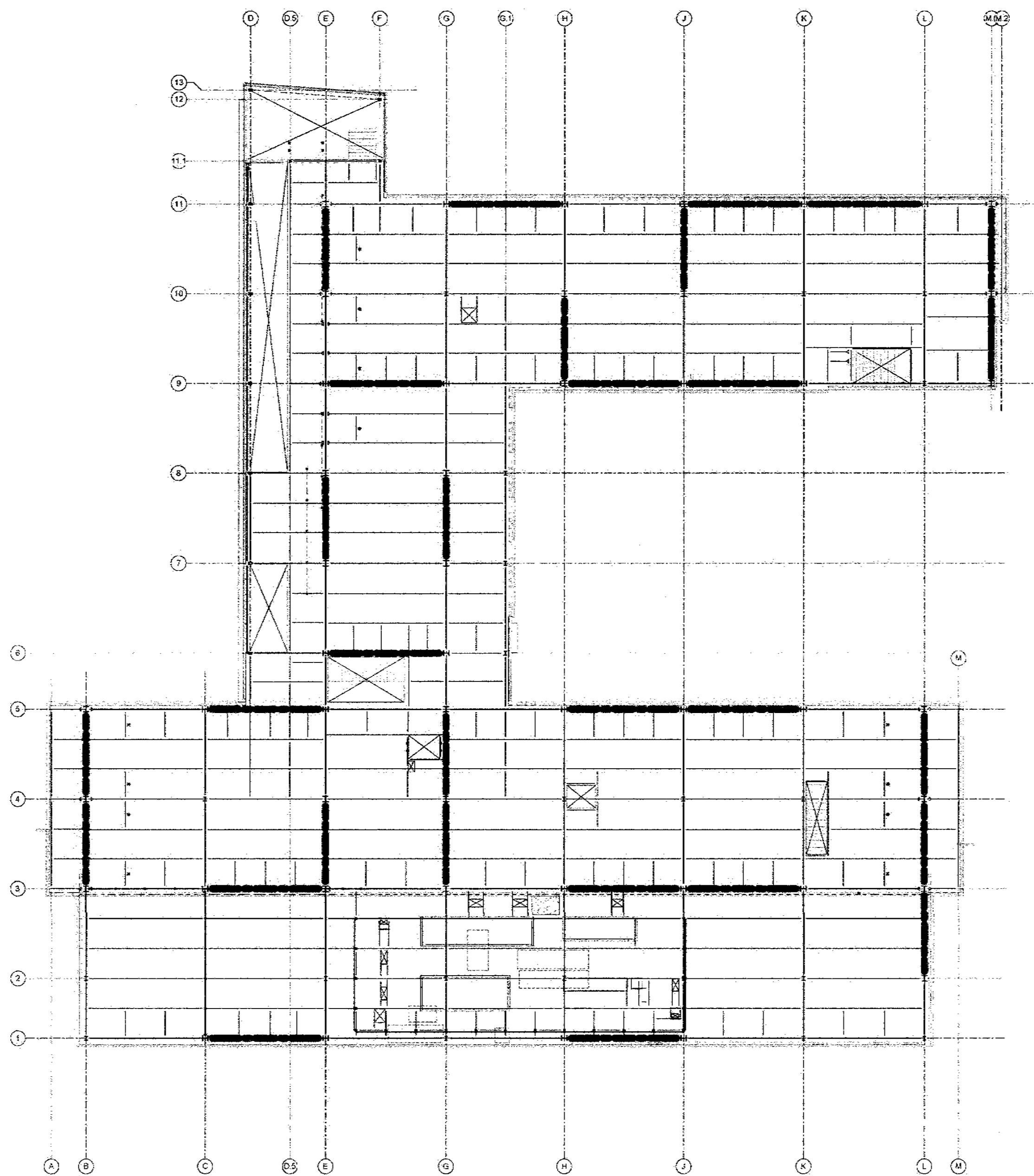
COLUMNS / BEAMS / CONNECTIONS

WELDING / STUDS

MEMBERS / SHEATHING / CONNECTIONS

SLRS NOTES:

1. THE SEISMIC LOAD RESISTING SYSTEM (SLRS) IS A STEEL SPECIAL MOMENT FRAME (SMF) DESIGNED TO MEET THE REQUIREMENTS OF AISC 341. ALL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THESE PROVISIONS.
2. THE SLRS SMFs ARE DESIGNATED ON THE PLAN BELOW BY HEAVY LINES.
3. WELDING OF THE SLRS SHALL CONFORM TO AISC 341, APPENDIX W.
4. DEMAND CRITICAL WELDS AS DEFINED IN AISC 341, SECTION 7.2c, SHALL MEET THE REQUIREMENTS OF AISC 341, SECTION 7.3b, APPENDIX W AND APPENDIX X.
5. IN ADDITION TO MEETING THE TESTING AND INSPECTION REQUIREMENTS OF THE 2010 CBC AS ADOPTED BY DSA-SSIC, THE WELDING AND BOLTING SHALL CONFORM TO THE QUALITY ASSURANCE PLAN OUTLINED IN AISC 341, APPENDIX Q.
6. THE PROTECTED ZONES, PER DETAIL 481.01.8, SHALL MEET THE REQUIREMENTS OF AISC 341, SECTION 7.4.

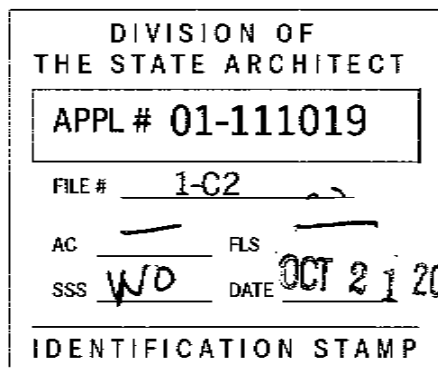


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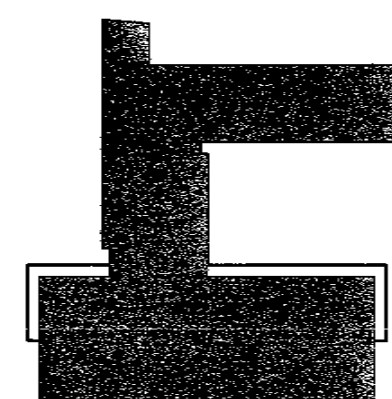
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San Jose, CA 95110

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Crosby Group Structural Engineering
2200 Bridge Parkway, Suite 104
Redwood City, CA 94065
(650) 367-0100
(650) 598-0277 (fax)



09 10.14.2010 DSA BACKCHECK
08 07.19.2010 100% DD SUBMITTAL TO DSA
06 06.03.2010 75% DD - DSA MEETING
05 05.20.2010 75% STRUCTURE - DSA COLLABORATIVE REVIEW
04 03.25.2010 100% DD SUBMITTAL TO LPC
03 03.11.2010 80% DD - DSA MEETING
02 12.12.2009 SD SUBMITTAL - DSA
01 11.19.2009 90% SD - DSA MEETING

REV NO DATE ISSUE



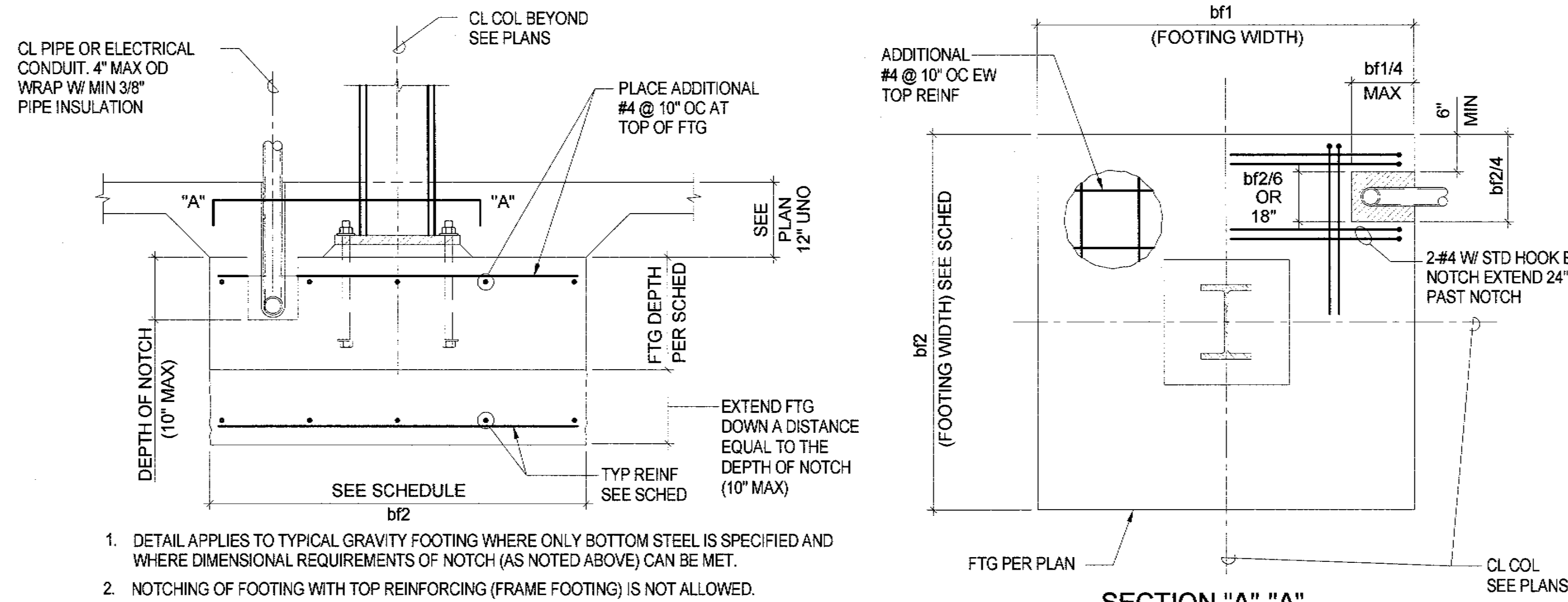
Student Services and
Administration
Building

Las Positas College
3200 Campus Hill Dr.,
Livermore, CA 94551

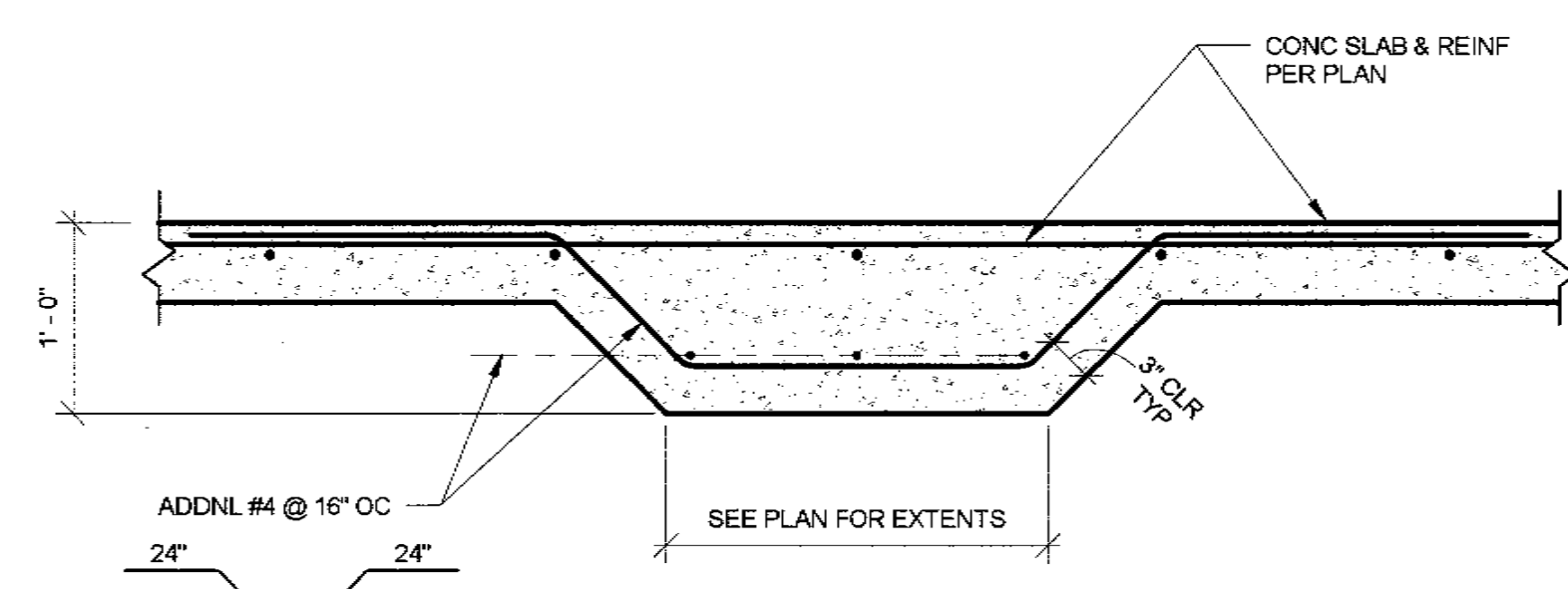
GENERAL NOTES

PROJECT # 09050.000
DATE: 2010 October 14
SCALE: As Indicated

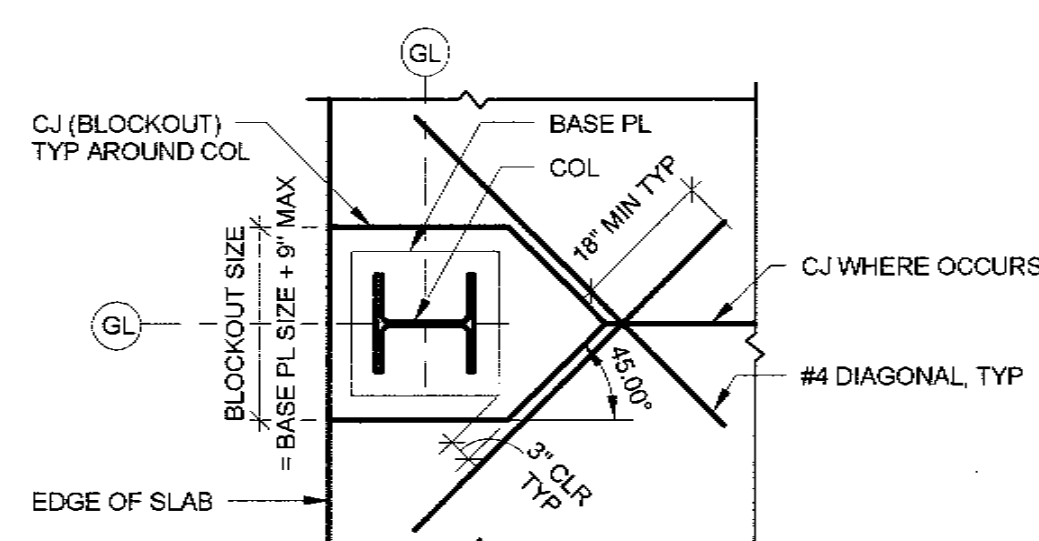
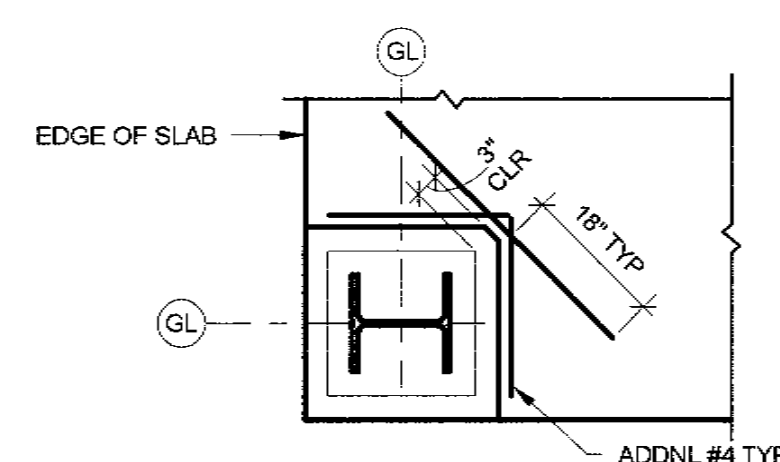
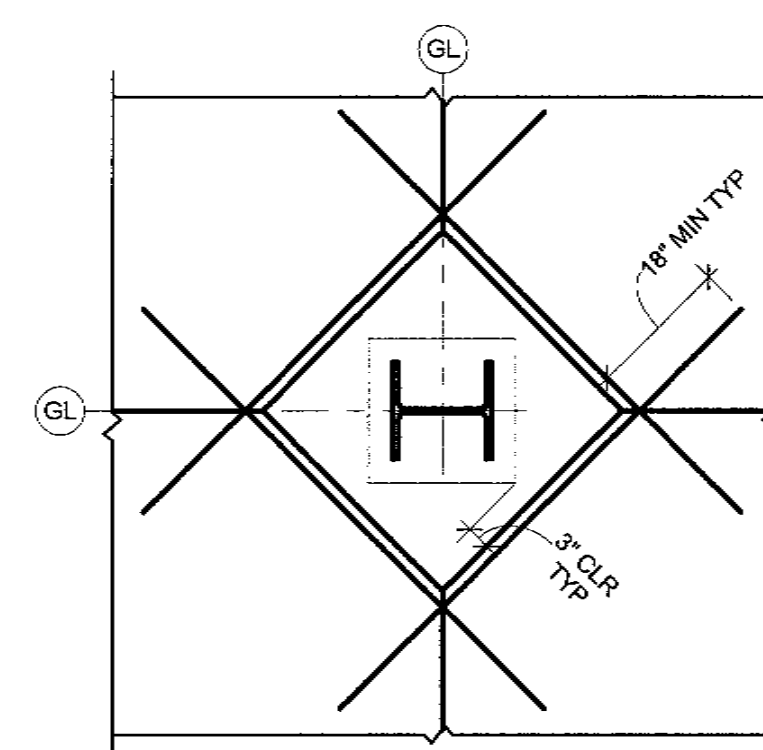
S0.02.2



13 TYP SECTION AT FTG NOTCH
NTS



14 THICKENED SLAB-INT
NTS



12 COLUMN BLOCKOUTS IN SOG
NTS

REINFORCEMENT DEVELOPMENT LENGTHS (14 OR CLASS A, INCHES) GR60 EXCEPT AS NOTED

REINFORCED CONCRETE		REINFORCEMENT SIZE									
REIN LOCATION	#3, GR40	#4	#5	#6	#7	#8	#9	#10	#11		
TOP	15	29	36	43	63	72	81	91	101		
OTHER	12	22	28	33	48	55	62	70	78		

REINFORCED CONCRETE		REINFORCEMENT SIZE									
REIN LOCATION	#3, GR40	#4	#5	#6	#7	#8	#9	#10	#11		
TOP	13	25	31	37	54	62	70	79	87		
OTHER	12	19	24	29	42	48	54	61	67		

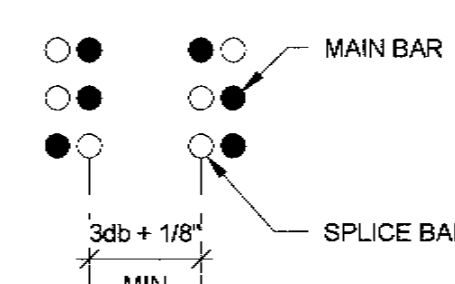
REINFORCEMENT LAP SPlice LENGTHS (CLASS B, INCHES) GR60 EXCEPT AS NOTED

REINFORCED CONCRETE		REINFORCEMENT SIZE									
REIN LOCATION	#3, GR40	#4	#5	#6	#7	#8	#9	#10	#11		
TOP	20	38	47	56	82	94	106	119	132		
OTHER	16	29	37	43	63	72	81	91	102		

REINFORCED CONCRETE		REINFORCEMENT SIZE									
REIN LOCATION	#3, GR40	#4	#5	#6	#7	#8	#9	#10	#11		
TOP	17	33	41	49	71	81	91	103	114		
OTHER	16	25	32	38	55	63	71	80	88		

NOTES:

TOP REINFORCING IS HORIZONTAL REINFORCEMENT THAT HAS MORE THAN TWELVE INCHES OF CONCRETE PLACED BELOW IT.
USE WELDED SPlice OR MECHANICAL CONNECTOR IF THE LAP SPlice REINFORCEMENT DOES NOT HAVE A MINIMUM SPACING $3db + 1/8$ INCHES AND A MINIMUM COVER OF $db + 1/8$ INCHES.



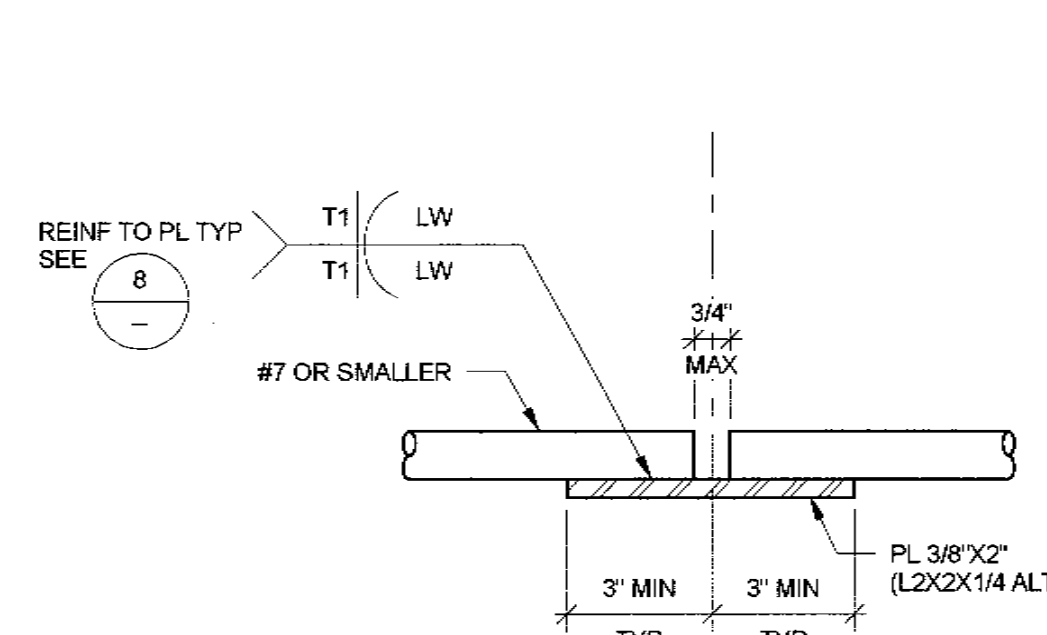
WELDED SPlice (DIRECT BUTT SPlices-FULL PENETRATION)

#5 AND SMALLER, SINGLE BEVEL GROOVE WELD
#6 AND LARGER, DOUBLE BEVEL GROOVE WELD
WELDS SHALL HAVE THE ABILITY TO DEVELOP 125 PERCENT OF THE YIELD STRENGTH FOR THE SPliced REINFORCEMENT (75 KSI).

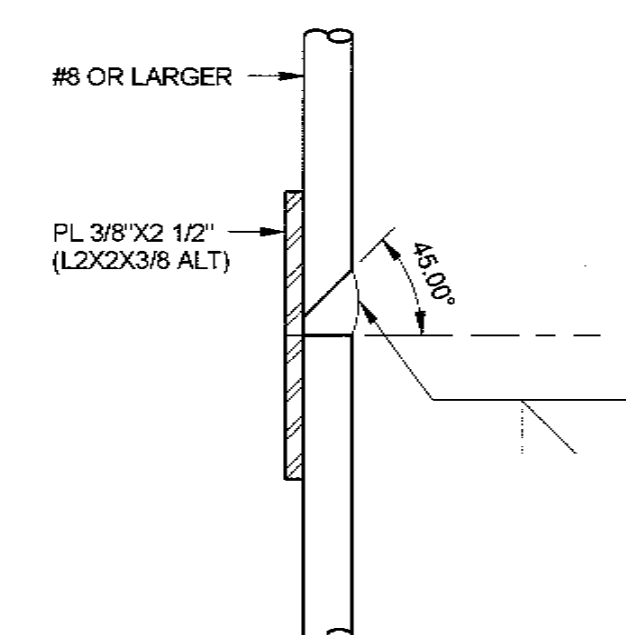
MECHANICAL CONNECTORS

MECHANICAL CONNECTORS SHALL BE TENSION-COMPRESSION TYPE WITH THE ABILITY TO DEVELOP 125% OF THE YIELD STRENGTH FOR THE SPliced REINFORCEMENT (75 KSI). ALSO, MECHANICAL CONNECTORS SHALL HAVE AND BE INSTALLED PER AN ICC EVALUATION REPORT.

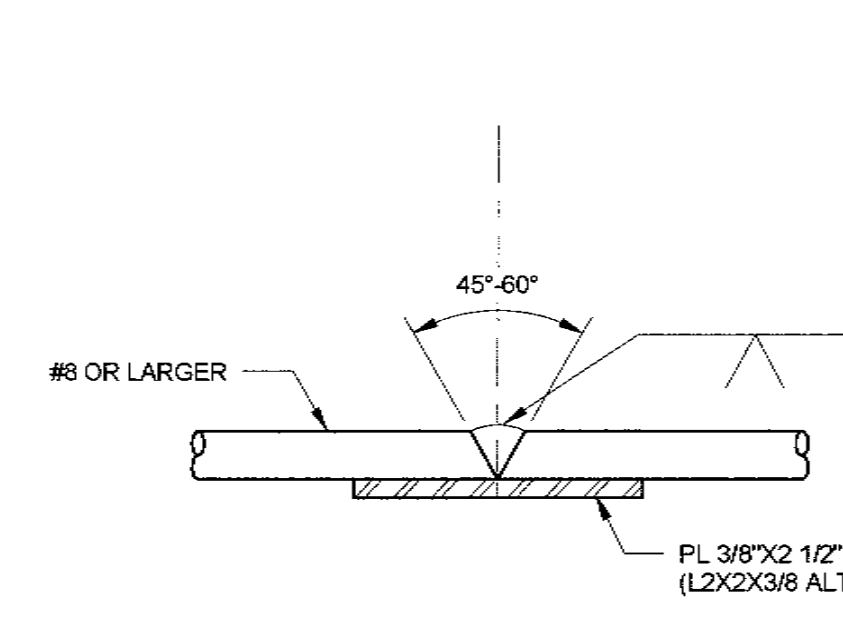
6 REINFORCEMENT DEVELOPMENT LENGTHS & LAP SPICES
NTS



#7 REIN OR SMALLER
(VERTICAL OR HORIZONTAL REIN)



#8 REIN OR LARGER
(VERTICAL REIN)

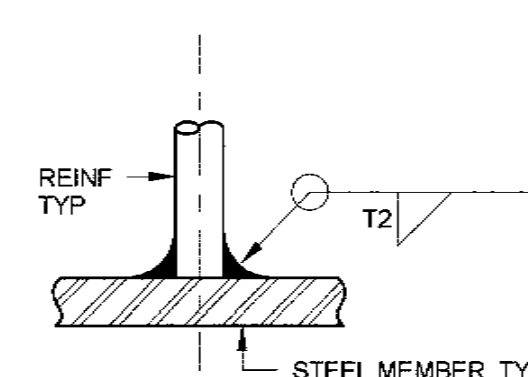


#8 REIN OR LARGER
(HORIZONTAL REIN)

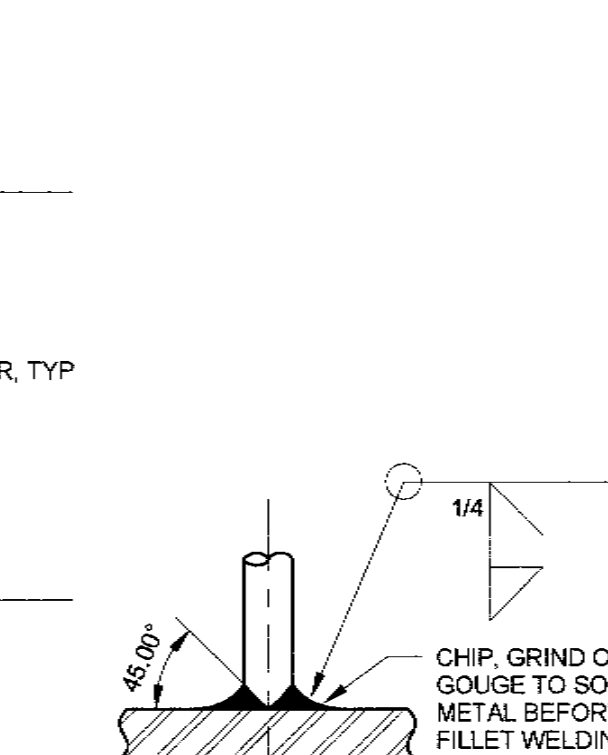
NOTES:

- ALL WELDED REIN SHALL BE PER ASTM A706.
- ALL WELDING OF REIN TO COMPLY WITH AWS D1.4.

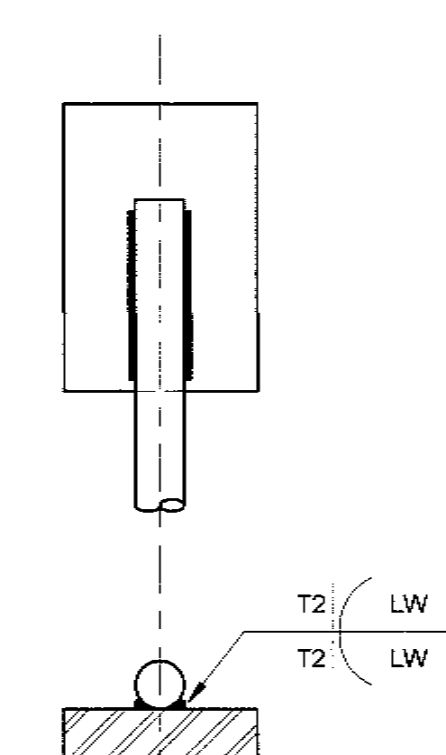
7 WELDED SPlice IN REIN
NTS



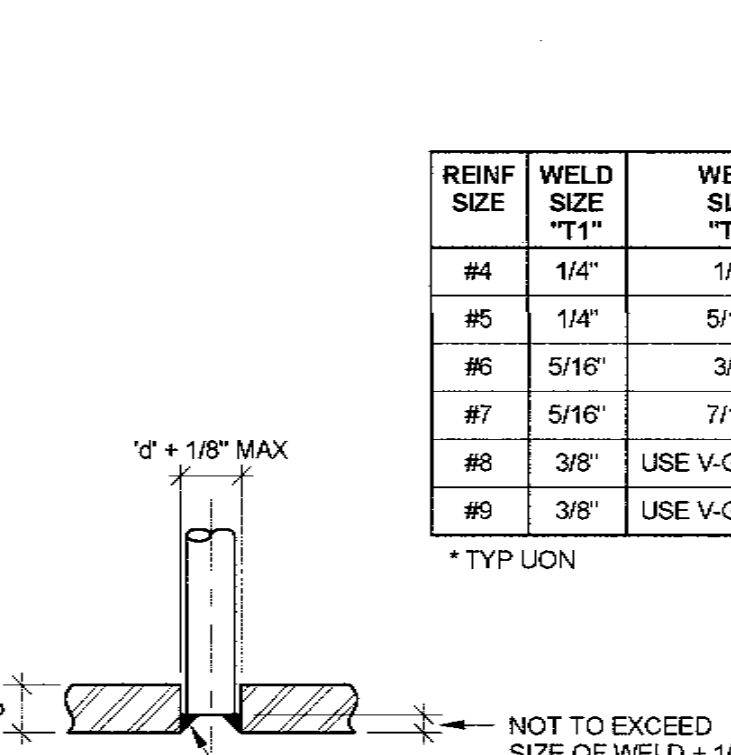
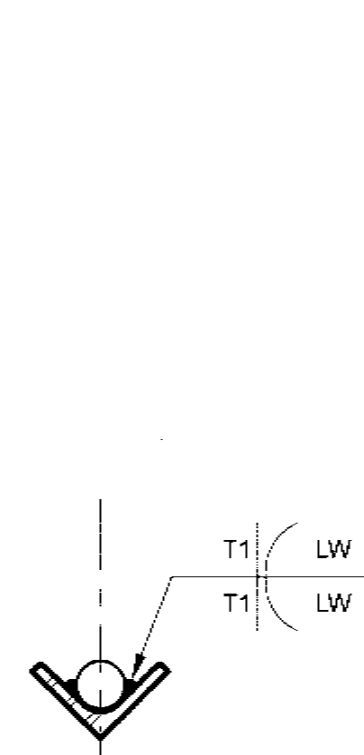
A-EXTERNAL FILLET
WELDS



B-COMplete JOINT
PENETRATION GROOVE WELD



C-FLARE BEVEL GROOVE
WELD



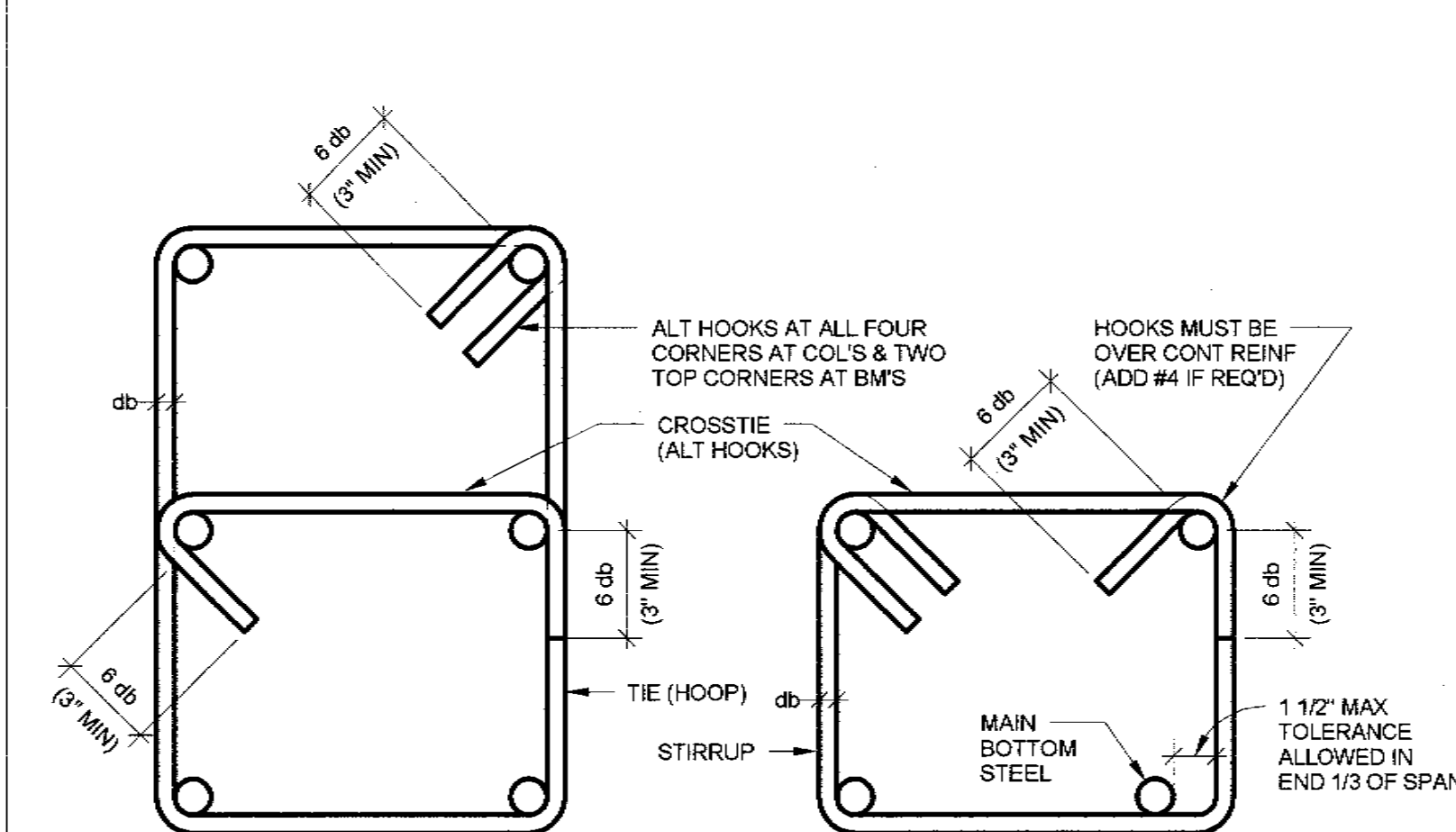
D-INTERNAL FILLET
WELDS

REIN SIZE	WELD SIZE "T1"	WELD SIZE "T2"	LENGTH "LW"
#4	1/4"	1/4"	3 1/2"
#5	1/4"	5/16"	3 1/2"
#6	5/16"	3/8"	4"
#7	5/16"	7/16"	4"
#8	3/8"	USE V-GROOVE	4 1/2"
#9	3/8"	USE V-GROOVE	4 1/2"

* TYP UCN

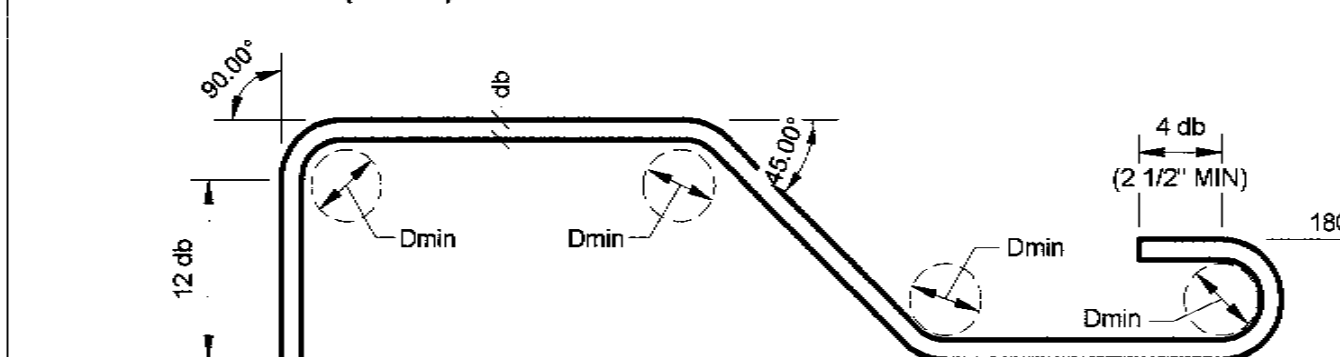
8 REIN WELDED TO STEEL
NTS

2 REIN HOOKS & BENDS
NTS



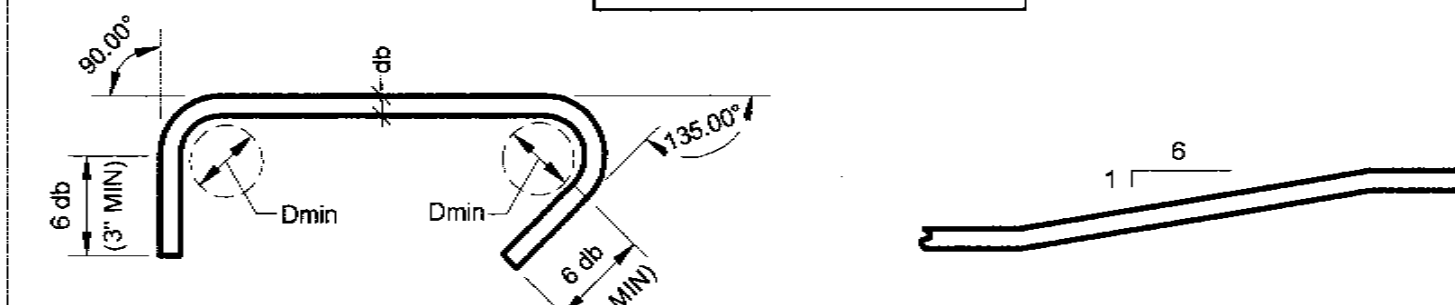
COLUMN & BEAM TIE
(HOOP)

BEAM STIRRUP



PRINCIPAL REIN

Dmin = 6 db FOR #3 THRU #8
Dmin = 6 db FOR #9 THRU #11
Dmin = 10 db FOR #14 THRU #18



CROSSTIES & STIRRUPS

Dmin = 1 1/2" FOR #3
Dmin = 2" FOR #4
Dmin = 2 1/2" FOR #5

MAX OFFSET BEND

NOTES:

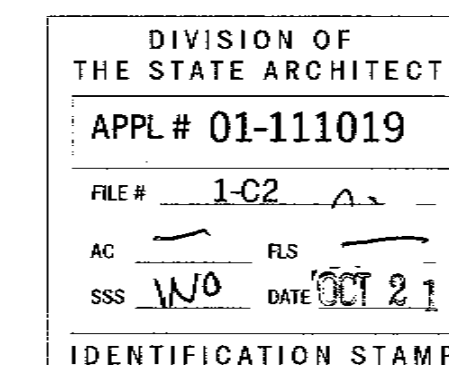
- ALL BENDS SHALL BE MADE COLD.

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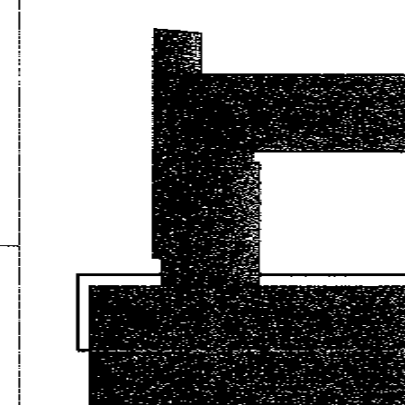
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Pleasanton, CA 94588

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Steinberg Architects
60 Pierce Avenue
San Jose, CA 95110

CONSULTANT DISCIPLINE
Crosby Group Structural Engineering
2200 Bridge Parkway, Suite 104
Redwood City, CA 94065
(650) 361-6100
(650) 598-0277 (fax)



REV	NO	DATE	ISSUE
09	10.14.2010	DSA BACKCHECK	
05	07.19.2010	100% CD SUBMITTAL TO DSA	
06	06.03.2010	75% CD - DSA MEETING	
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW	
04	03.25.2010	100% DD SUBMITTAL TO LPC	
03	03.11.2010	80% DD - DSA MEETING	
02	12.12.2009	SD SUBMITTAL - DSA	
01	11.19.2009	90% SD - DSA MEETING	



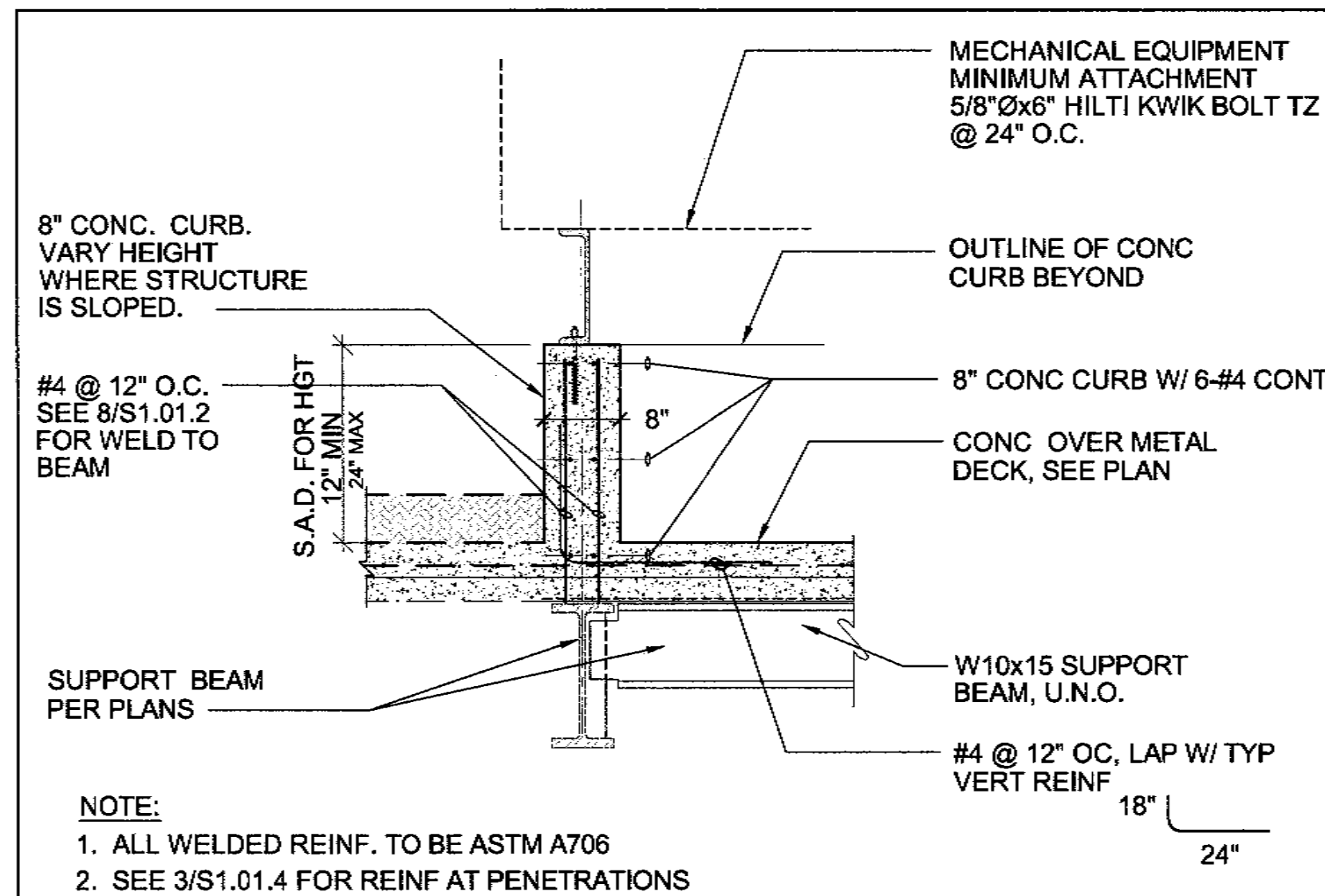
Student Services and Administration Building

Las Positas College

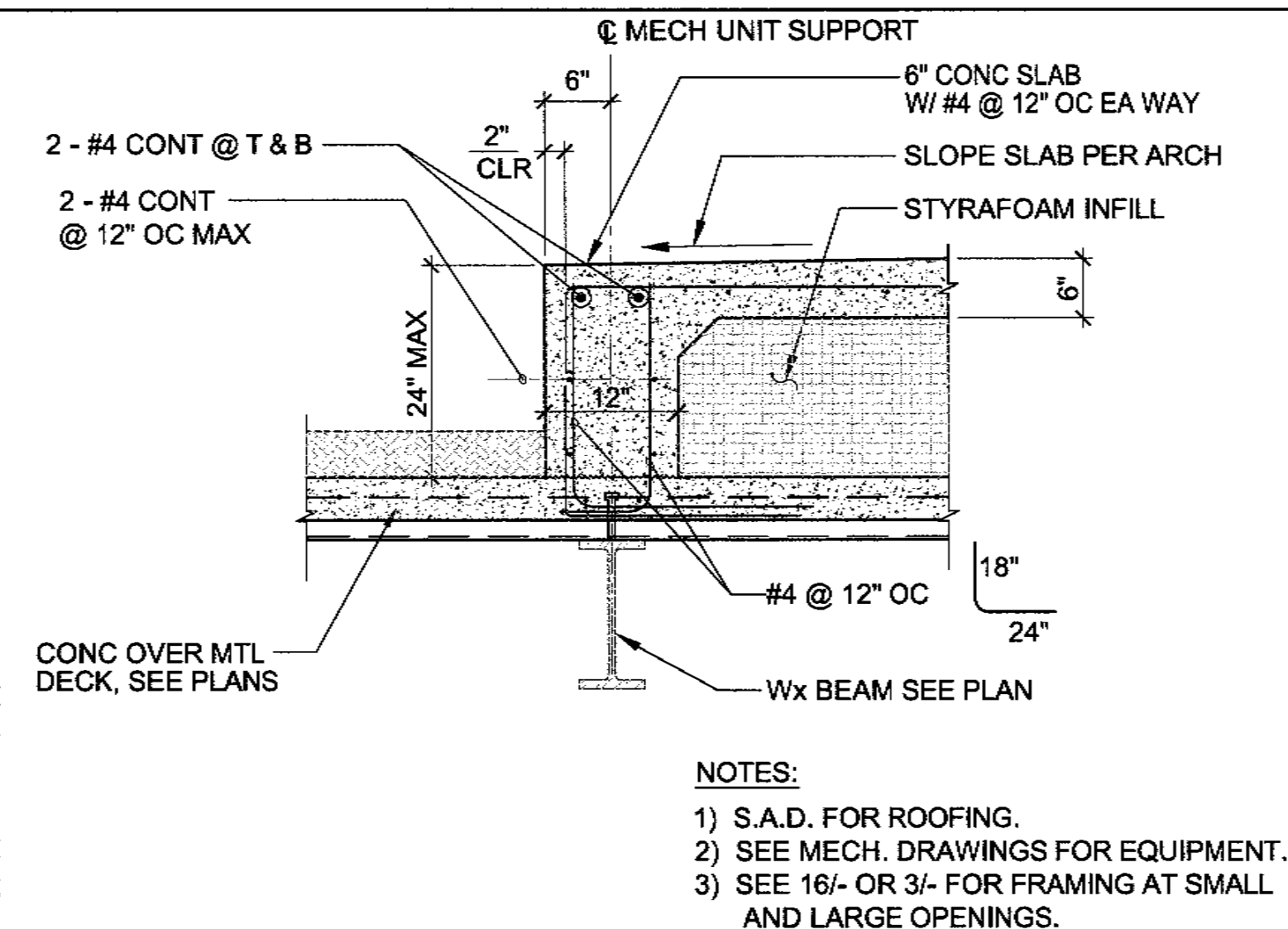
3000 Campus Hill Dr.,
Livermore, CA 94551

TYPICAL CONCRETE DETAILS

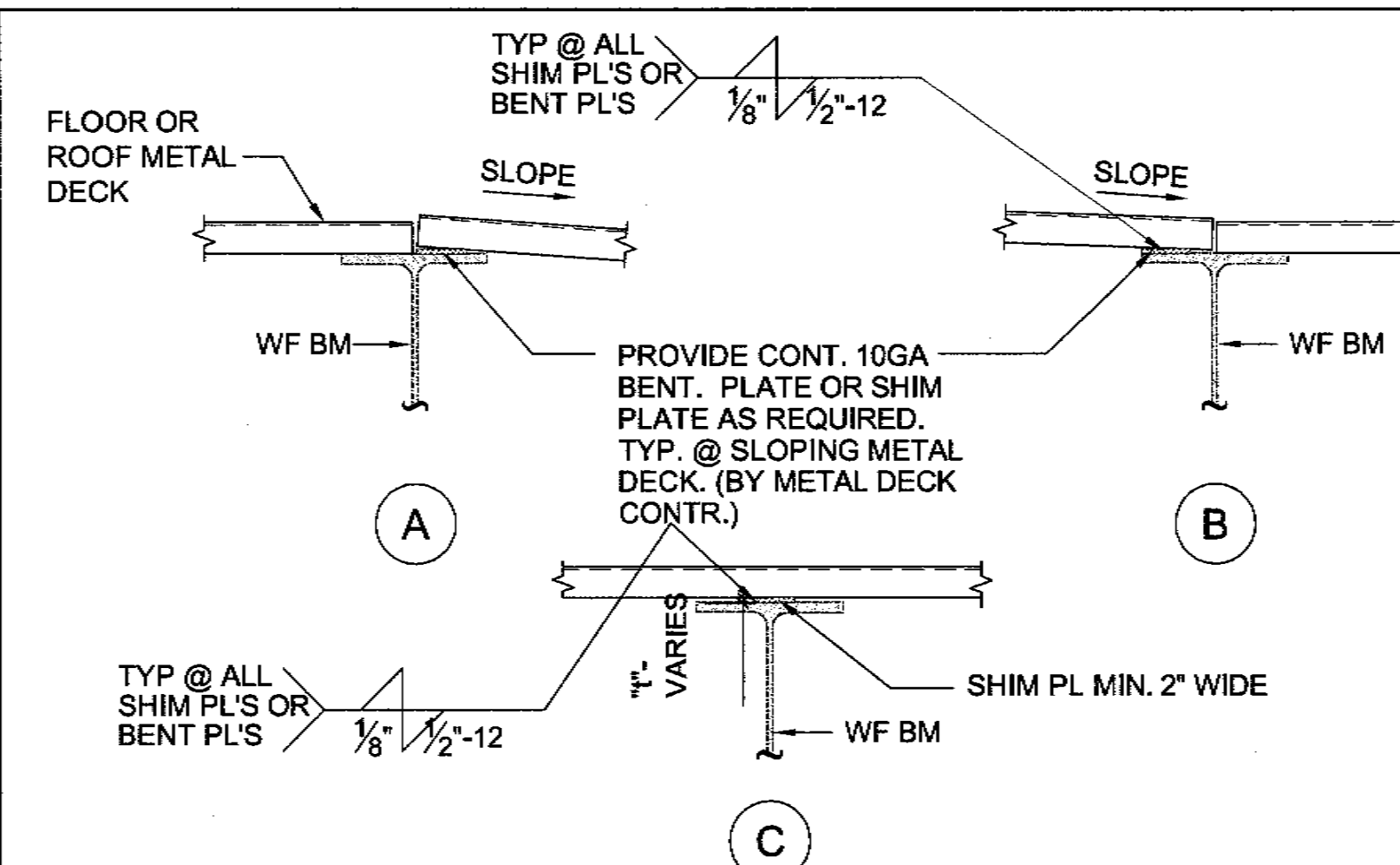
PROJECT # 08060.000
DATE: 2010 October 14
SCALE: As indicated



13 MECH. UNIT SUPPORT @ ROOF
N.T.S. (CURB SUPPORT)

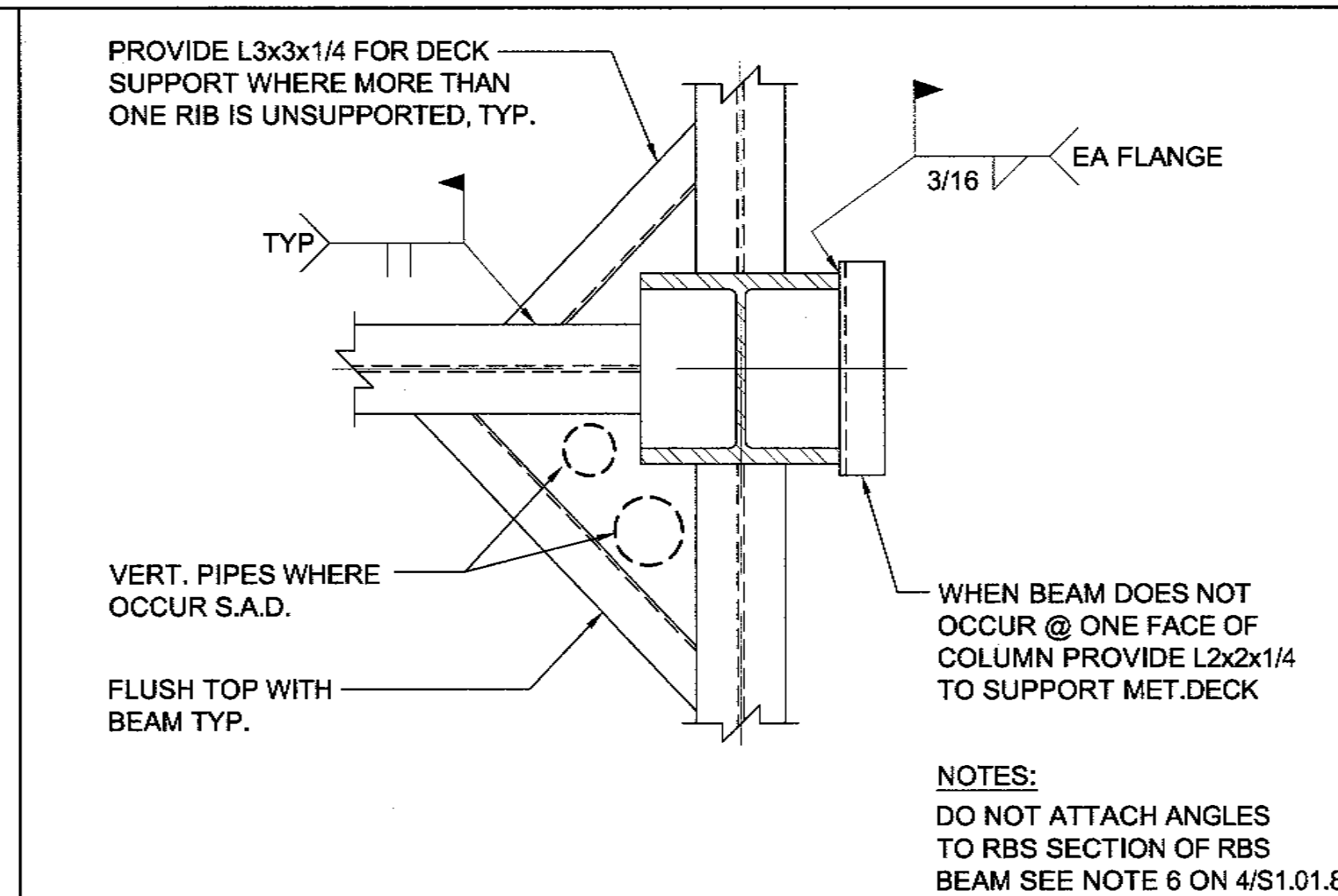


9 MECH. UNIT SUPPORT @ ROOF
N.T.S. (PAD SUPPORT)

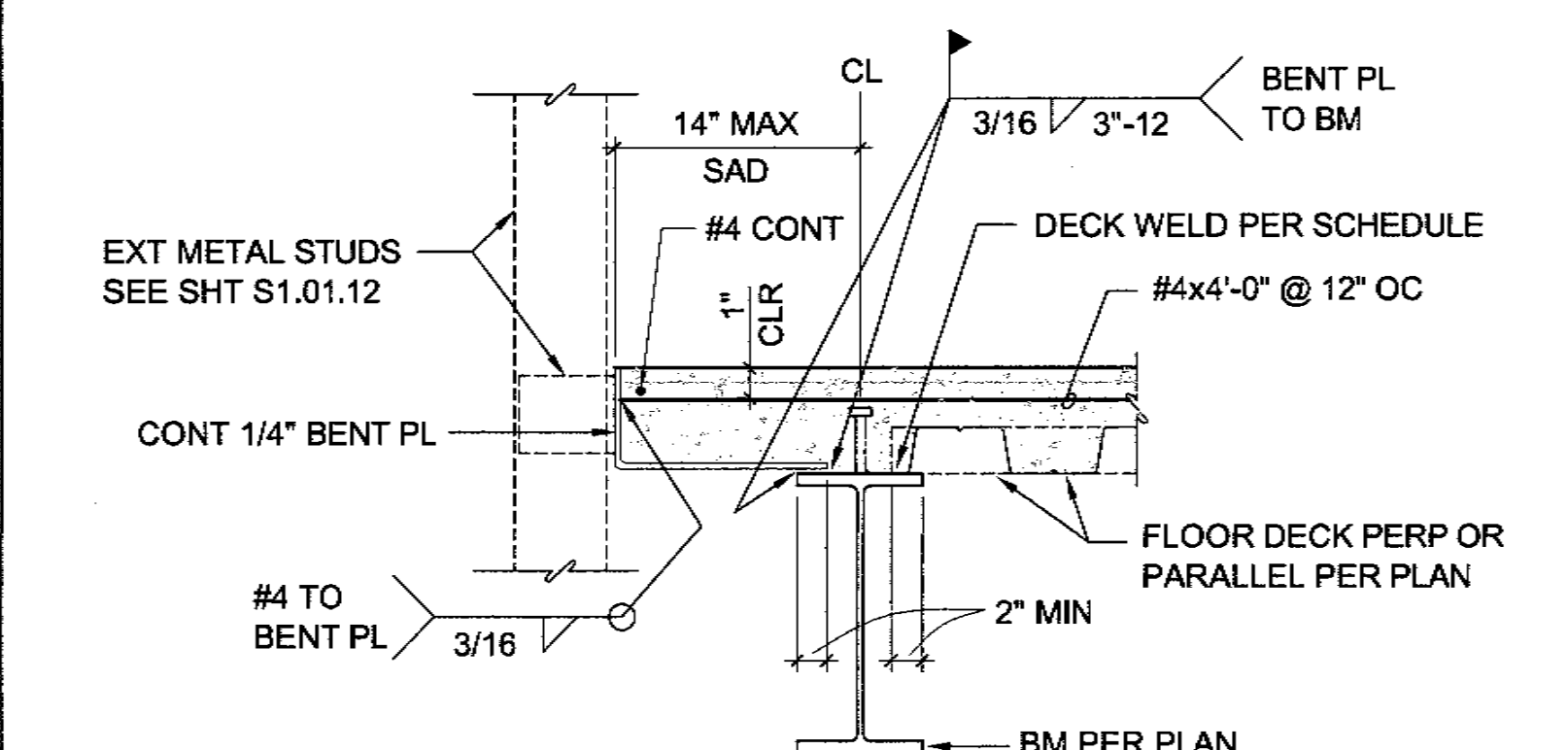


NOTES:
1. FOR BEAM SIZES SEE PLANS (TYP)

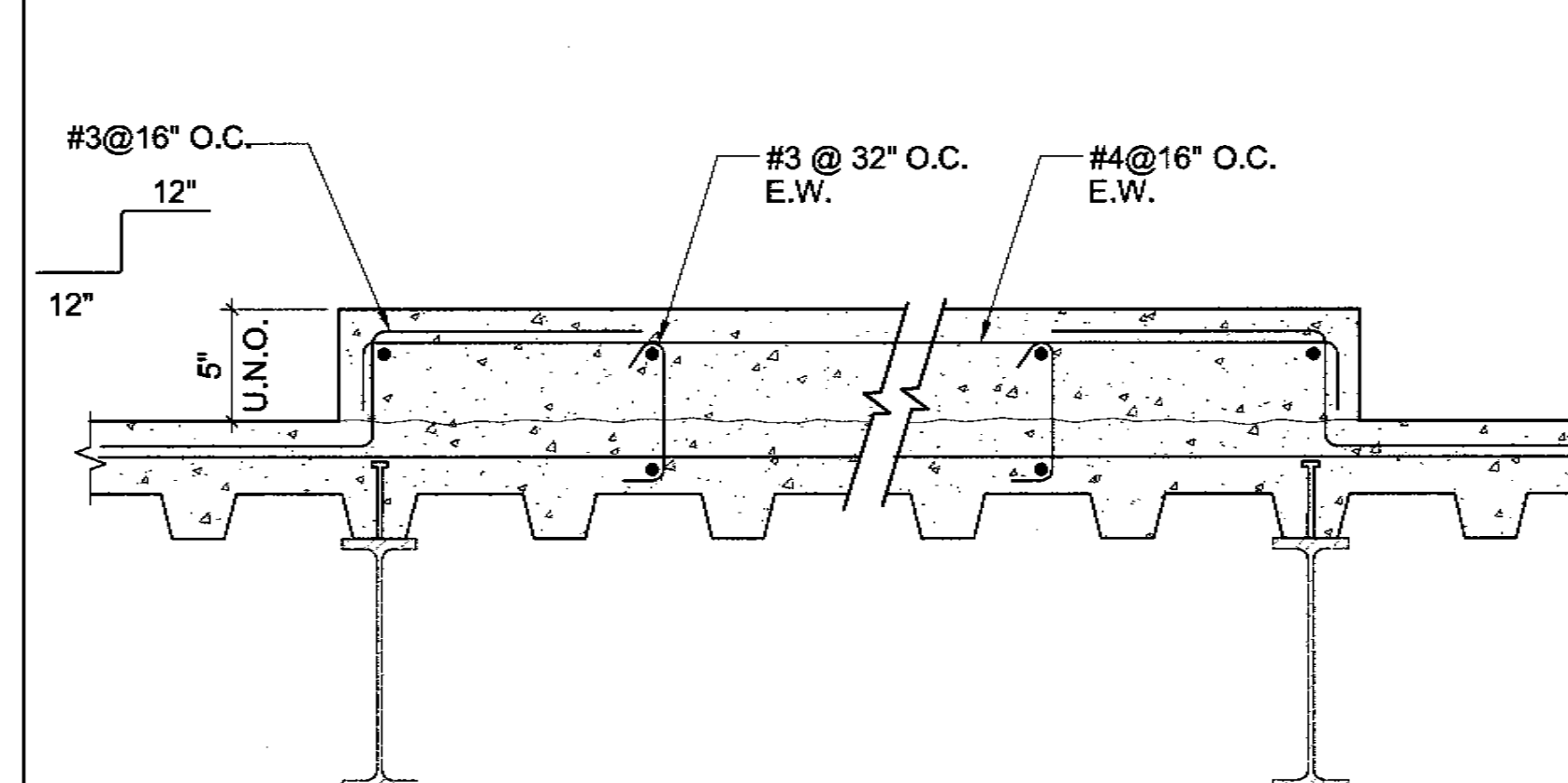
5 TYPICAL SHIM PLATE DETAIL
N.T.S.



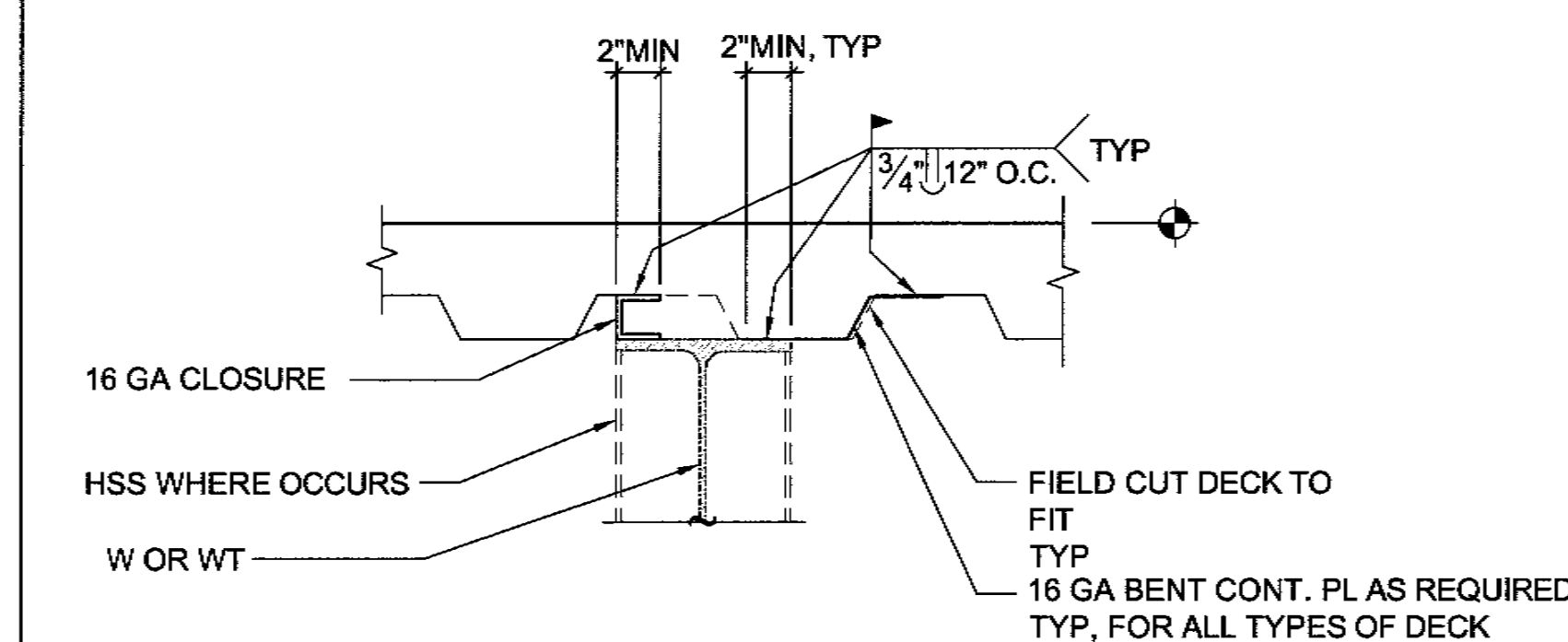
1 METAL DECK SUPPORT @ COLUMN
N.T.S.



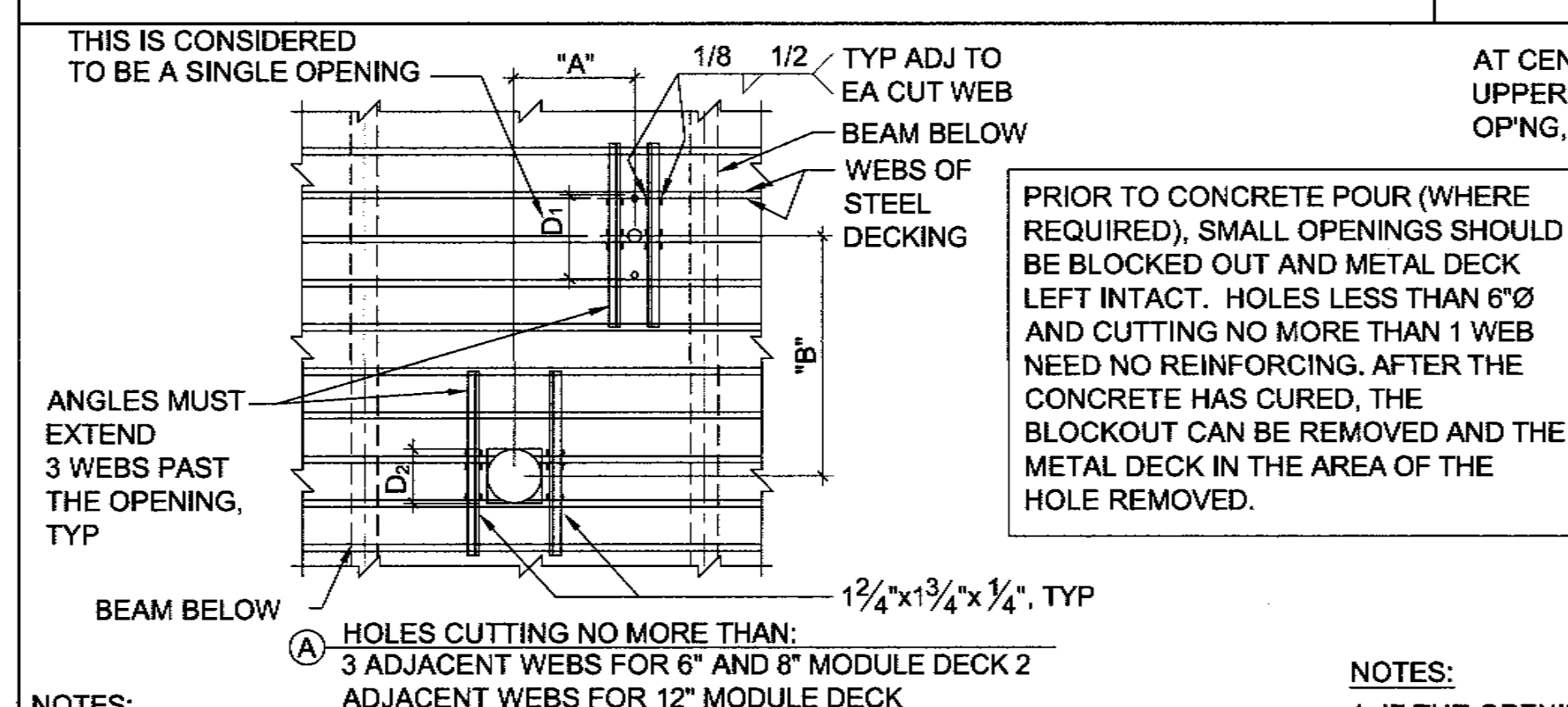
6 EDGE OF DECK AT EXT METAL STUD WALLS
N.T.S.



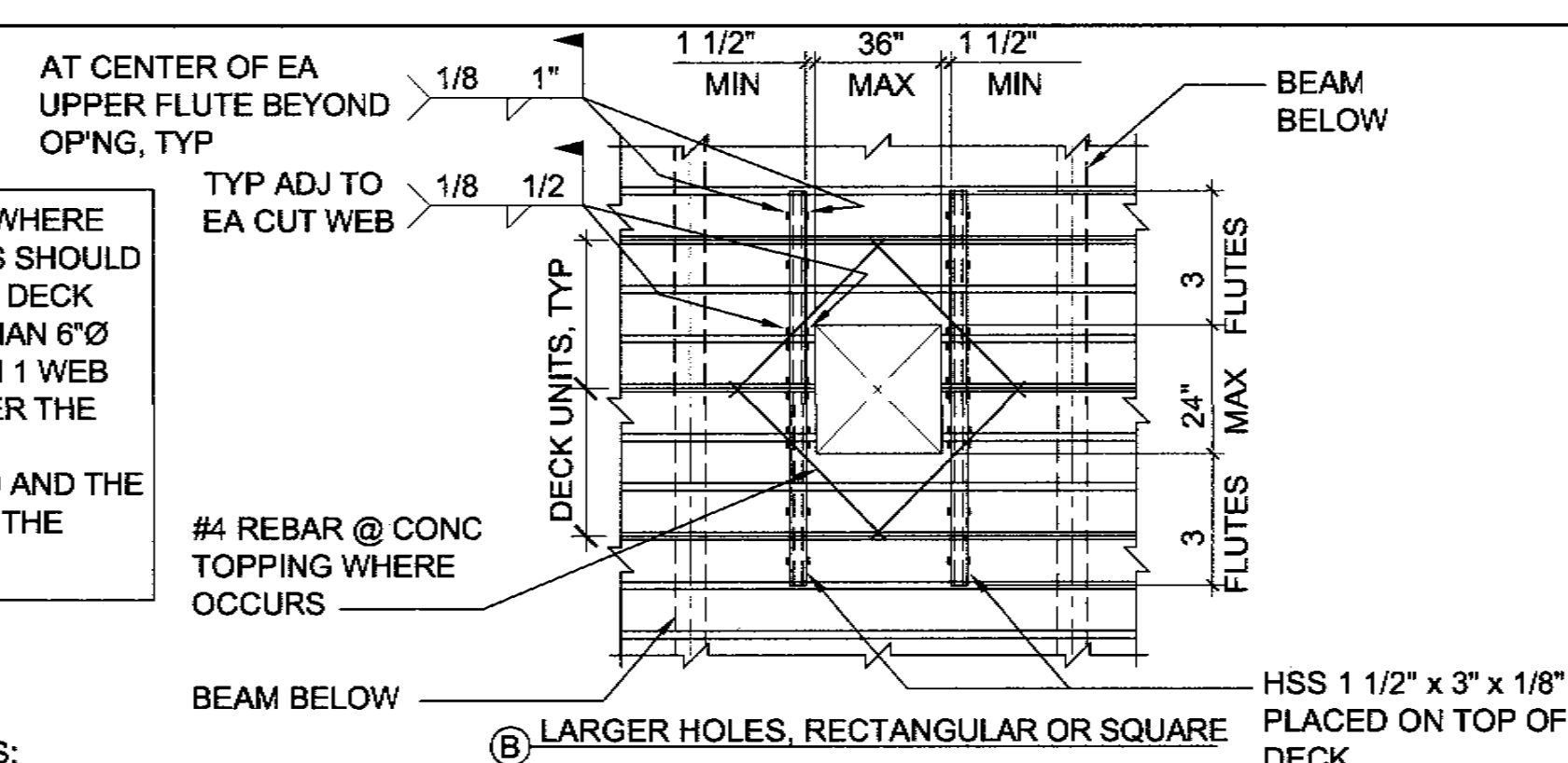
2 SECTION @ RAISED MECH. PAD
N.T.S.



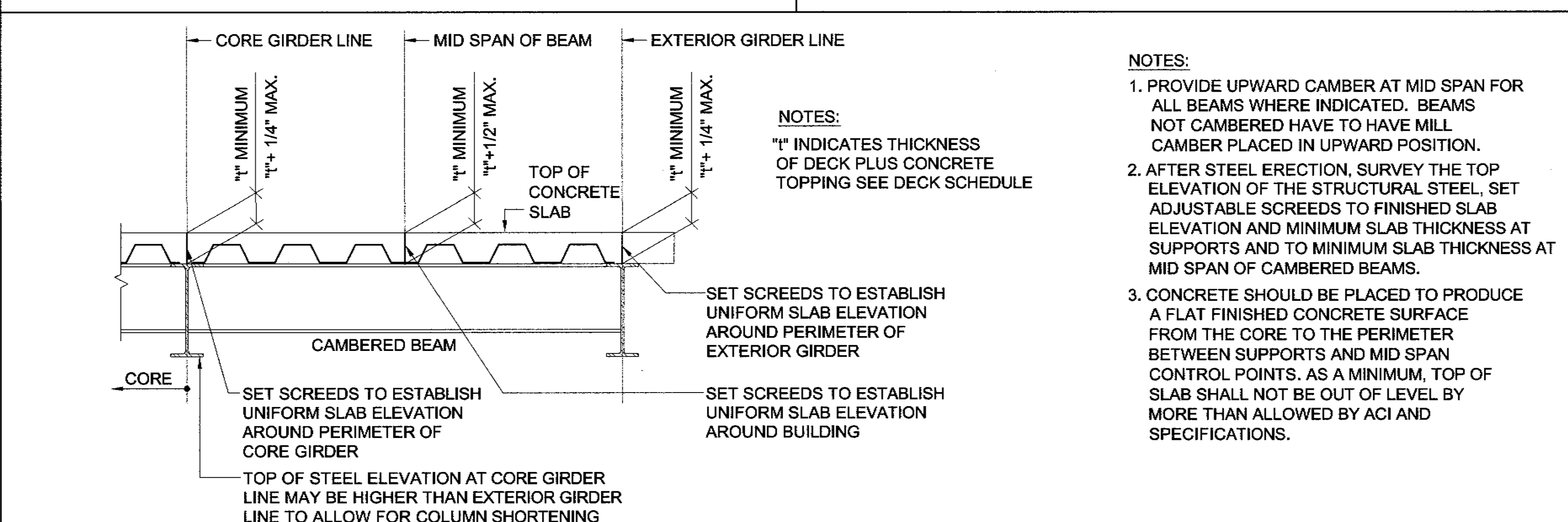
15 TYPICAL DECK SPLICE AT SUPPORT



16 **SMALL MECHANICAL UNIT OR DECK OPENING WITH OR WITHOUT TOPPING (MAX. WT. = 200 LBS)**
N.T.S.



8 **PROCEDURE FOR CONSTRUCTION OF SLAB ON CAMBERED BEAMS**



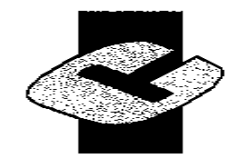
3 FRAMING AT SMALL MECH. EQUIPMENT
& LARGE OPENINGS N.T.S. MAX. EQUIP. WT=1000#

Steinberg Architects

CLIENT
Chabot-Las Positas Community College District
5020 Franklin Dr.,
Pleasanton, CA 94588

ARCHITECT
Steinberg Architects
60 Pierce Avenue
San Jose, CA 95110

STRUCTURAL
Crosby Group Structural Engineering
2200 Bridge Parkway, Suite 104
Redwood City, CA 94065
(650) 367-8100
(650) 598-0277 (fax)



Crosby
group

DIVISION OF
THE STATE ARCHITECT

APPL # **01-111019**

FILE # 1-C2

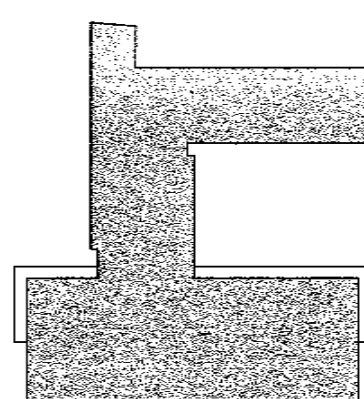
AC FLS

SSS WS DATE OCT 2 1 2001

IDENTIFICATION STAMP



09	10.14.2010	DSA BACKCHECK
08	07.19.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD - DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% DD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING



**Student Services and
Administration
Building**

Las Positas College
3000 Campus Hill Dr.
Livermore, CA 94551

TYPICAL STEEL DETAILS

PROJECT #: 09050.000
DATE: 2010.October.14
SCALE: AS NOTED

S1.01.4

BEAM CONNECTION SCHEDULE				
BEAM DESIGNATION	PLATE "tp"	BOLTS	WELD "E"	REMARKS
C6, W6	1/4"	2 - 3/4" DIA	3/16"	
C8-C10, W8-W10	3/8"	2 - 3/4" DIA	1/4"	
W12, W14	3/8"	3 - 3/4" DIA	1/4"	
W16	3/8"	4 - 3/4" DIA	1/4"	
W18, W21	3/8"	5 - 3/4" DIA	1/4"	
W24	7/16"	6 - 3/4" DIA	5/16"	
W27	7/16"	7 - 3/4" DIA	5/16"	
W30	7/16"	8 - 3/4" DIA	5/16"	
W33, W36	7/16"	9 - 3/4" DIA	5/16"	

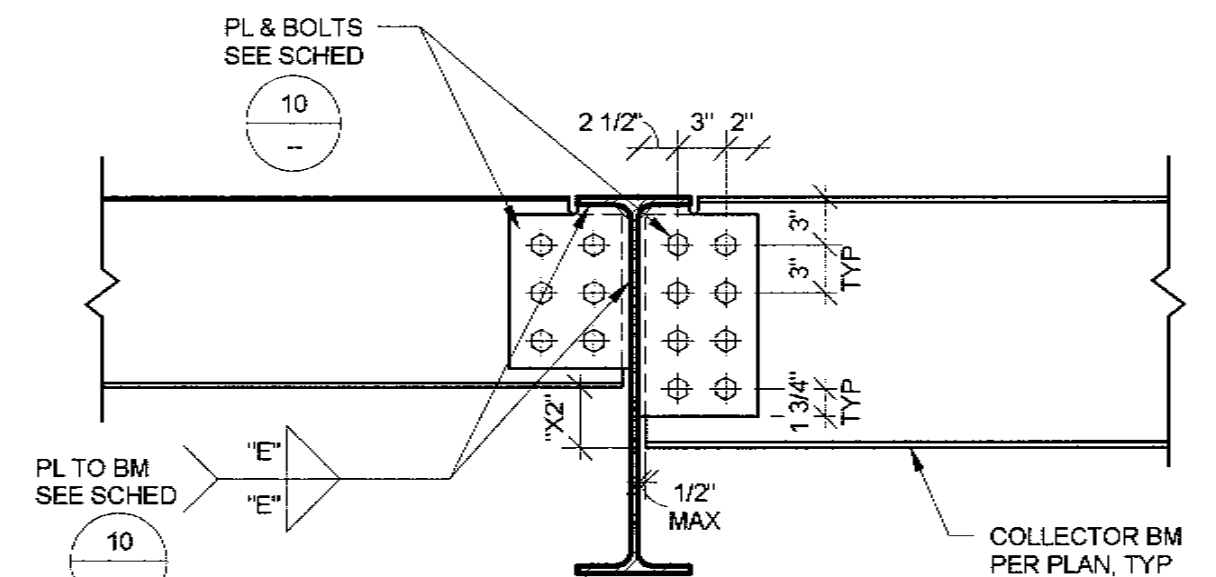
NOTES:
1. USE A325-N BOLTS.
2. USE PLATE - Fy = 36 KSI, Fu = 58 KSI.
3. USE A325-SC BOLTS WHERE INDICATED.

9 BEAM CONNECTION SCHEDULE
NTS

COLLECTOR BEAM CONNECTION SCHEDULE				
BEAM DESIGNATION	PLATE "tp"	BOLTS	WELD "E"	REMARKS
W12, W14	5/8"	2 x 3 - 1" DIA = 6 BOLTS	3/8"	
W16	5/8"	2 x 4 - 1" DIA = 8 BOLTS	3/8"	
W18, W21	3/4"	2 x 5 - 1" DIA = 10 BOLTS	3/8"	
W24	3/4"	2 x 6 - 1" DIA = 12 BOLTS	3/8"	
W27	3/4"	2 x 7 - 1" DIA = 14 BOLTS	3/8"	
W30	3/4"	2 x 8 - 1" DIA = 16 BOLTS	3/8"	
W33, W36	3/4"	2 x 9 - 1" DIA = 18 BOLTS	3/8"	

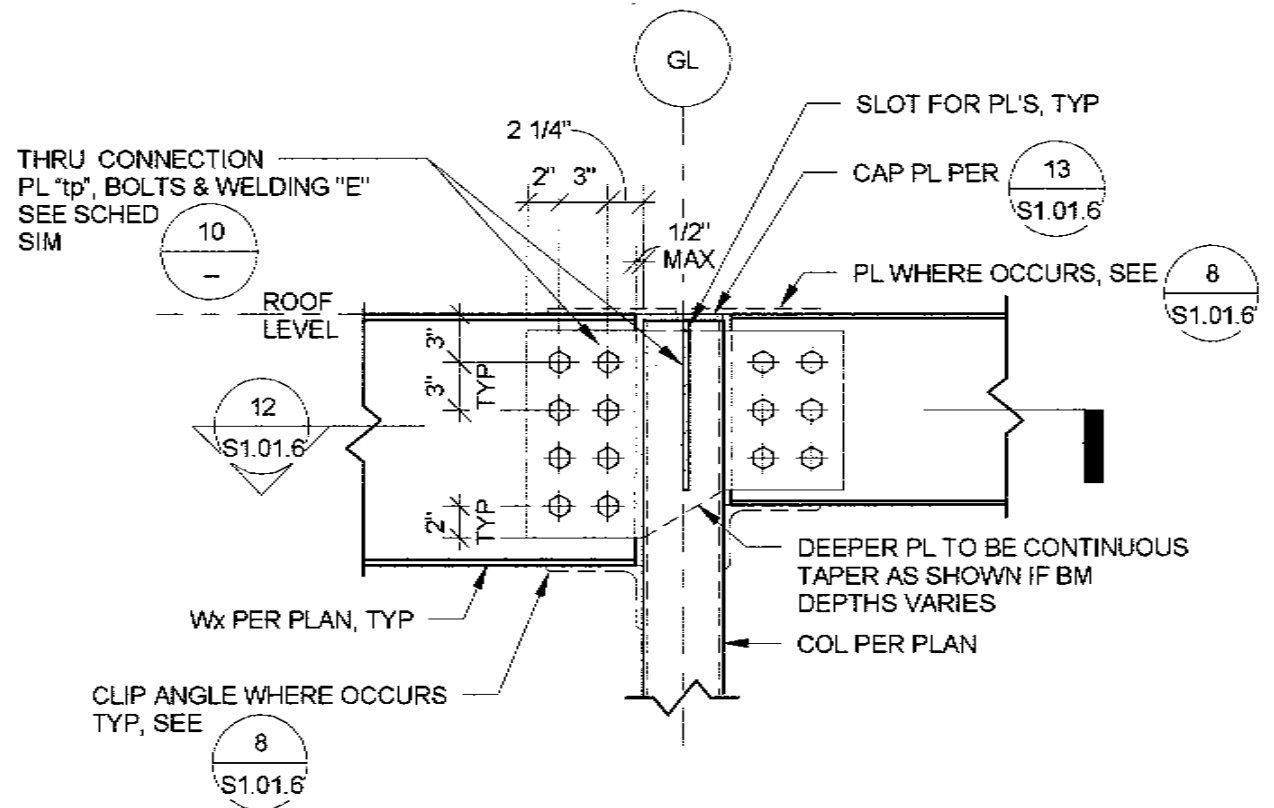
NOTES:
1. USE A490-SC BOLTS.
2. USE PLATE - Fy = 50 KSI, Fu = 65 KSI.

10 COLLECTOR BEAM CONNECTION SCHEDULE
NTS

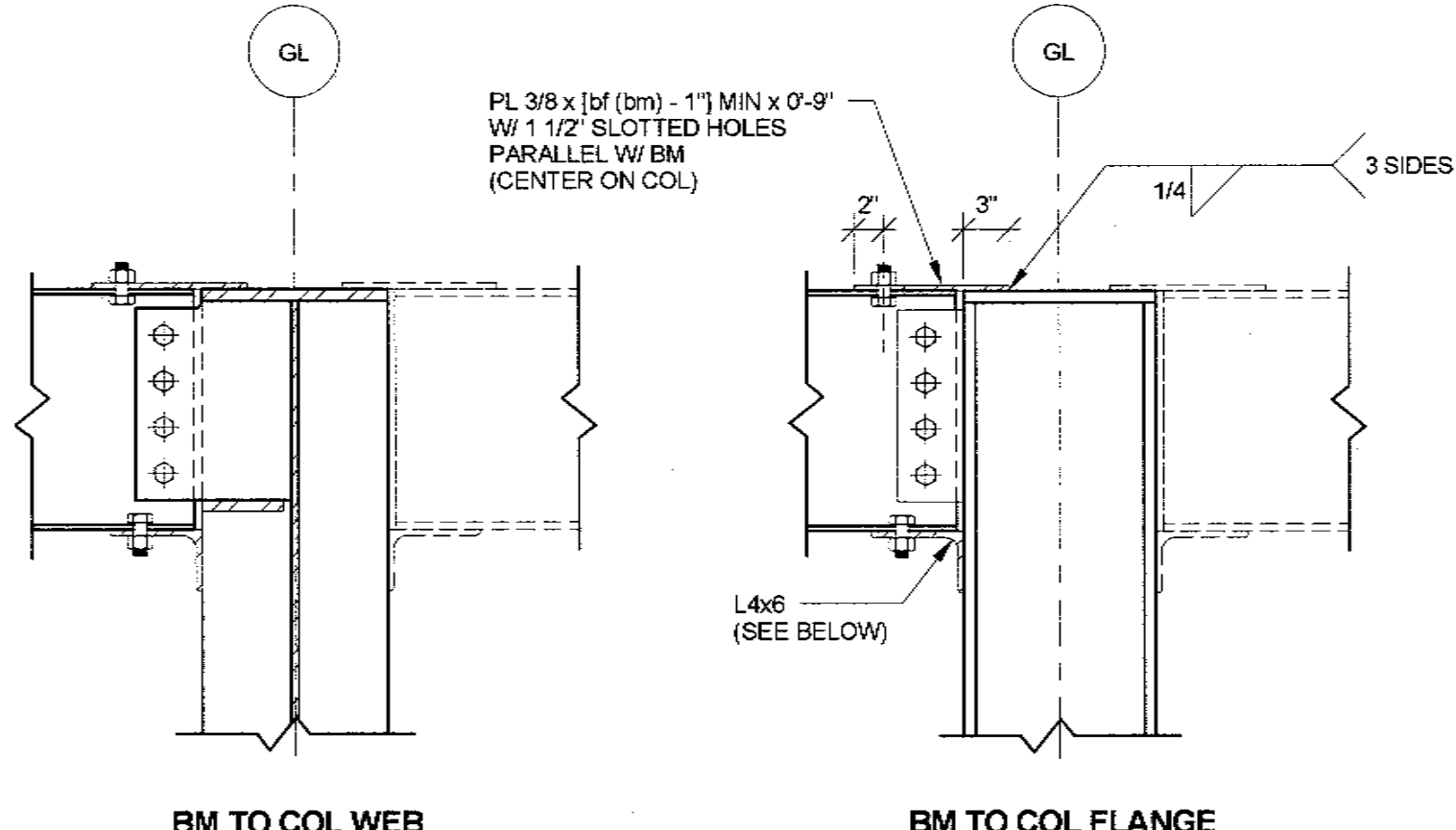


NOTES:
1. "X" > 6", EXTEND PLATE AS SHOWN IN DETAIL B.
2. DO NOT COPE FLANGES OF COLLECTOR BEAMS UNLESS SPECIFICALLY SHOWN.

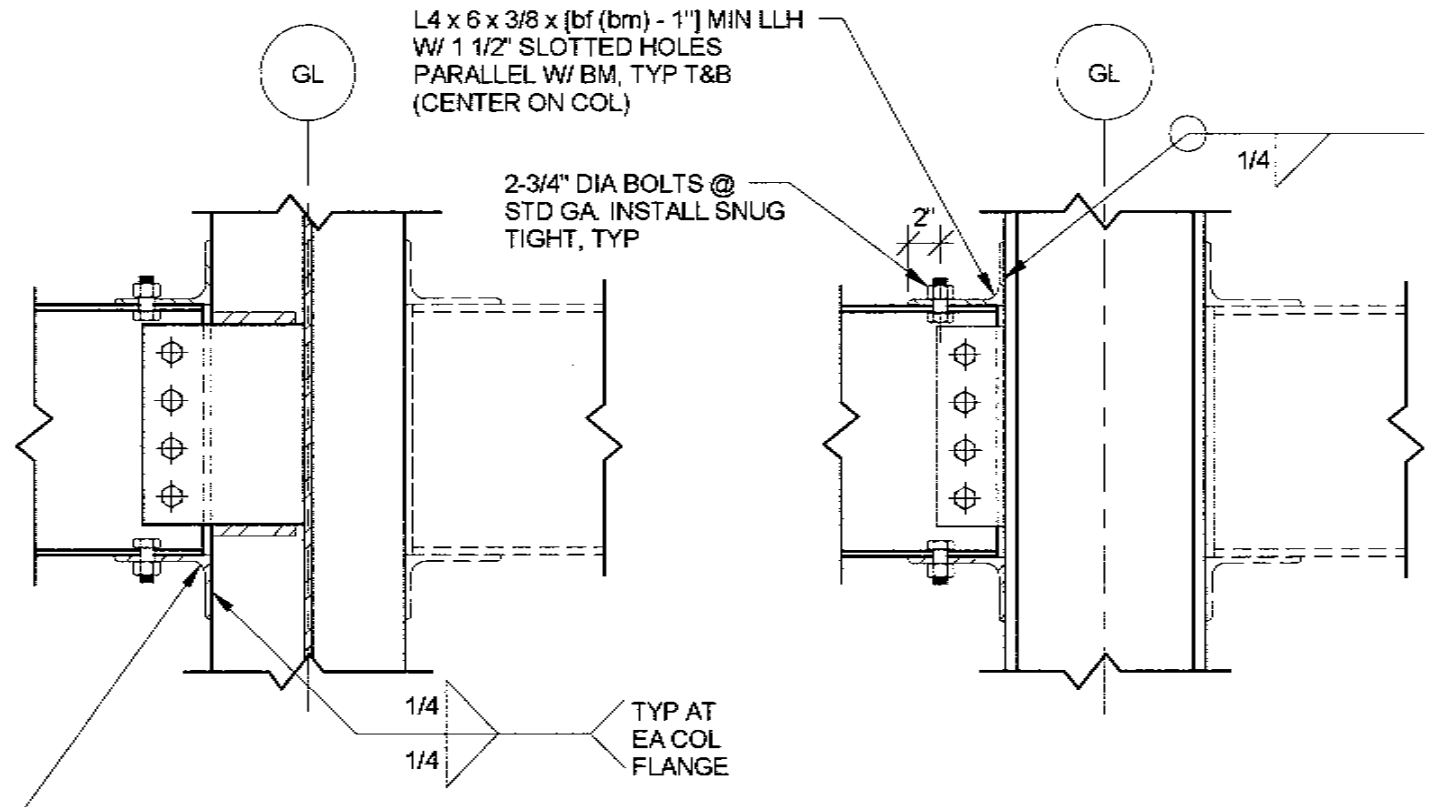
11 COLLECTOR CONN - BOLTED WxWM TO WxWM / GIRDER
NTS



12 COLLECTOR CONN - WxWM TO HSS COL AT ROOF
NTS



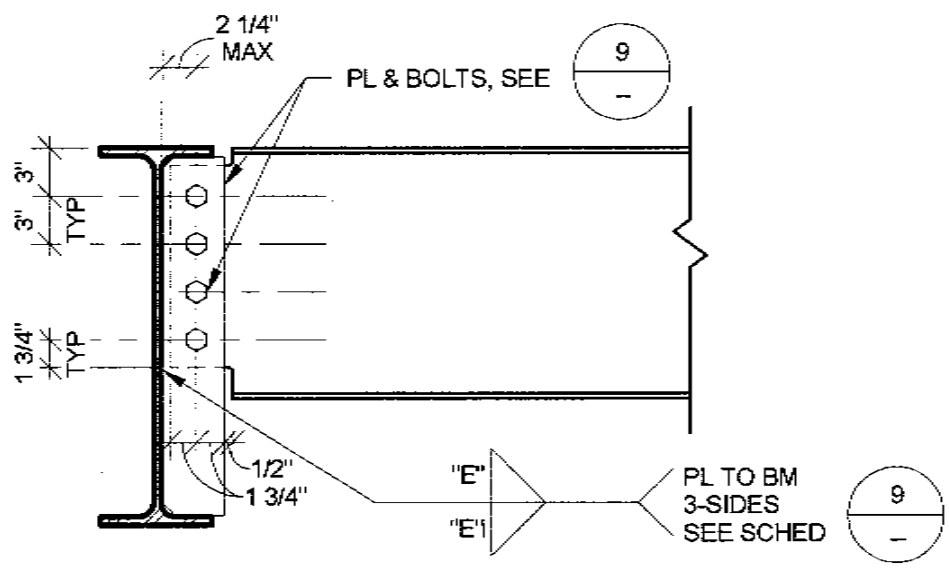
ROOF LEVEL



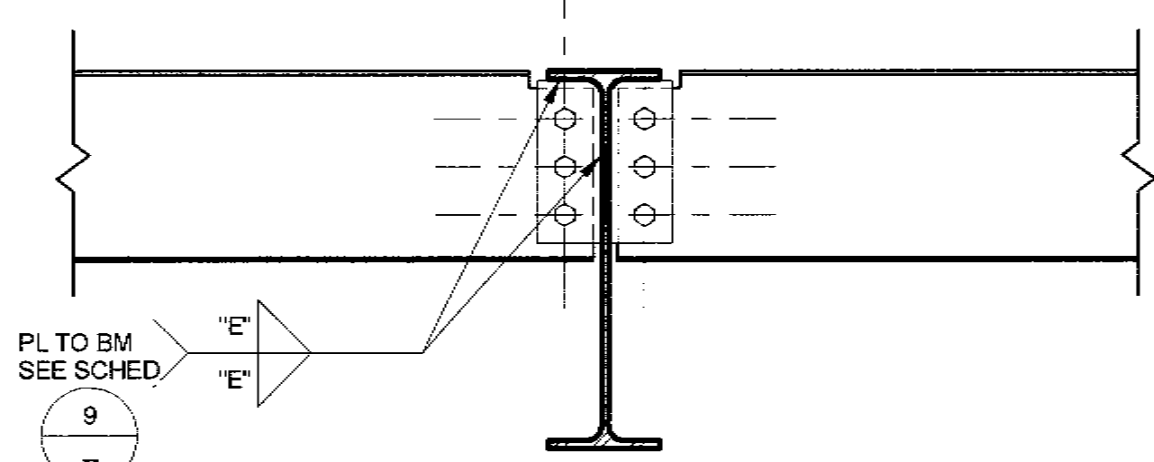
FLOOR LEVEL

NOTES:
1. PROVIDE PL'S AND ANGLES SHOWN AT BMS LOCATED AT PERIMETER OF BLDG AND AT STAIR AND ELEVATOR OPENINGS.
2. OMIT PL'S AND ANGLES AT SMRF BMS.
3. OMIT PL'S AND ANGLES AT TOP FLANGE OF BM WHERE BM FLANGE IS CONNECTED TO CONC OVER METAL DECK WITH WSA'S.

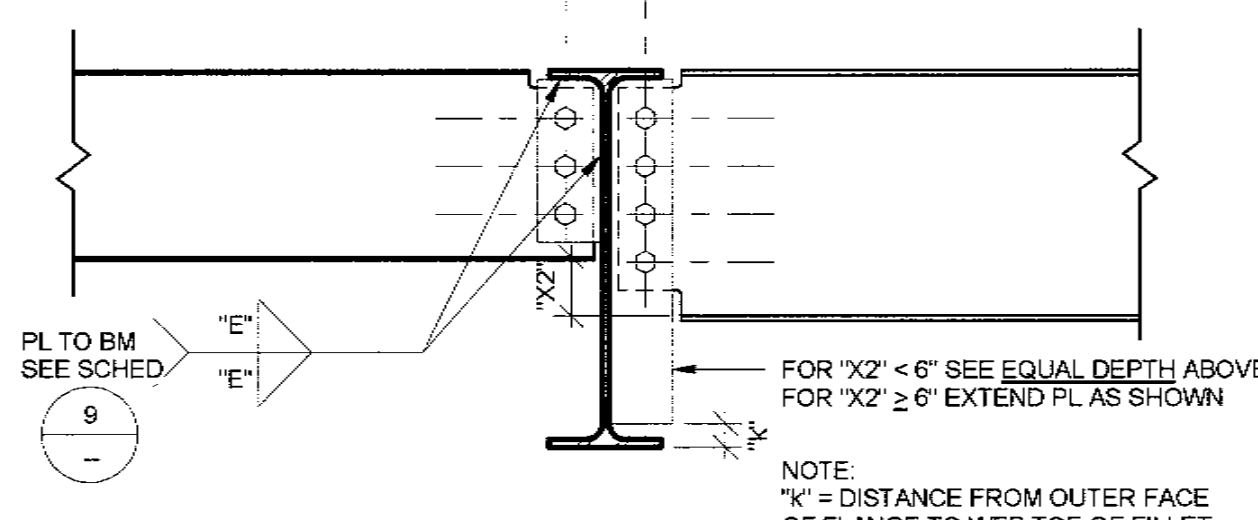
6 OUT OF PLANE BEAM RESTRAINT AT WxCOL
NTS



ONE SIDE ONLY

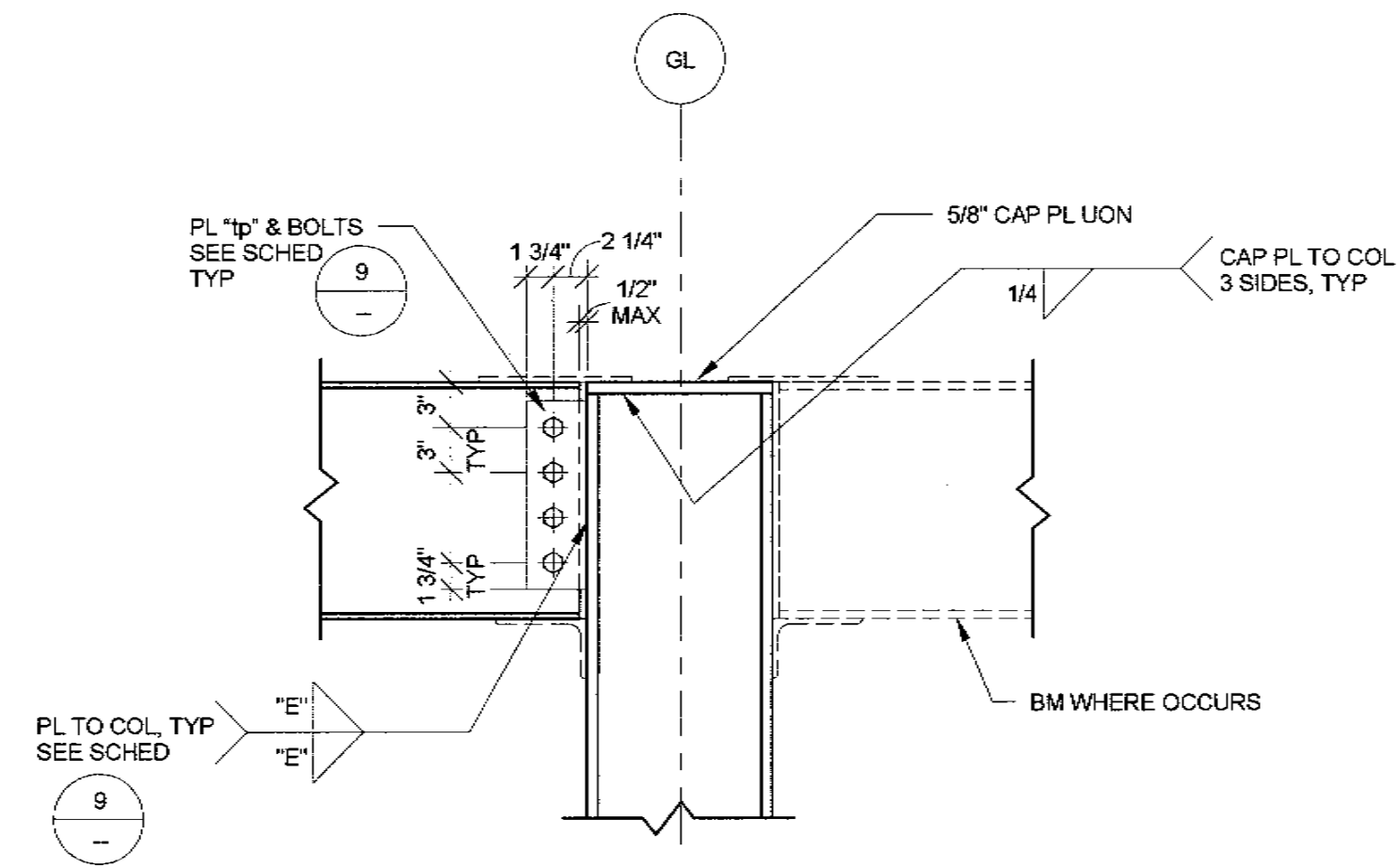
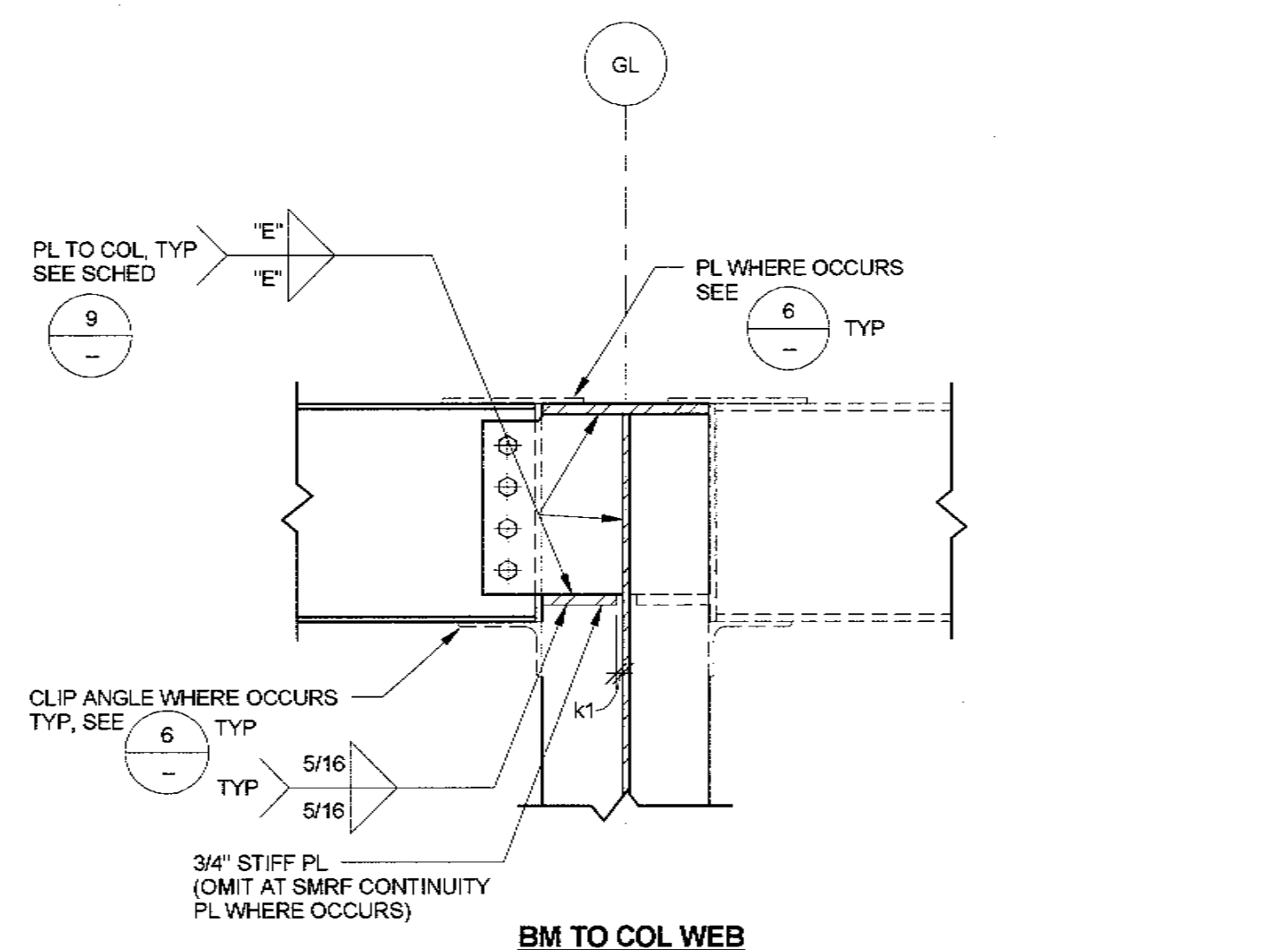


EQUAL DEPTH

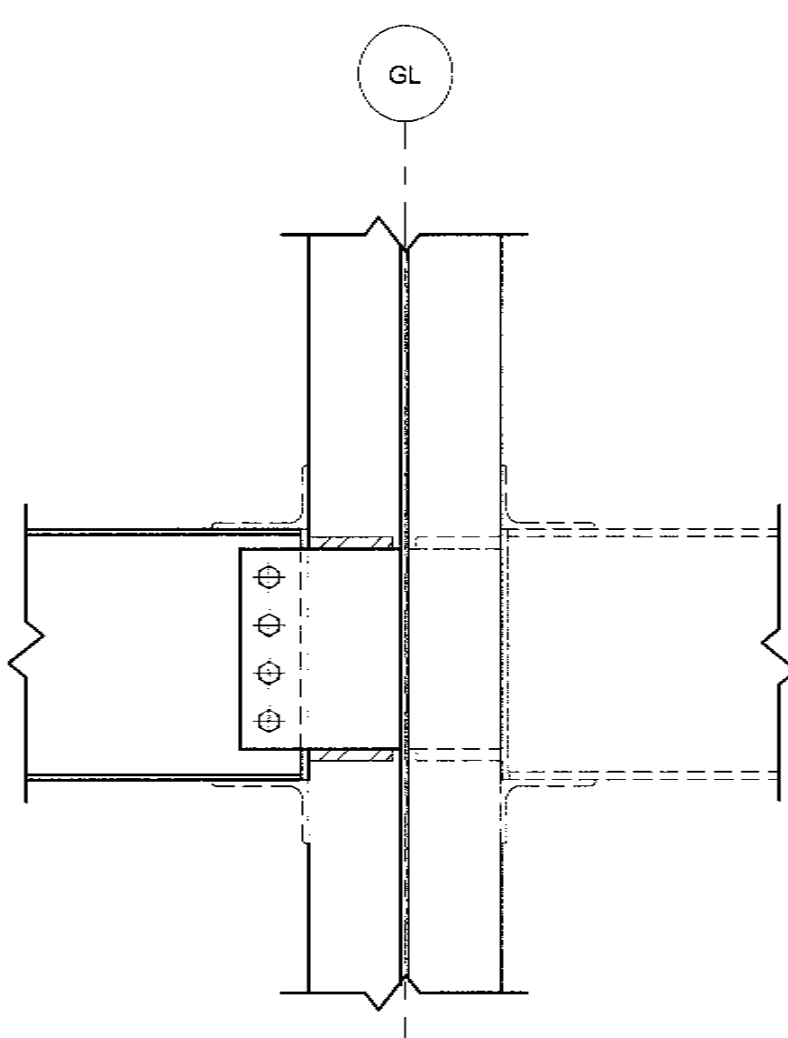


UNEQUAL DEPTH

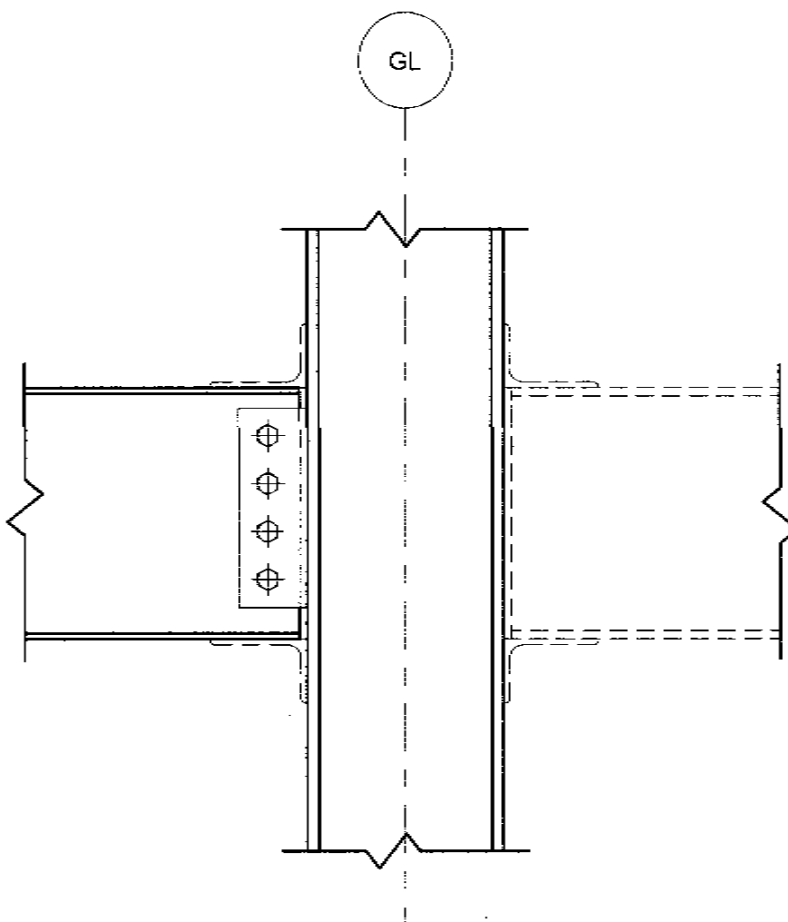
8 WxWM TO WxWM / GIRDER
NTS



2 WxWM TO WxCOL AT ROOF
NTS

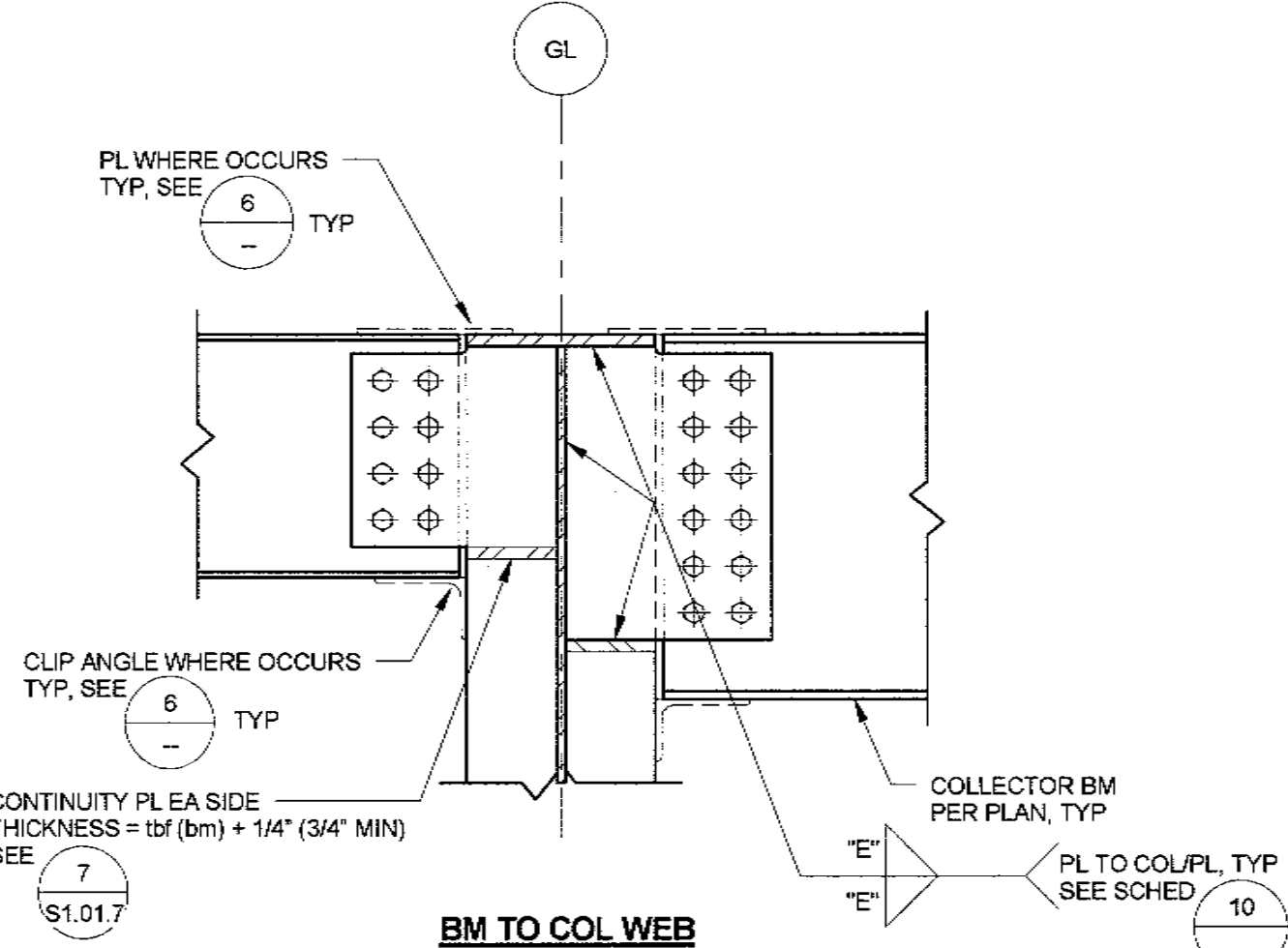


BM TO COL WEB

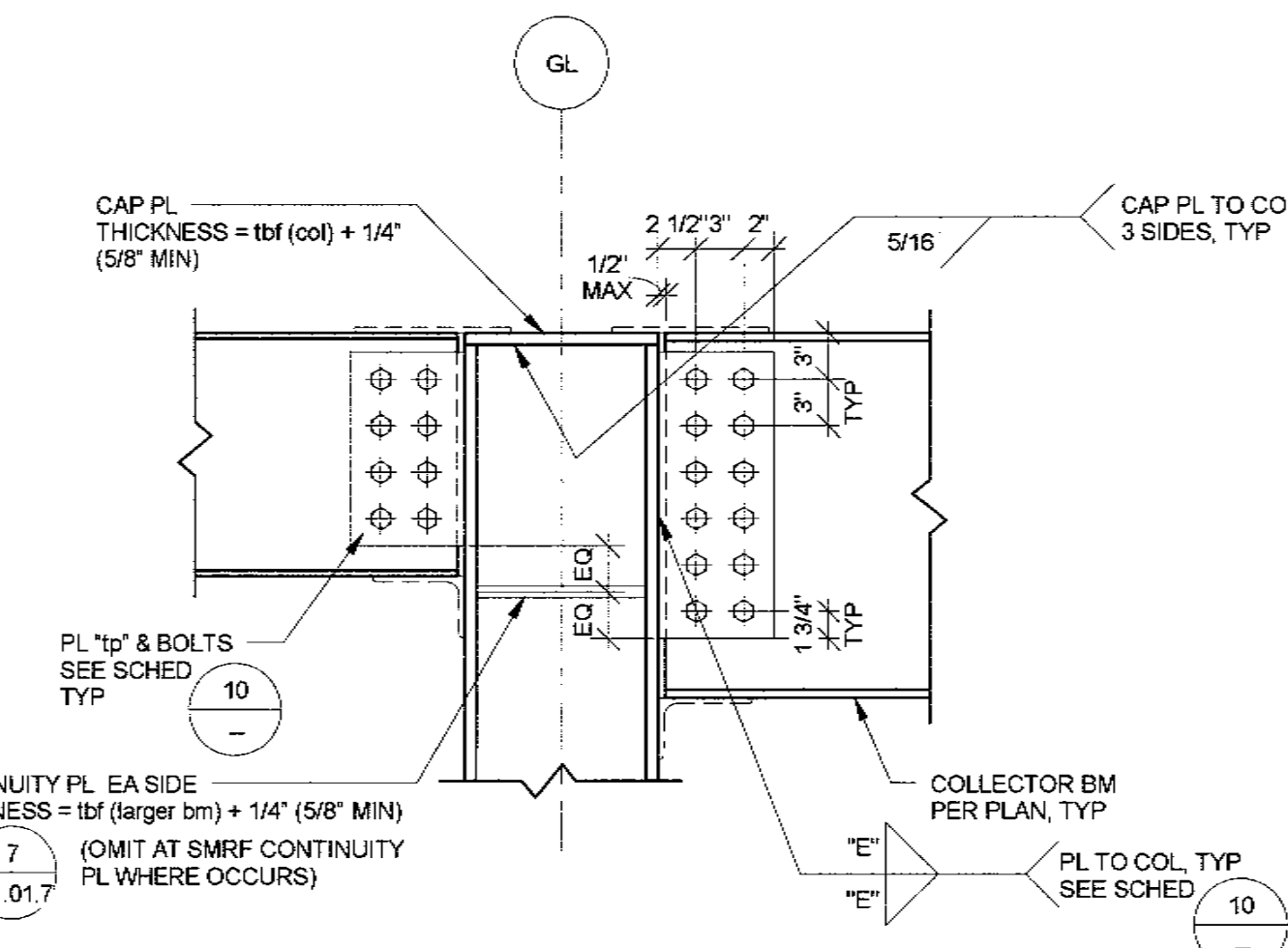


BM TO COL FLANGE

4 WxWM TO WxCOL AT FLOOR
NTS

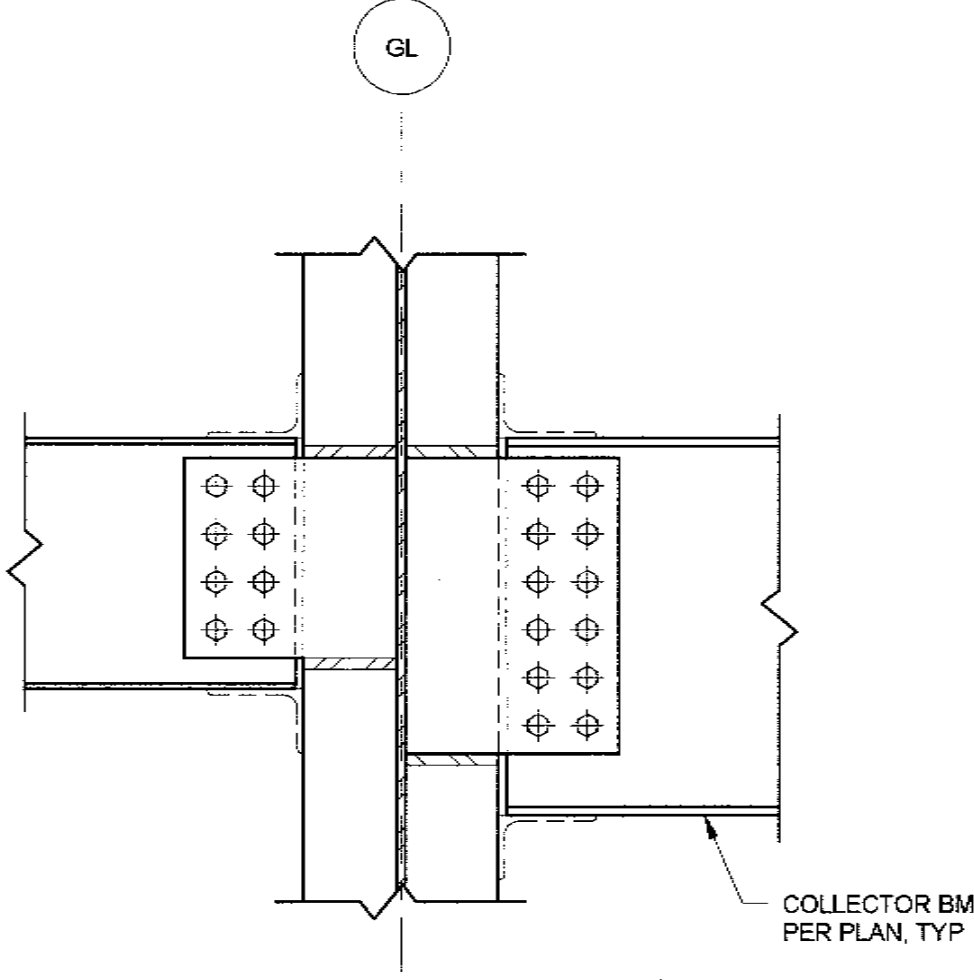


BM TO COL WEB

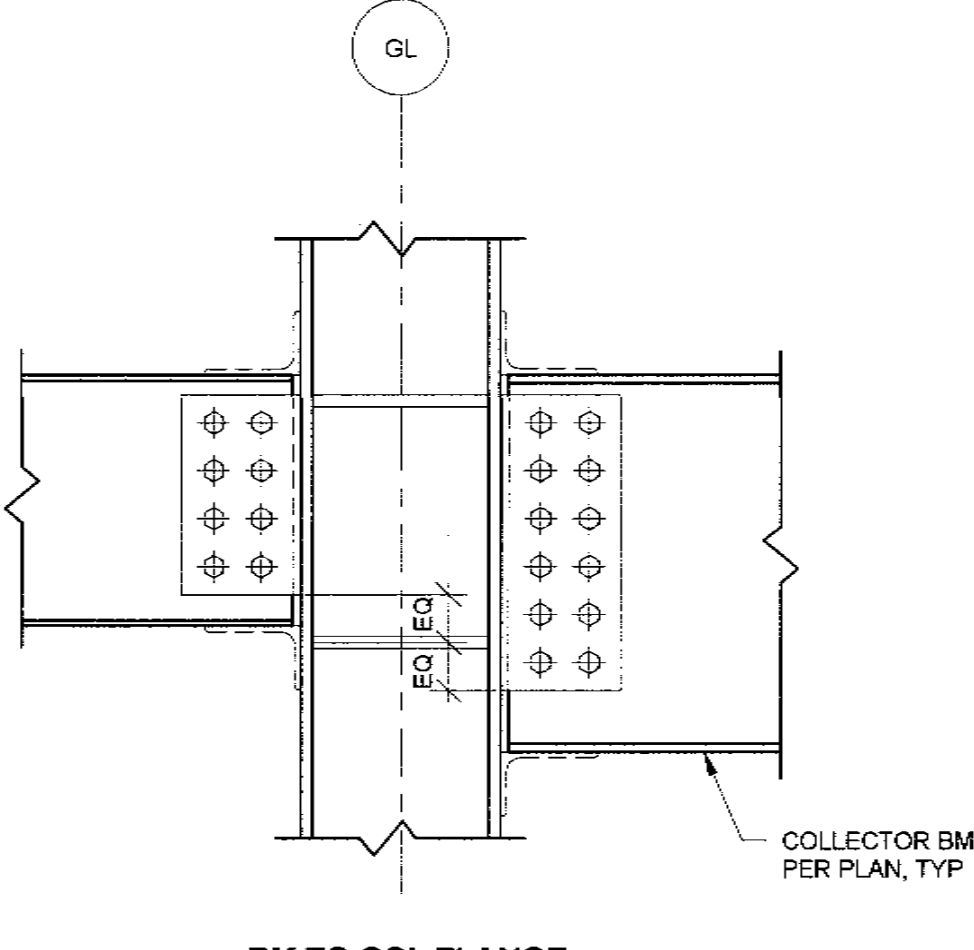


BM TO COL FLANGE

14 COLLECTOR CONN - WxWM TO WxCOL AT ROOF
NTS



BM TO COL WEB



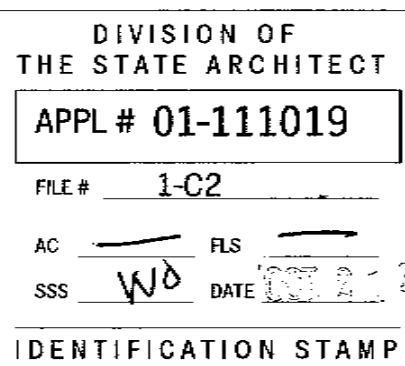
BM TO COL FLANGE

16 COLLECTOR CONN - WxWM TO WxCOL AT FLOOR
NTS

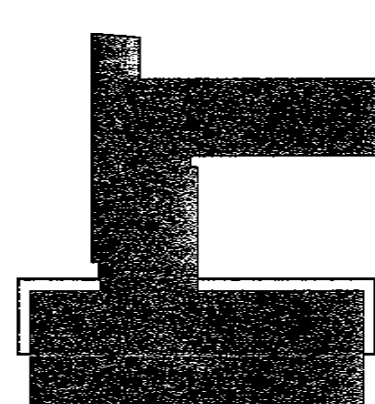
CLIENT
Chabot Las Positas Community College District
5020 Franklin Dr.
Pleasanton, CA 94568

ARCHITECT
Steinberg Architects
60 Pierce Avenue
San Jose, CA 95110

CONSULTANT/ENGINEER
Onosky Group Structural Engineering
2200 Bridge Parkway, Suite 104
Redwood City, CA 94065
(650) 361-9100
(650) 559-0277 (fax)



09 10.14.2010 DSA BACKCHECK
08 07.19.2010 100% CD SUBMITTAL TO DSA
06 06.03.2010 75% CD - DSA MEETING
05 05.20.2010 75% STRUCTURE - DSA COLLABORATIVE REVIEW
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03 03.11.2010 80% DD - DSA MEETING
02 12.12.2009 SD SUBMITTAL - DSA
01 11.19.2009 90% SD - DSA MEETING

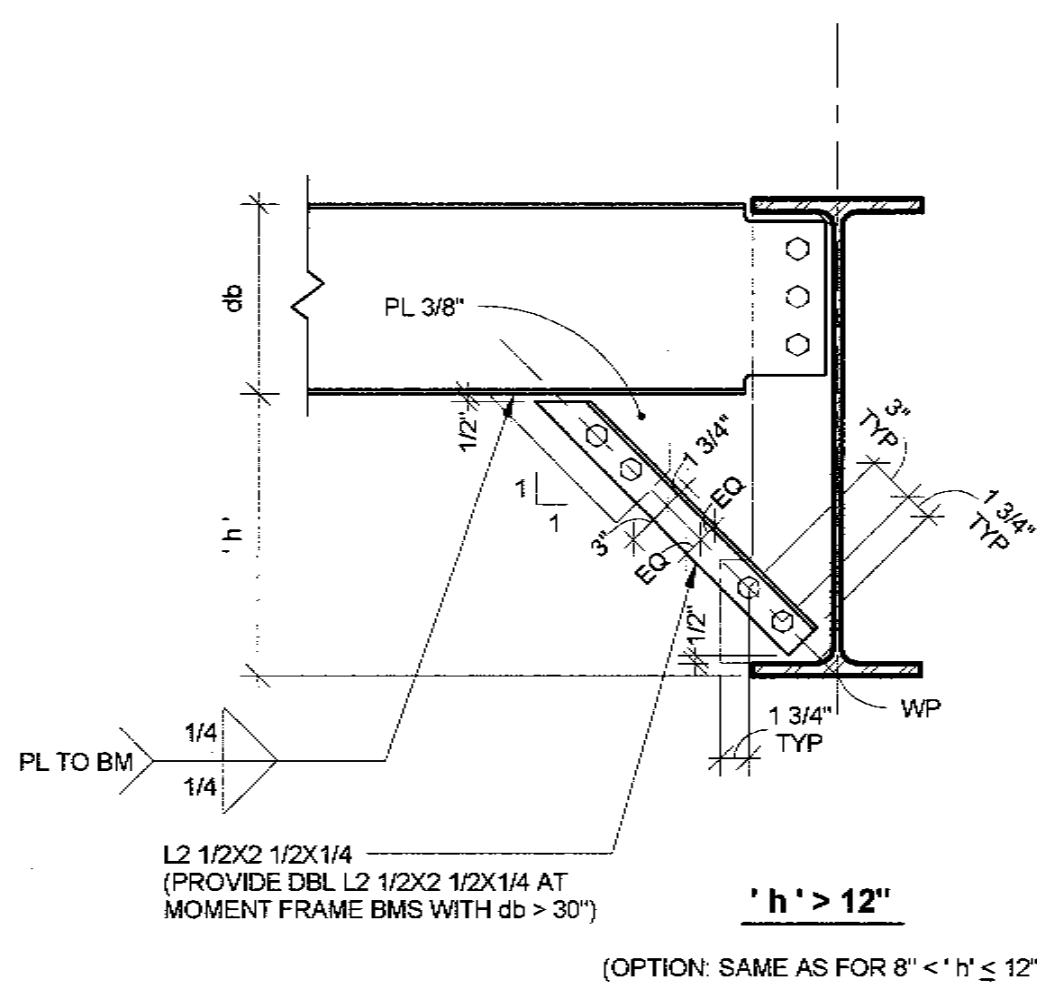
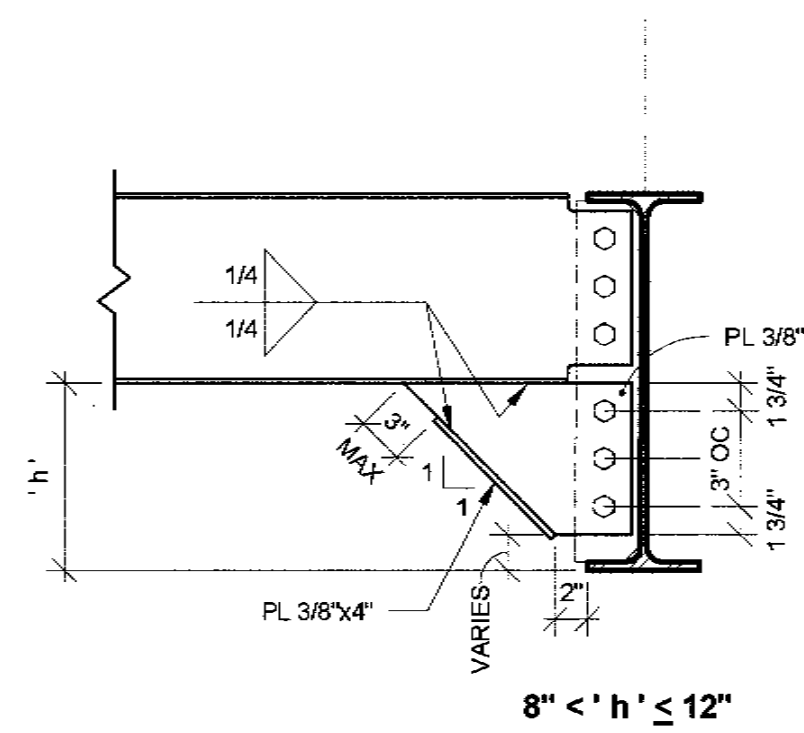
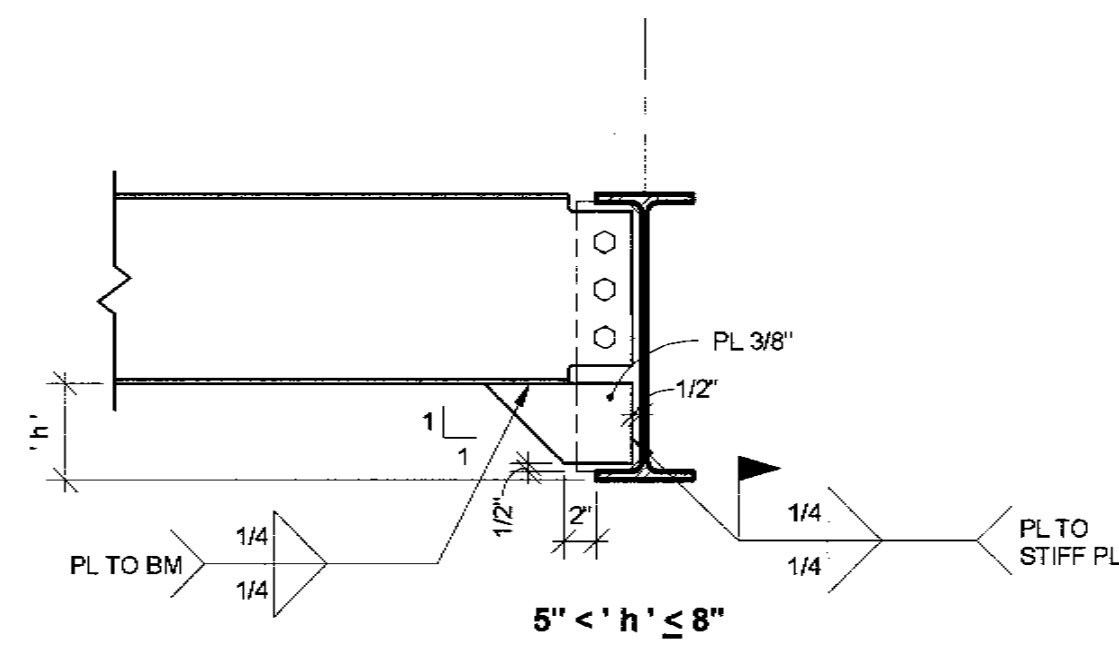
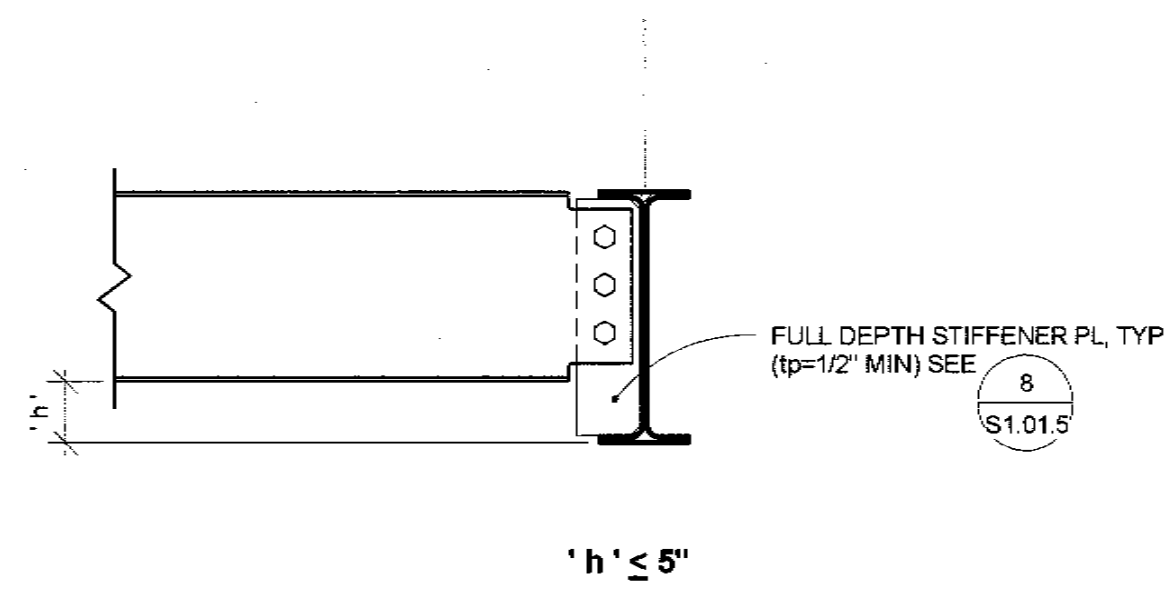


Student Services and Administration Building

Las Positas College
3000 Campus Hill Dr.,
Livermore, CA 94551

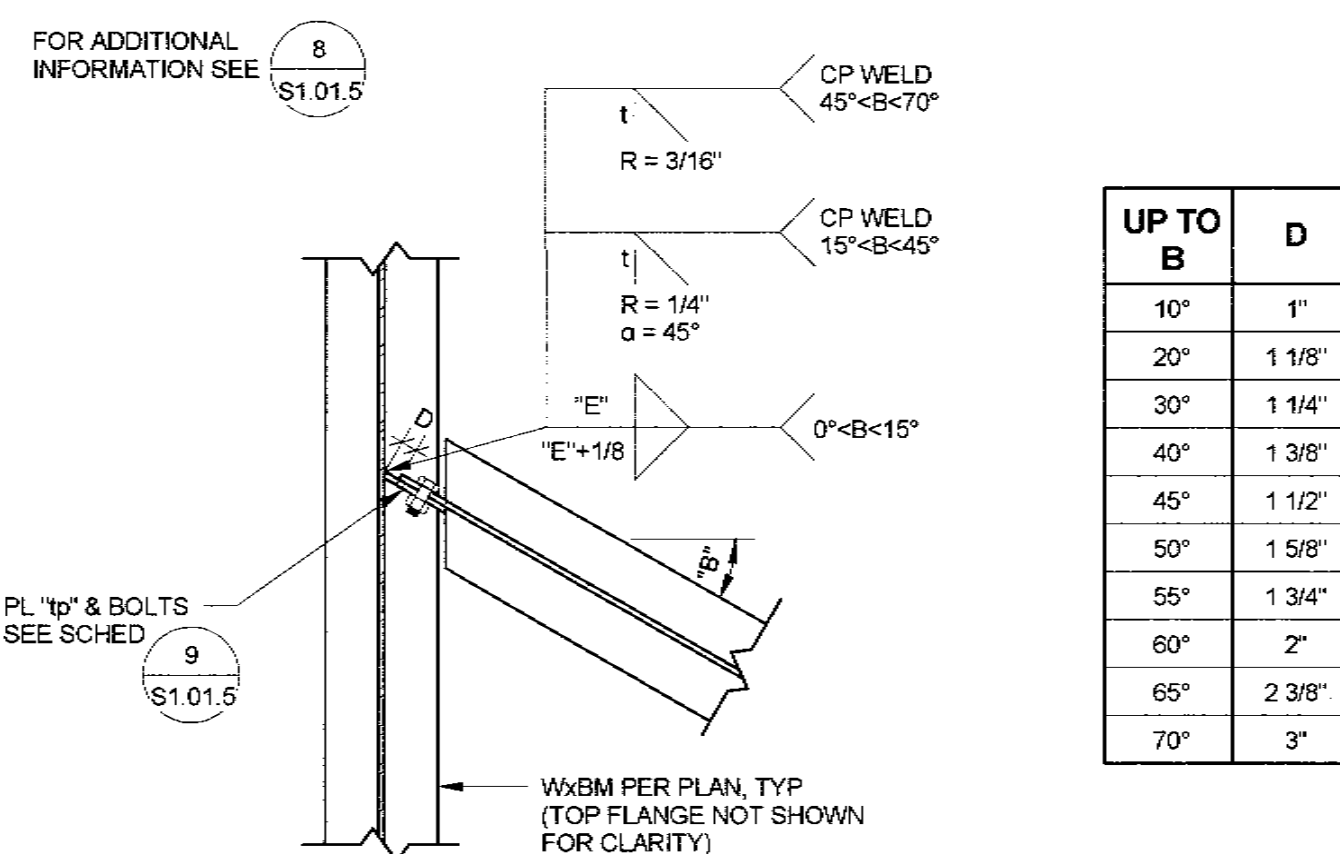
TYPICAL STEEL DETAILS

PROJECT #: 09050.000
DATE: 2010 October 14
SCALE: NTS



- NOTES:
- DETAIL APPLIES AT ONE SIDE OF:
A. PERIMETER BEAMS
B. MOMENT FRAME BEAMS
C. BRACED FRAME BEAMS
D. COLLECTOR BEAMS
 - ALL BOLTS, INCLUDING ALL BEAM WEB BOLTS, SHALL BE 3/4\"/>
 - FOR ADDITIONAL INFO, SEE S1.01.5

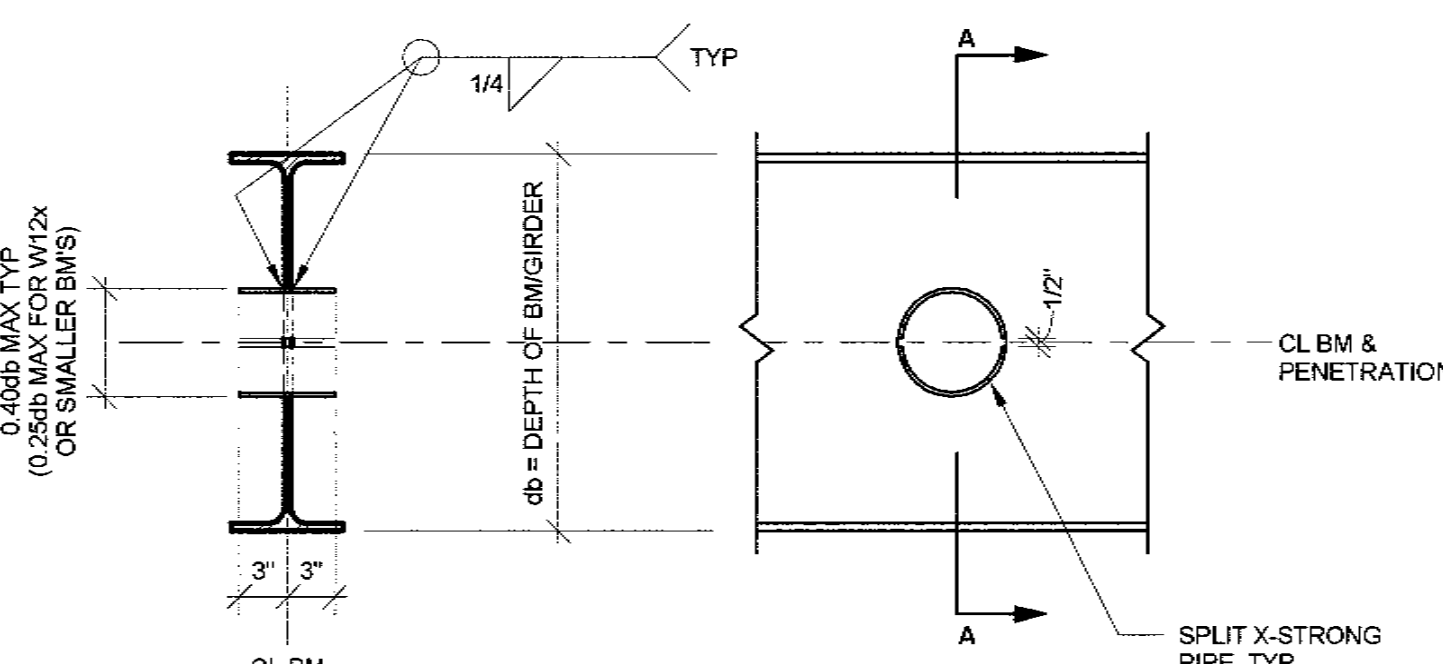
3 WxM CONN AT PERIMETER & FRAME BEAMS
NTS



4 SKEWED WxM TO WxM CONN
NTS

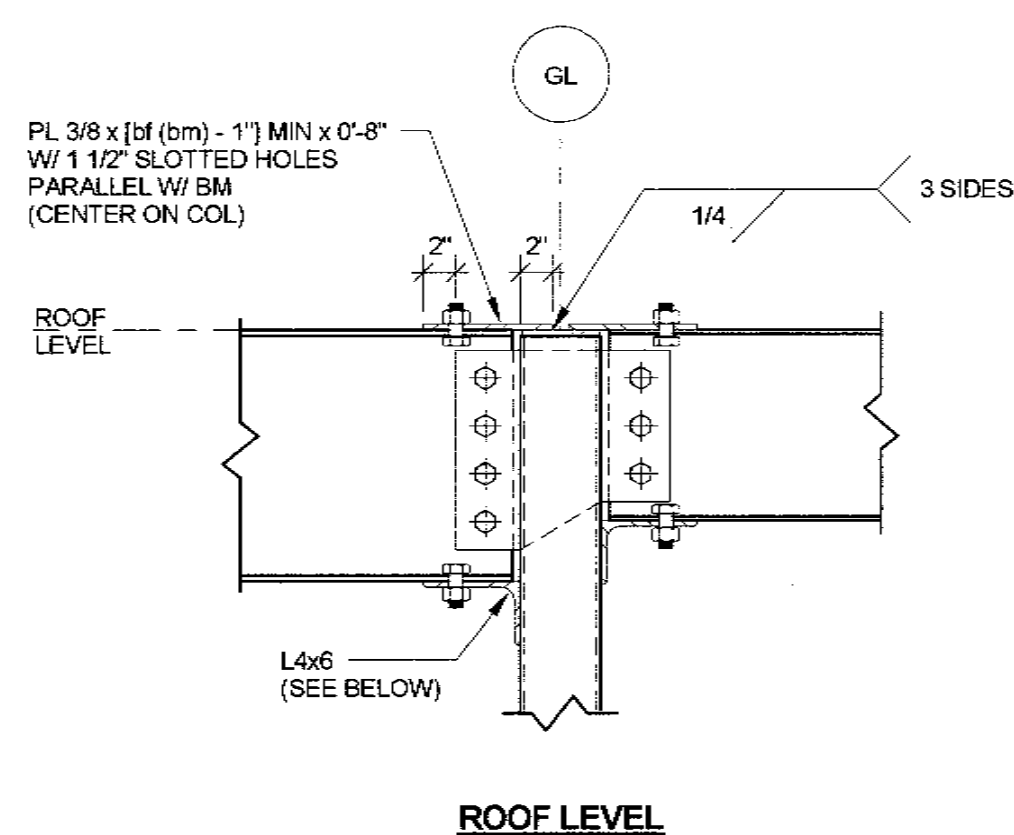
BM / GIRDER SIZE	MAXIMUM SIZE OF OPENING (IN)		REINFORCING PLATES (IN)	
	D	L	t	B
W16	5 1/2	16	2 1/2	2 1/2
W18	6	18	2 1/2	2 1/2
W21	7	21	3	3
W24	8	24	3 1/2	3 1/2
W27	9	27	3 1/2	3 1/2
W30	10	30	4	4
W36	12	32	6	6

- NOTES:
- SEE PLANS FOR LOCATION OF PENETRATIONS.
 - PENETRATIONS NOT PERMITTED AT COLLECTOR, MOMENT FRAME, OR BRACED FRAME BEAMS.

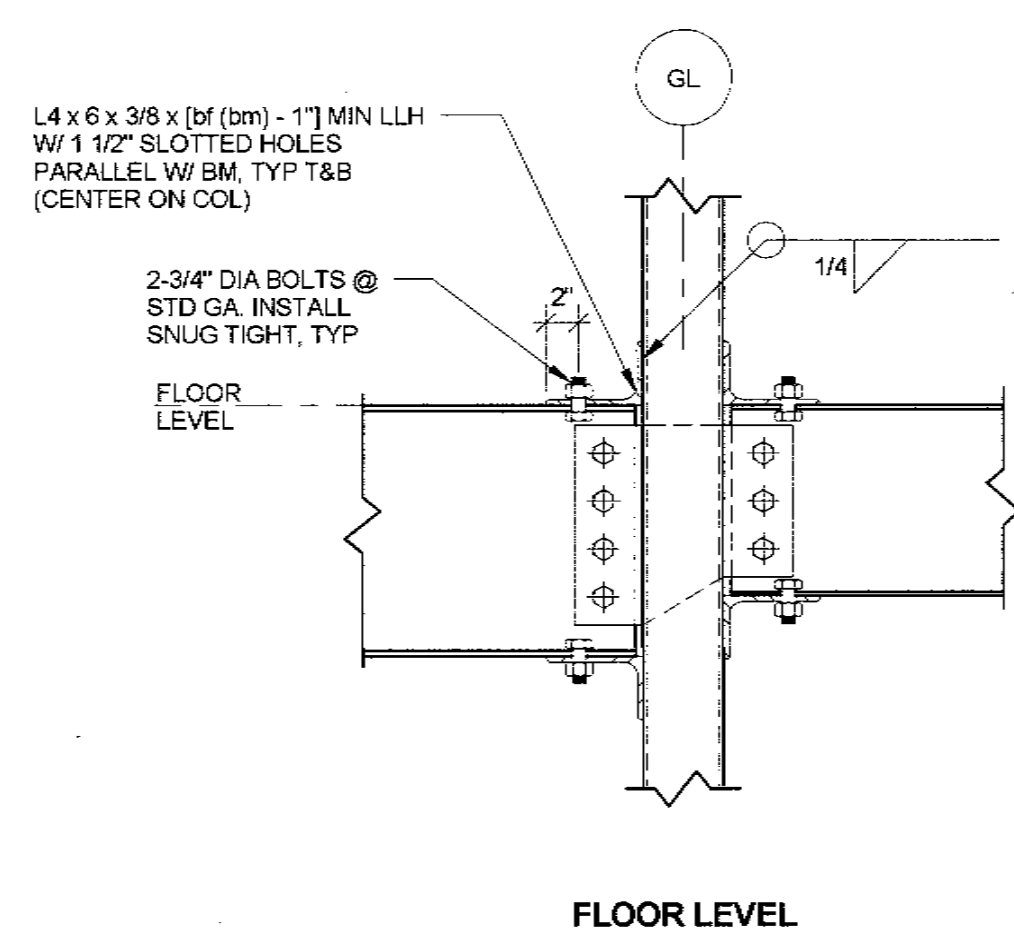


- NOTES:
- SEE PLANS FOR LOCATION OF PENETRATIONS.
 - PENETRATIONS NOT PERMITTED AT COLLECTOR, MOMENT FRAME, OR BRACED FRAME BEAMS.

6 WxM REINF AT SMALL PIPE PENETRATION
NTS



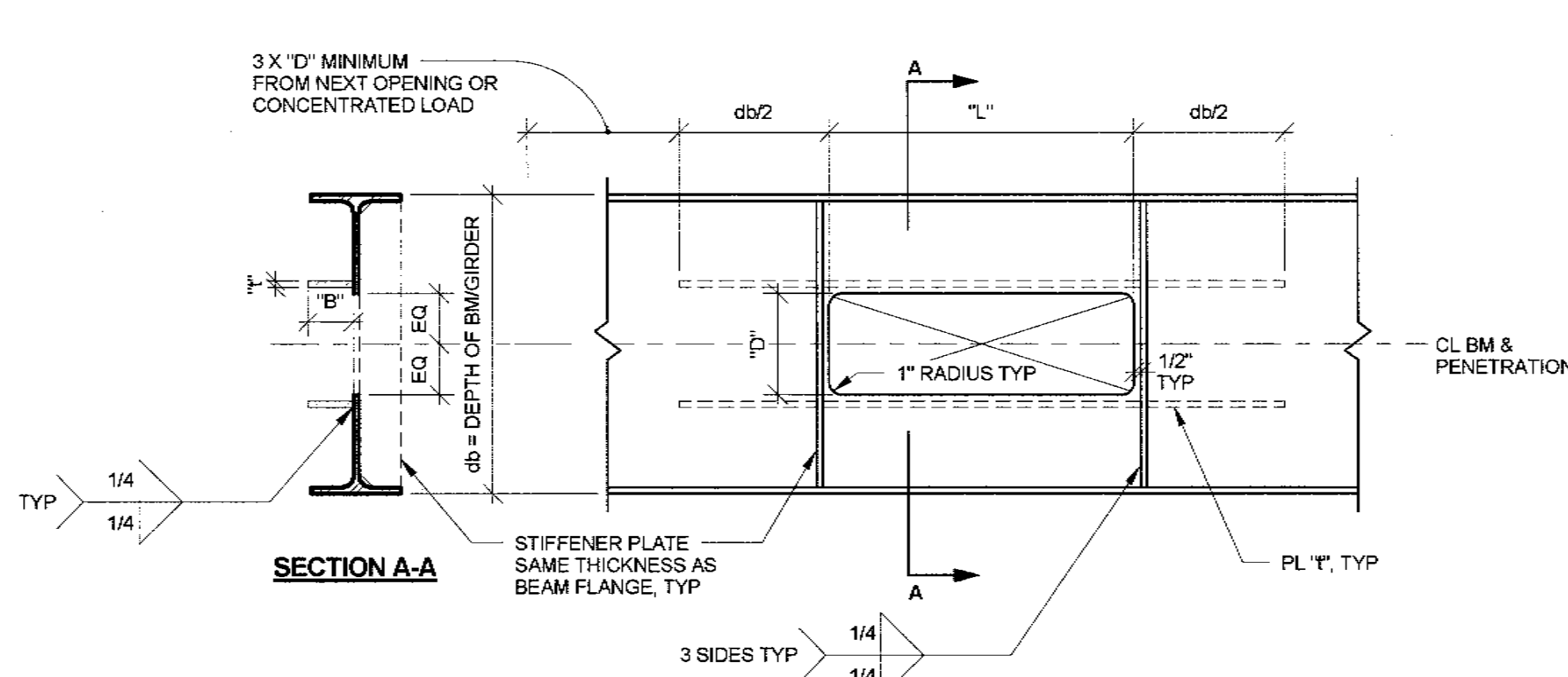
ROOF LEVEL



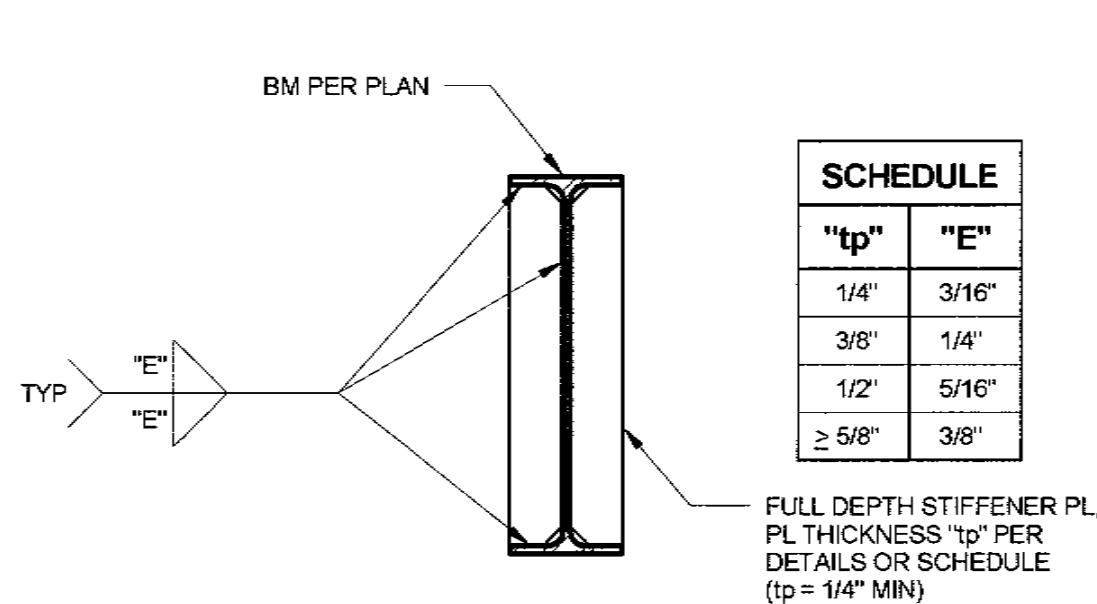
FLOOR LEVEL

- NOTES:
- PROVIDE PLS AND ANGLES SHOWN AT BMS LOCATED AT PERIMETER OF BLDG AND AT STAIR AND ELEVATOR OPENINGS.
 - OMIT PLS AND ANGLES AT TOP FLANGE OF BM WHERE BM FLANGE IS CONNECTED TO CONC OVER METAL DECK WITH WSAs.

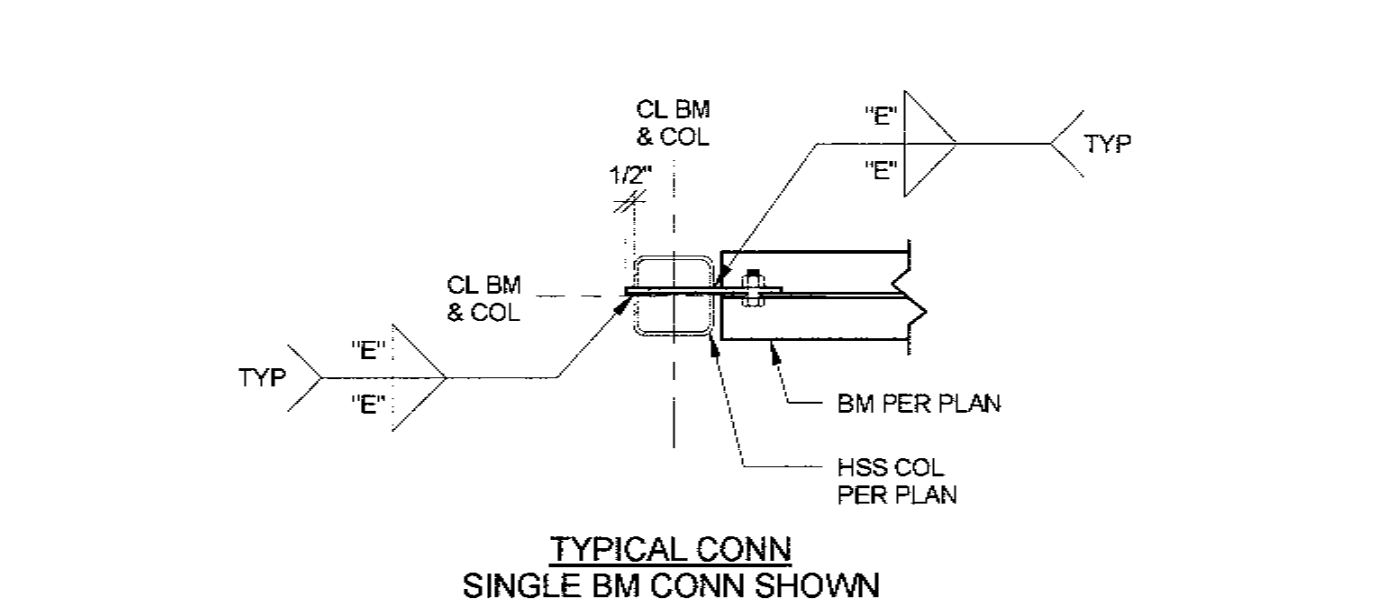
8 OUT OF PLANE BEAM RESTRAINT AT HSS COL
NTS



9 WxM REINF AT LARGE PIPE PENETRATION
NTS

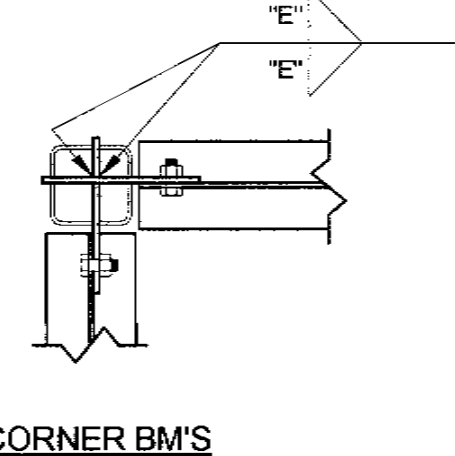


10 WxM FULL DEPTH STIFFENER PLATE
NTS

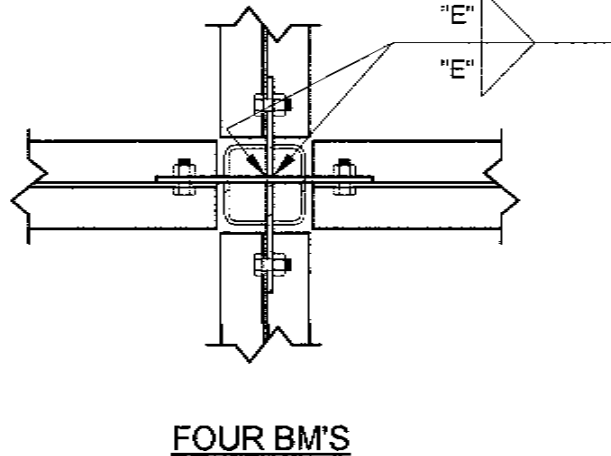


TWO BMS

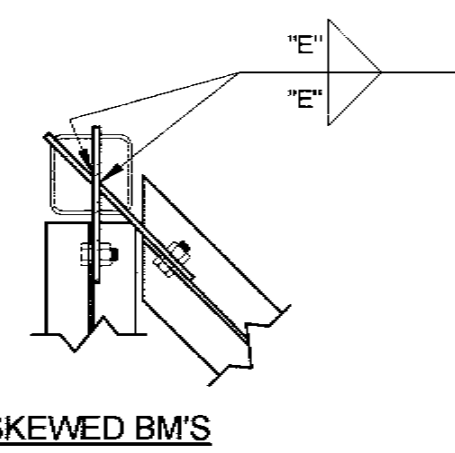
THREE BMS



CORNER BMS



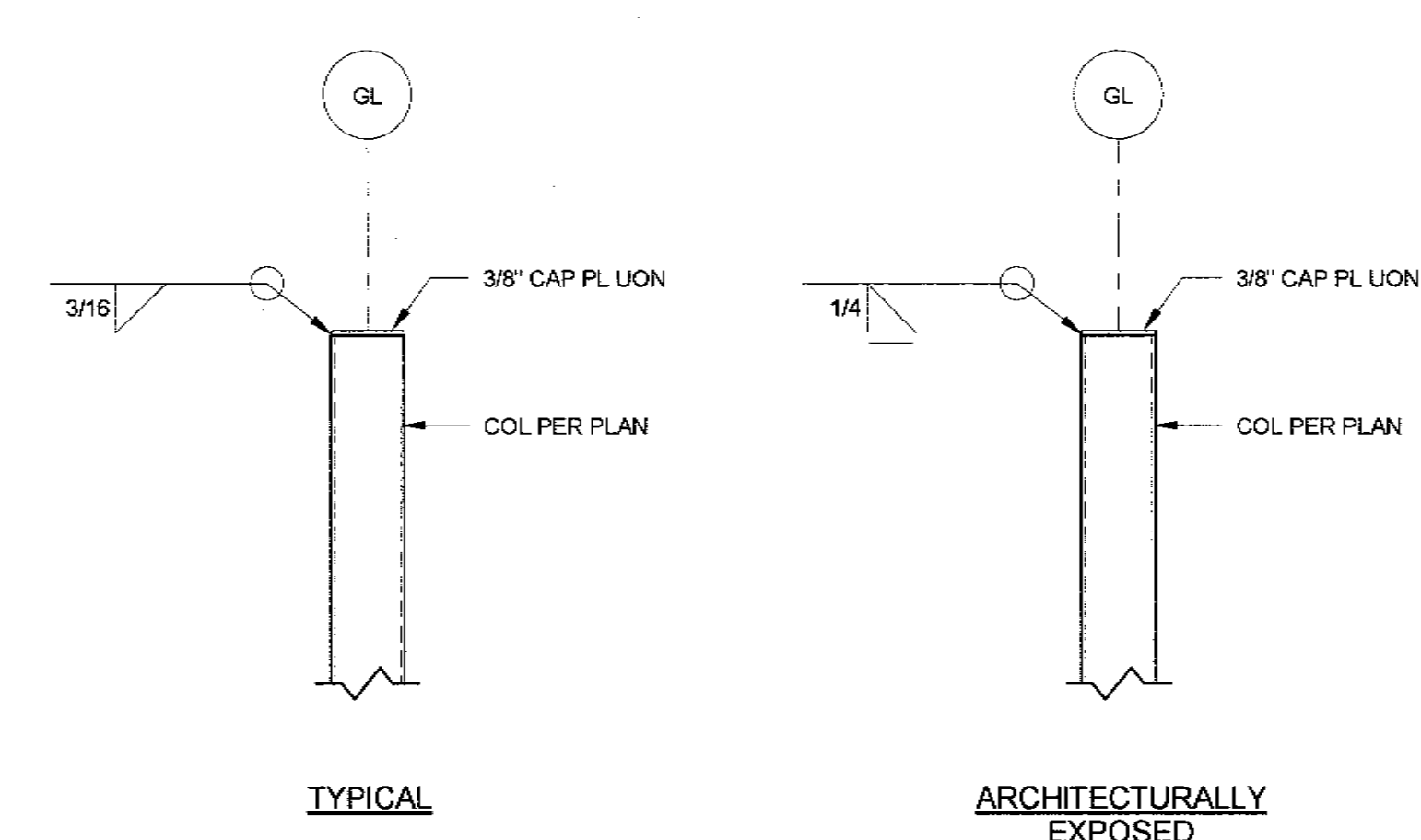
FOUR BMS



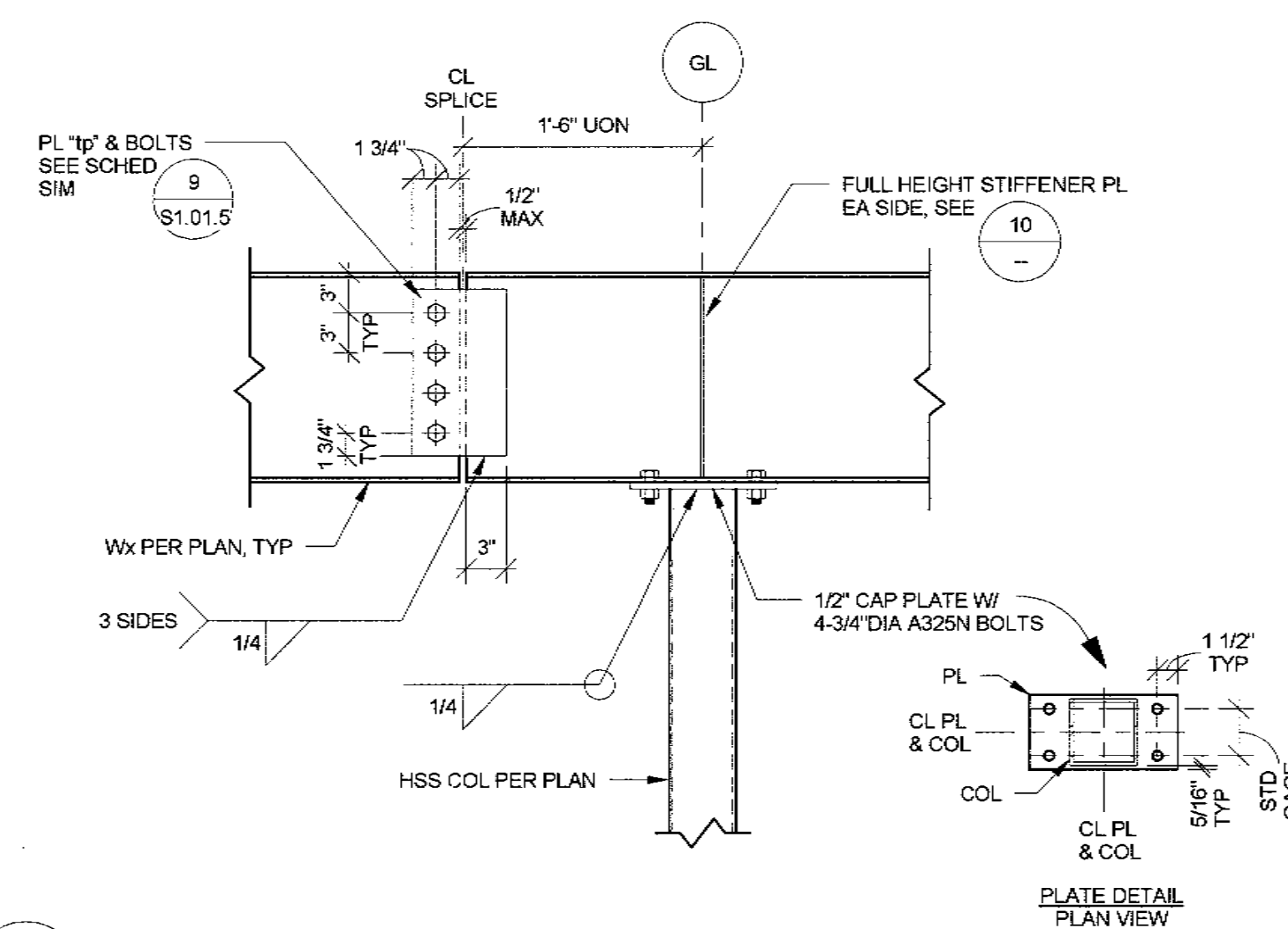
SKEWED BMS

- NOTES:
- FOR WELDING "E" SEE SCHED S1.01.5
 - SLOTS FOR HSS CONNECTIONS SHALL BE 1/8\"/>
 - FOR ADDITIONAL INFORMATION NOT SHOWN, SEE 15- AT ROOF CONDITION AND 16- AT FLOOR CONDITION

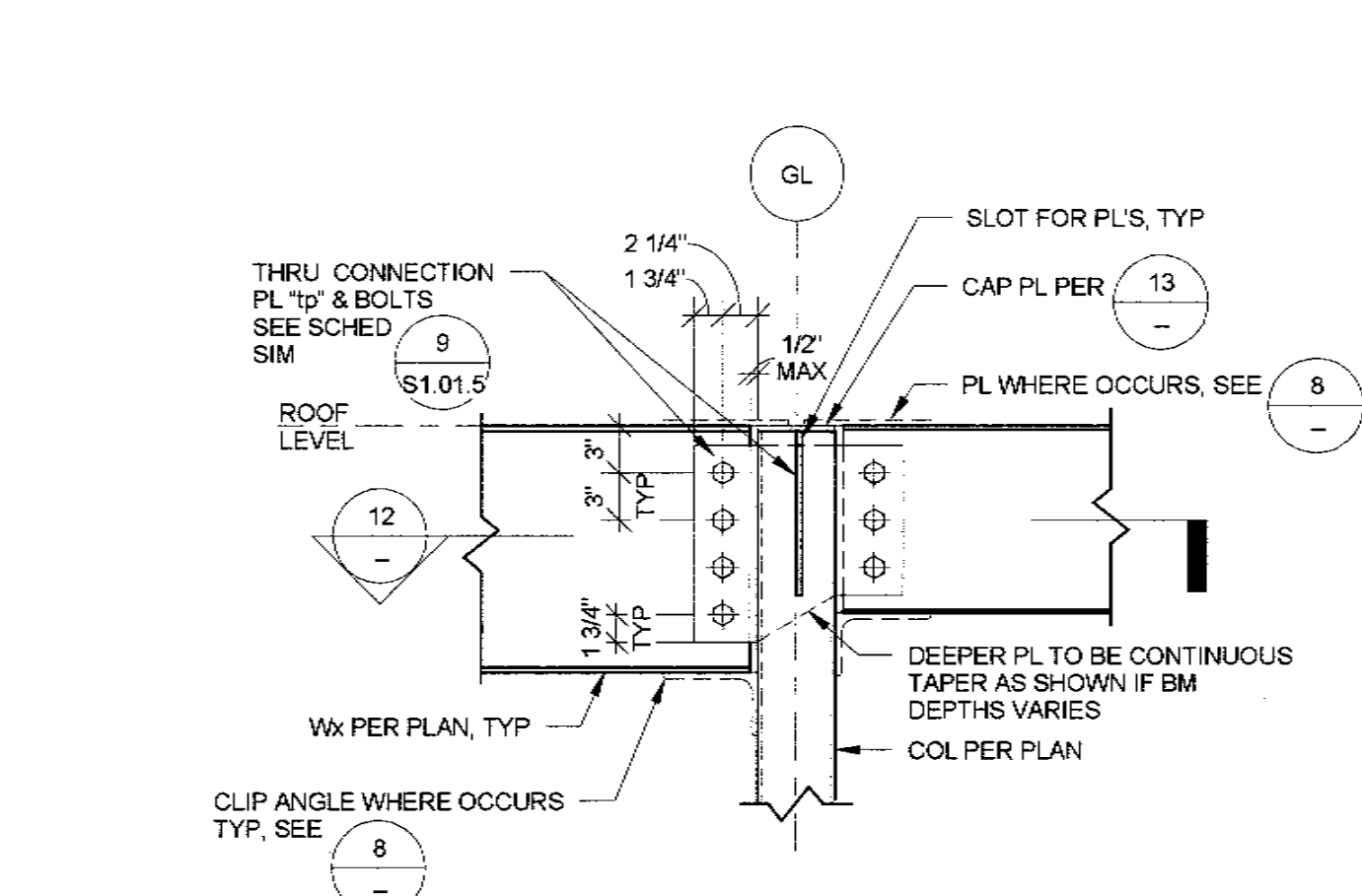
12 WxM TO HSS COL W/ THRU PLATE (PLAN VIEW)
NTS



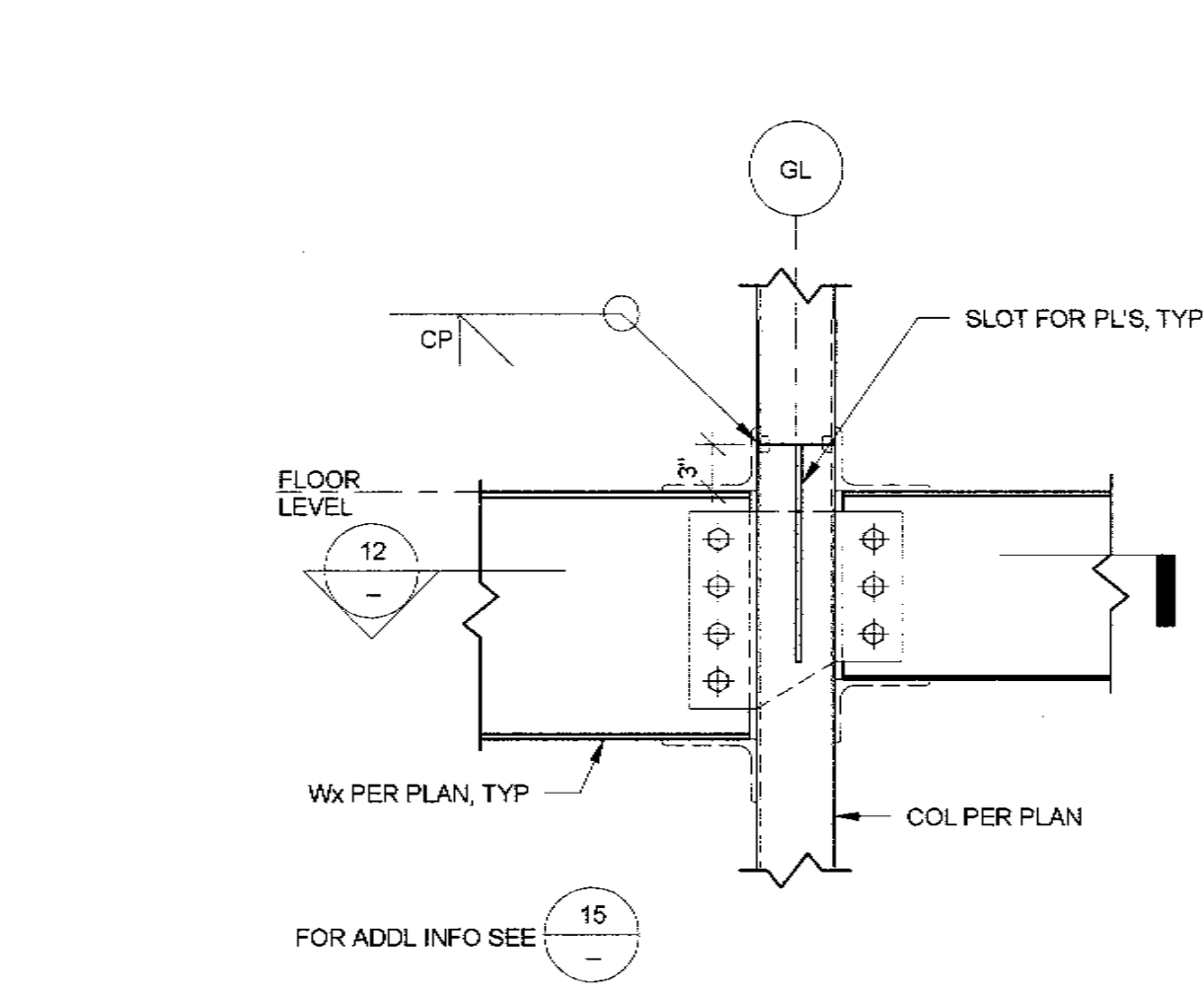
13 HSS CAP PLATE
NTS



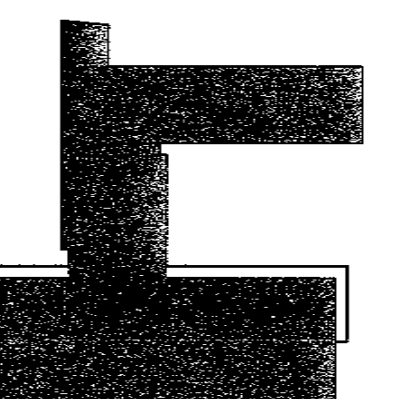
14 CONT WxM OVER HSS COL
NTS



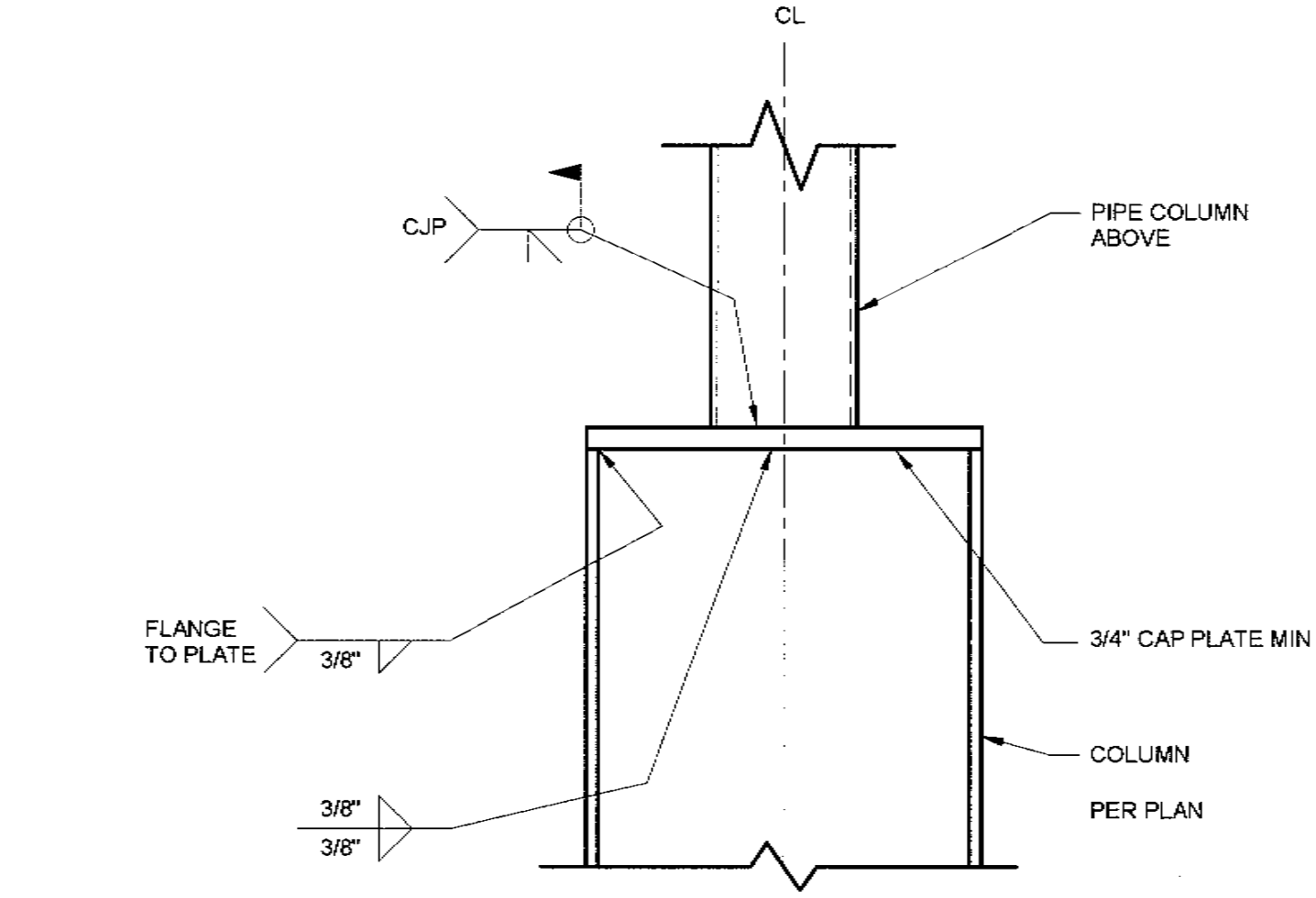
15 WxM TO HSS COL W/ THRU PLATE AT ROOF
NTS



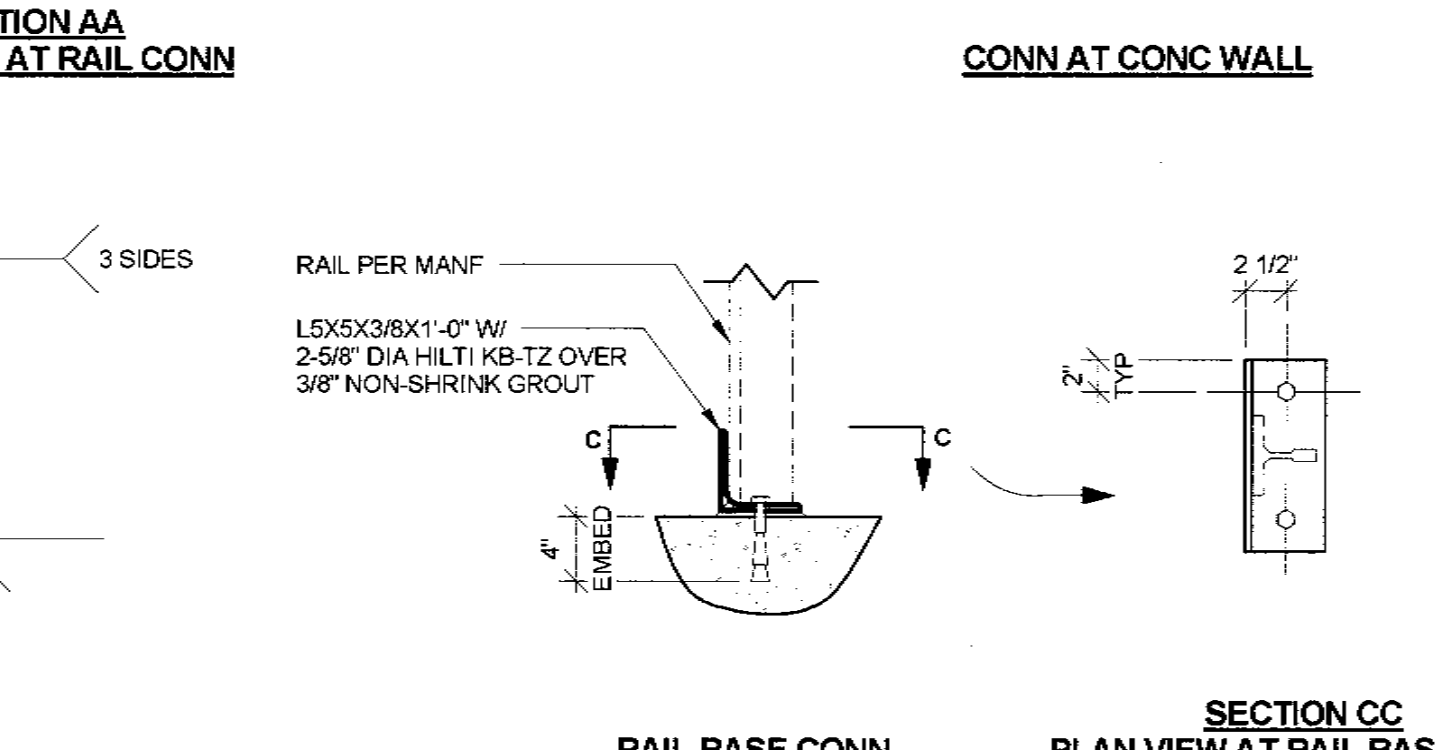
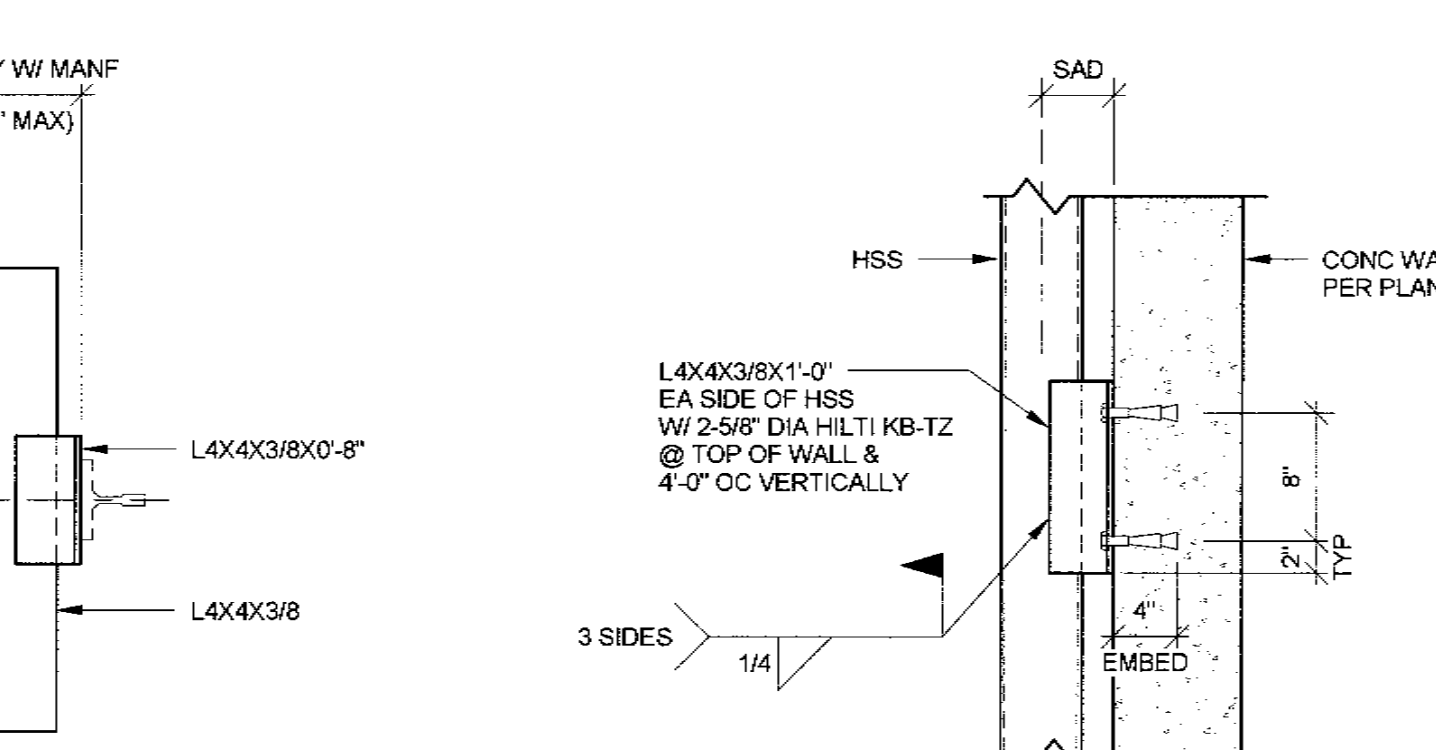
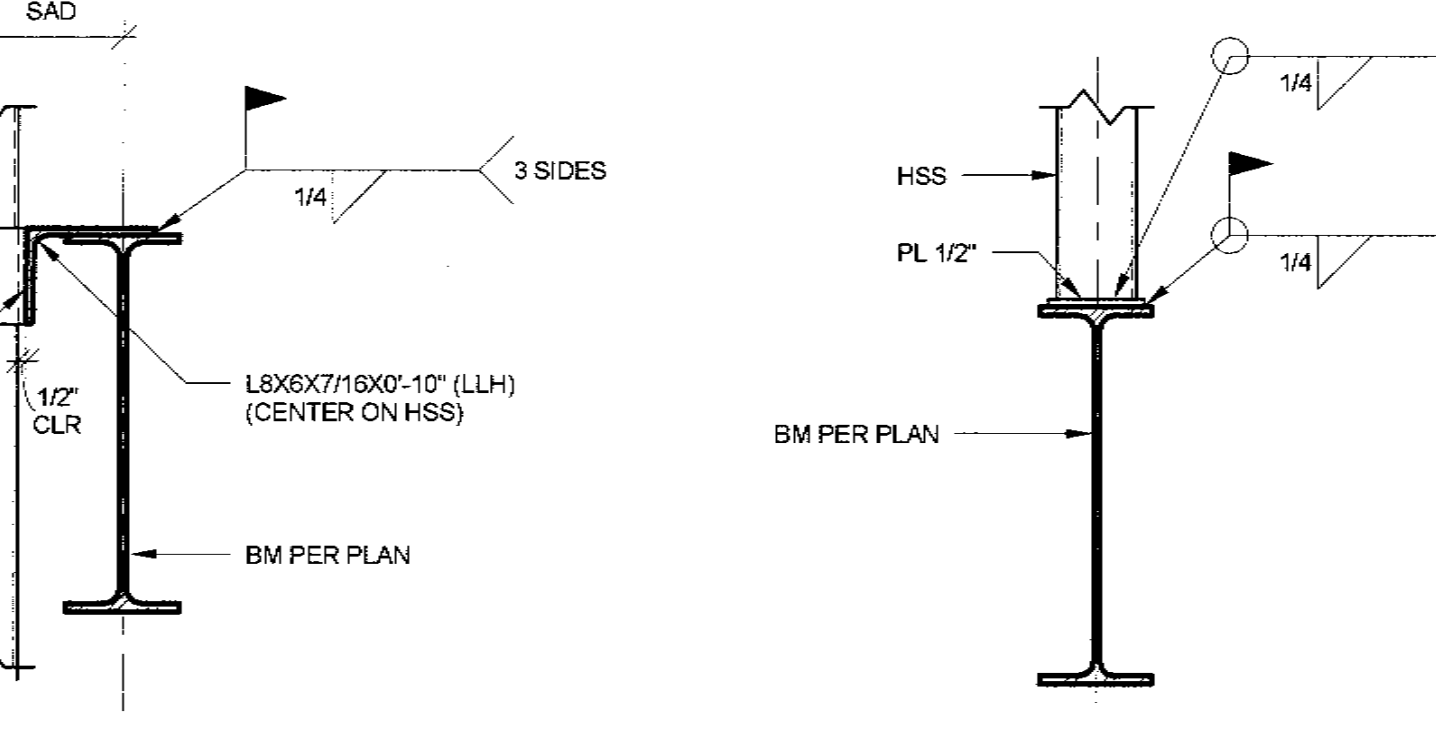
16 WxM TO HSS COL W/ THRU PLATE AT FLOOR
NTS



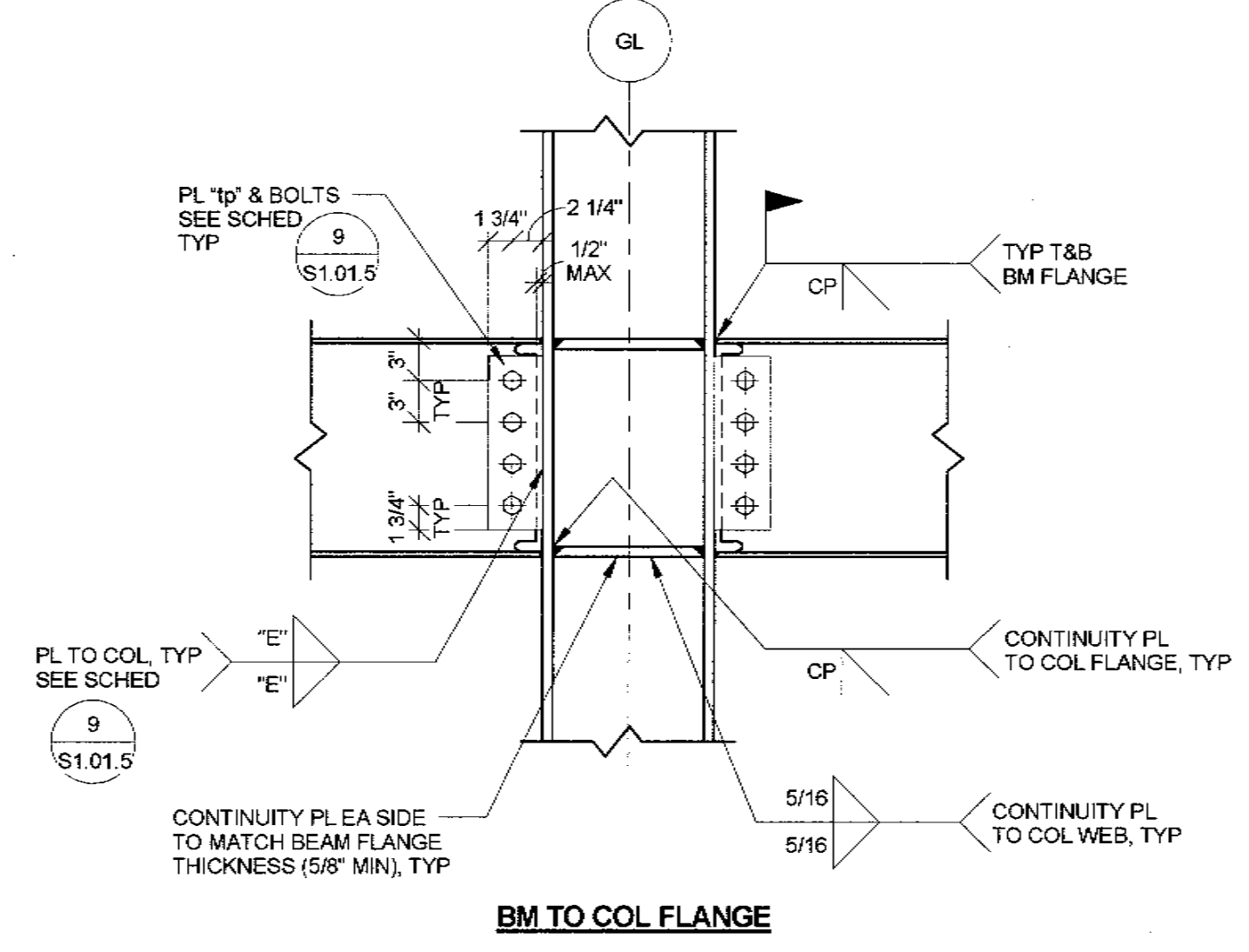
**9 FALL ARREST DETAIL
NTS**



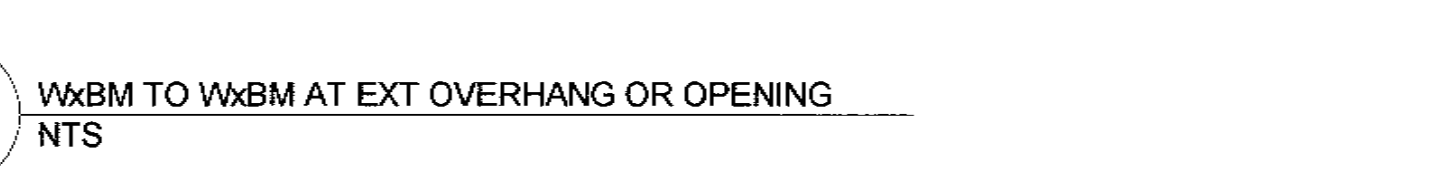
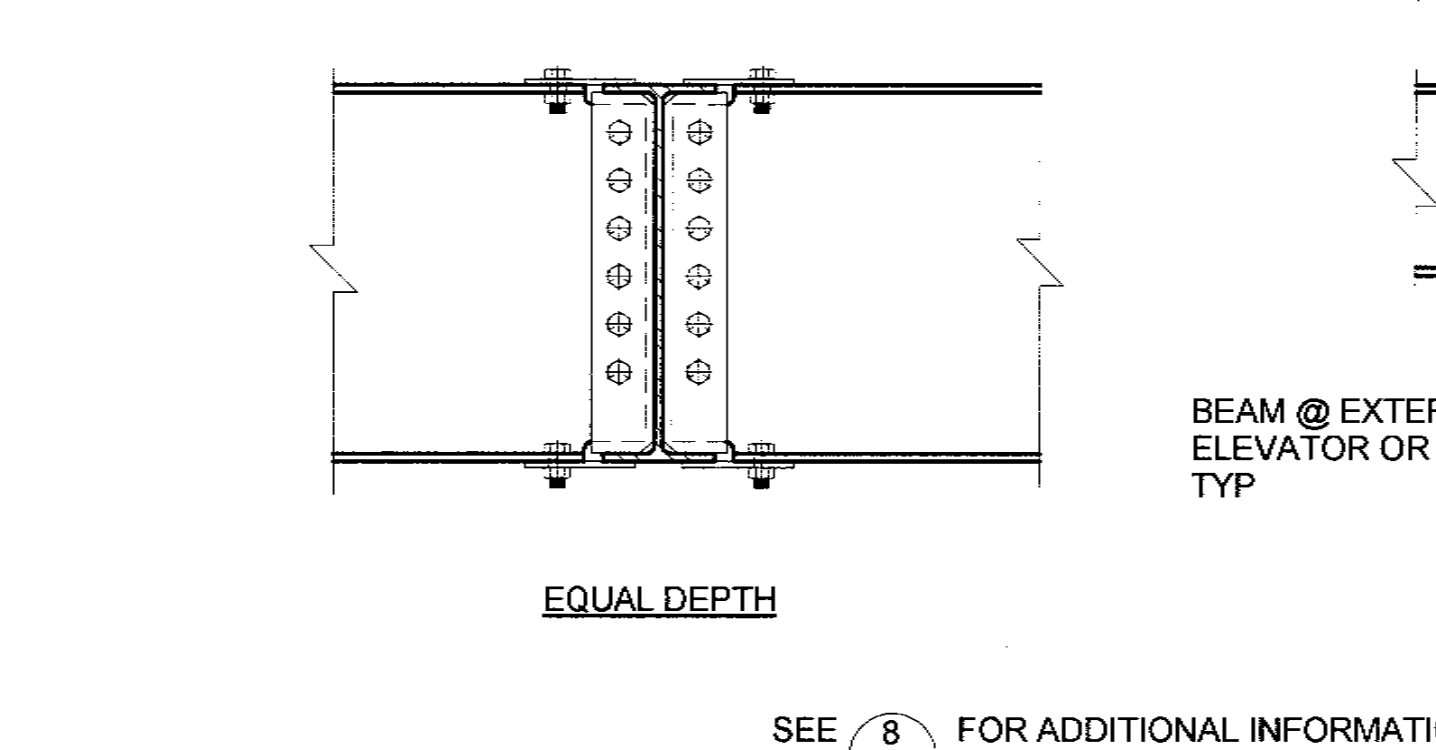
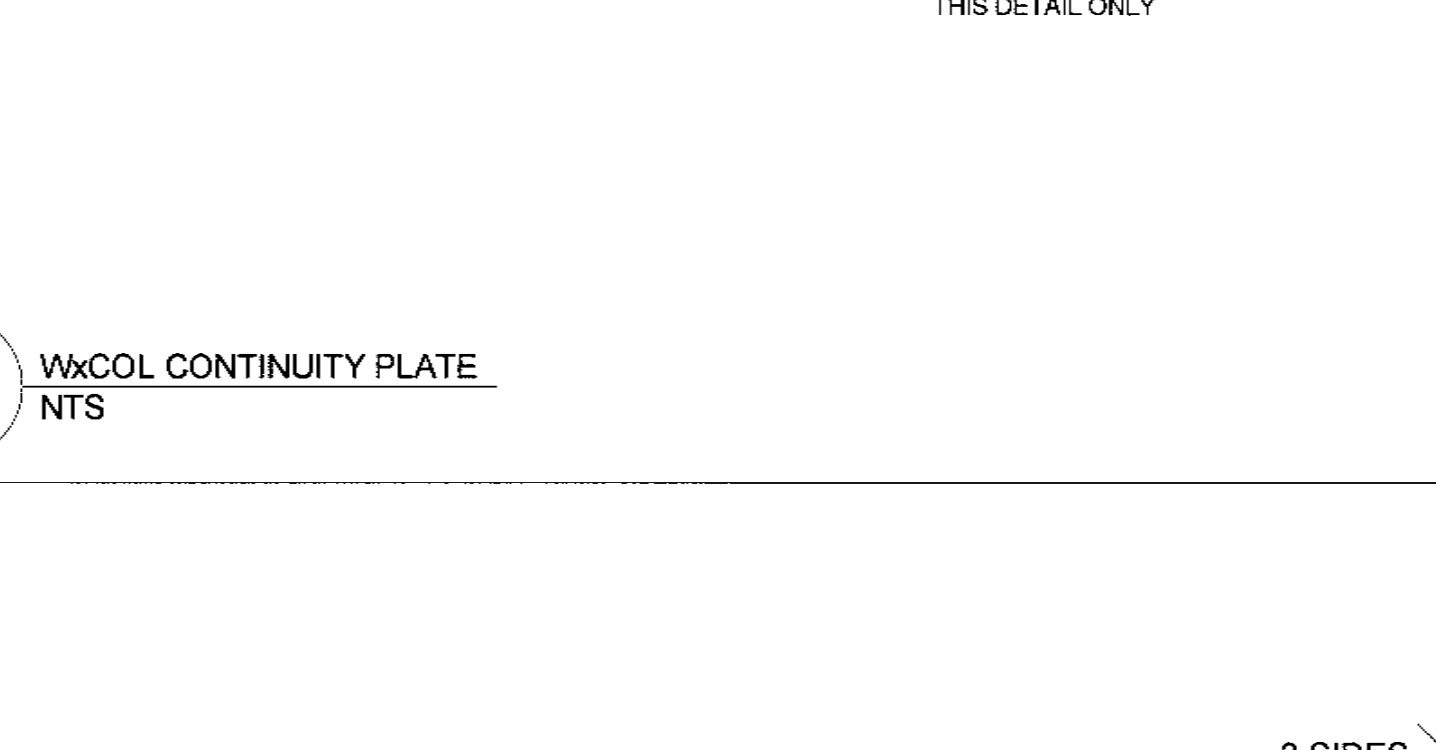
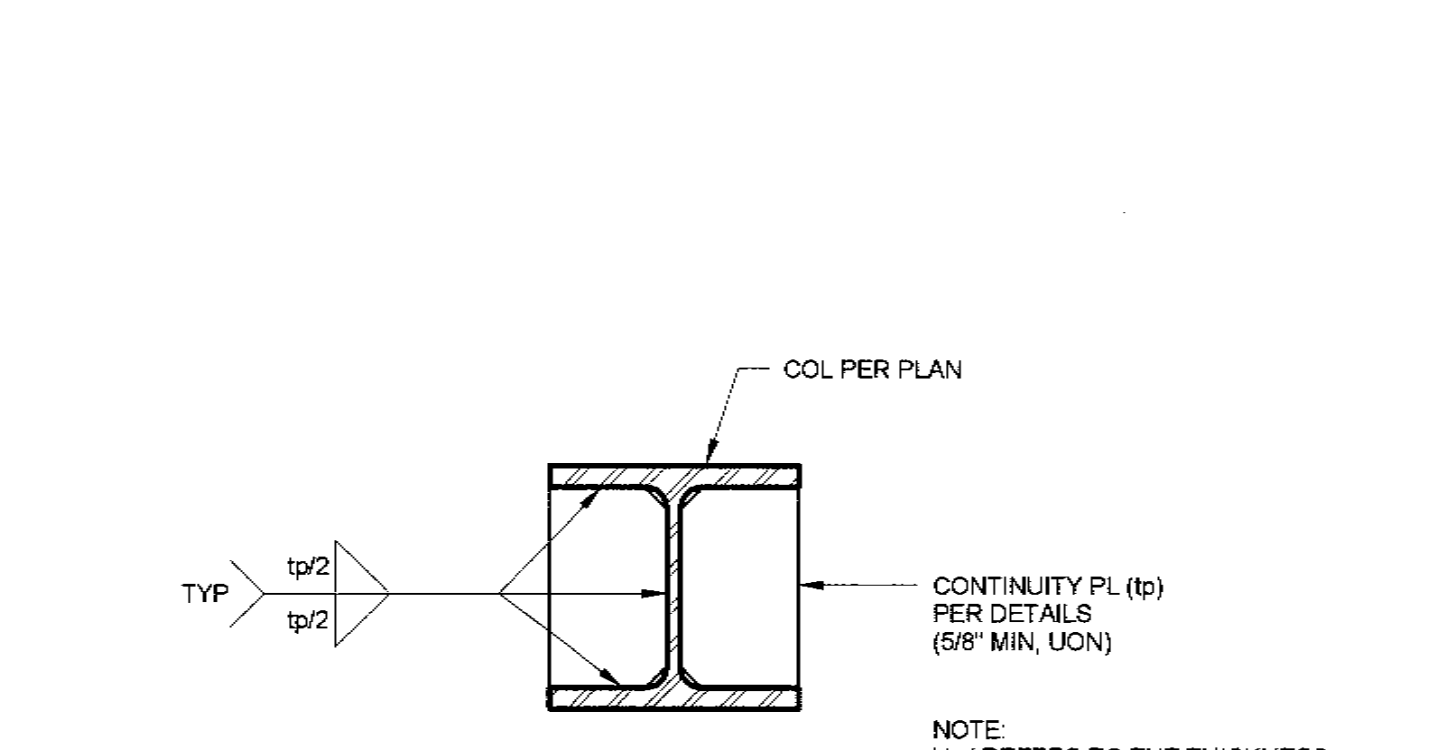
**10 PIPE TO COL
NTS**



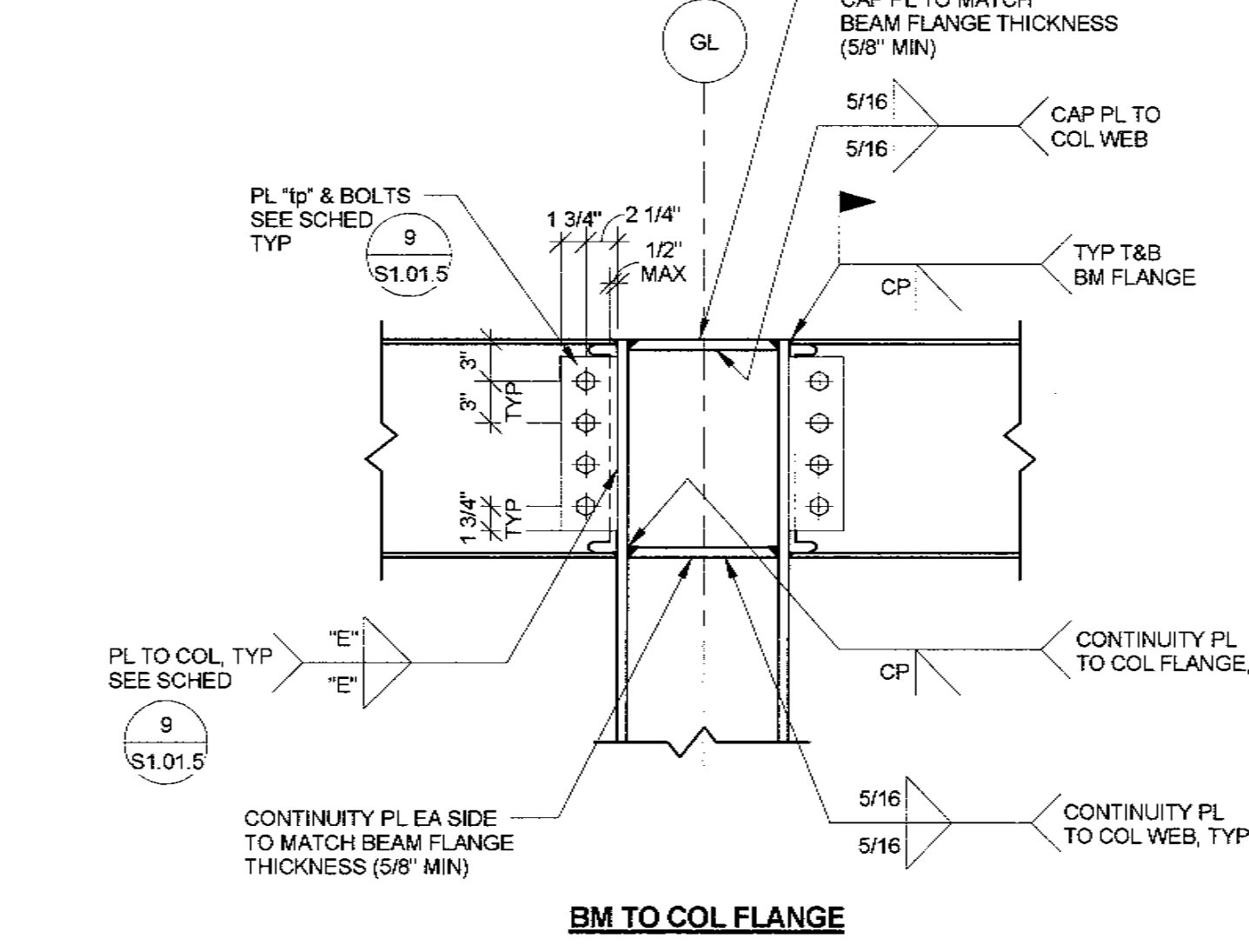
**6 BM TO COL WEB
NTS**



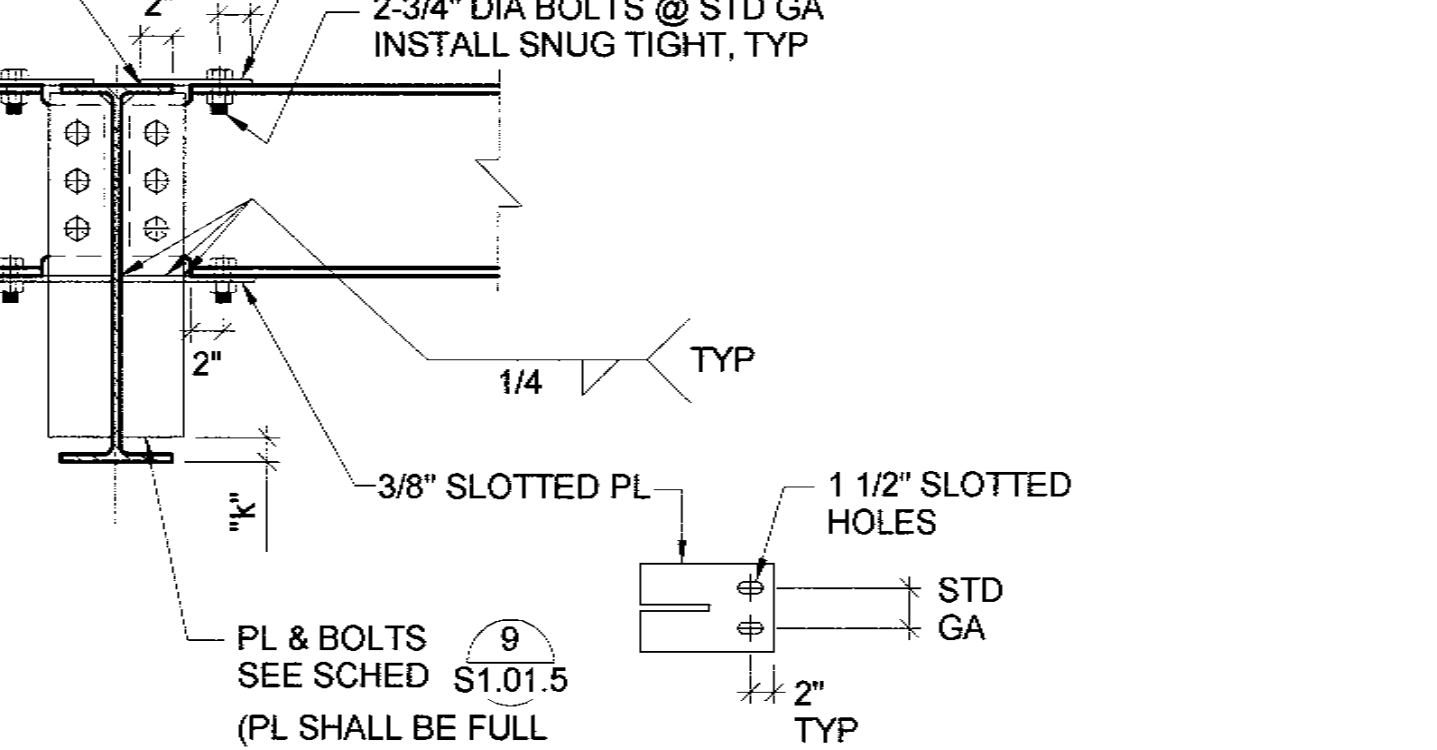
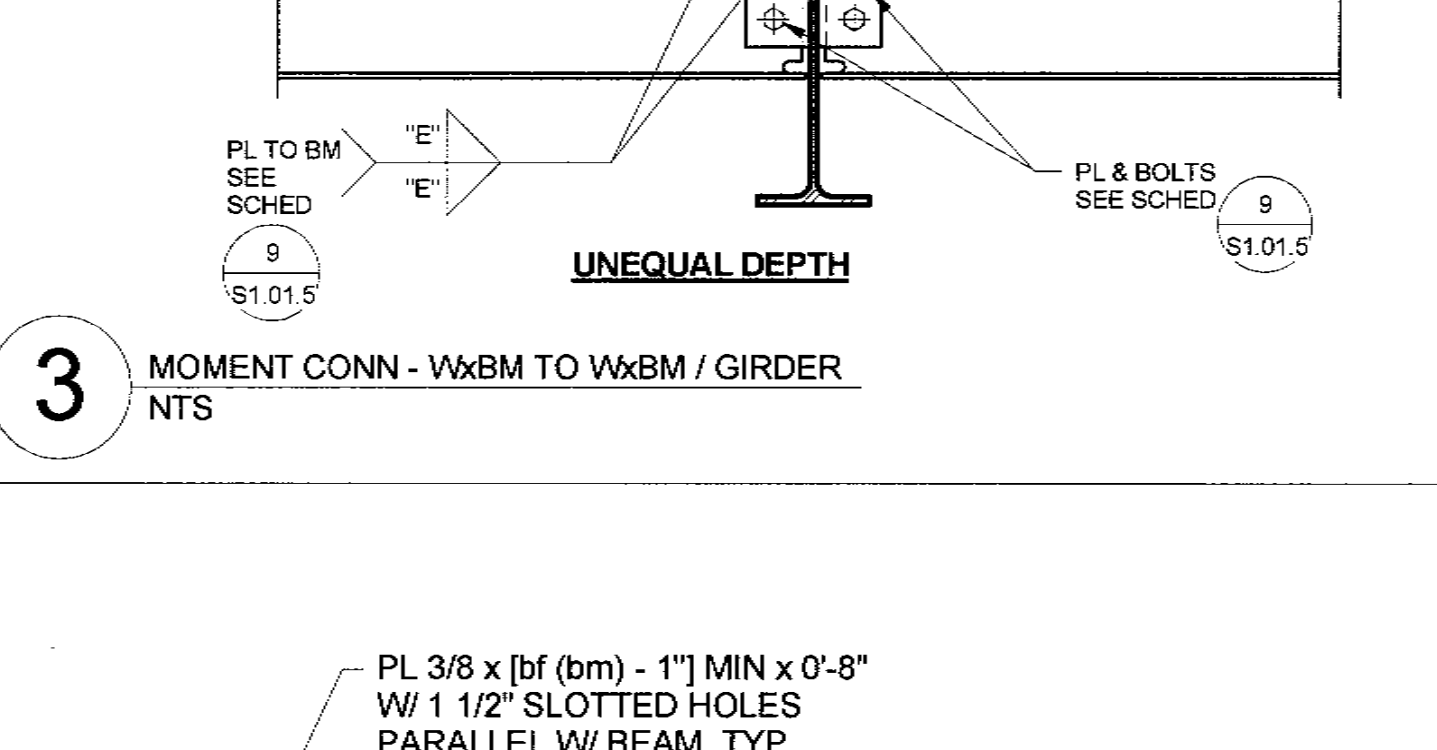
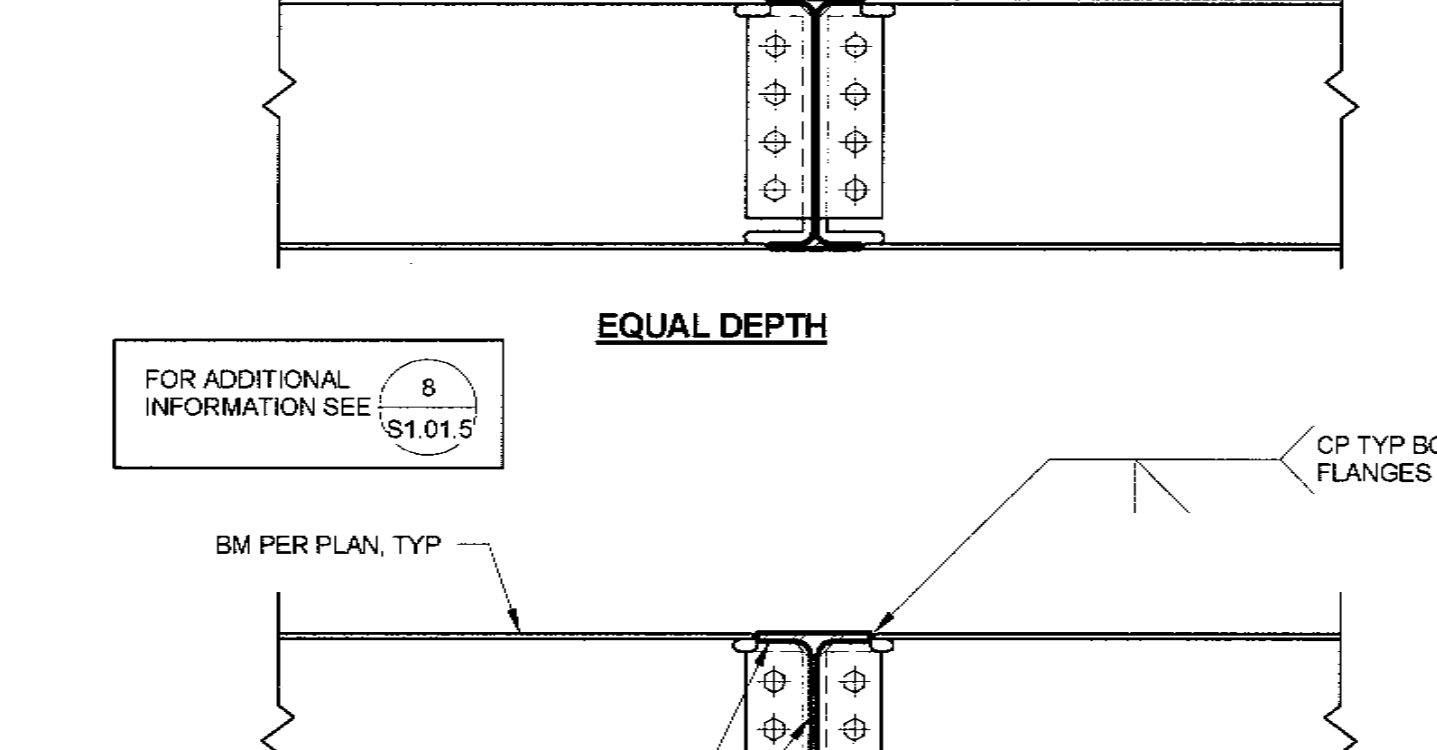
**7 WxCOL CONTINUITY PLATE
NTS**



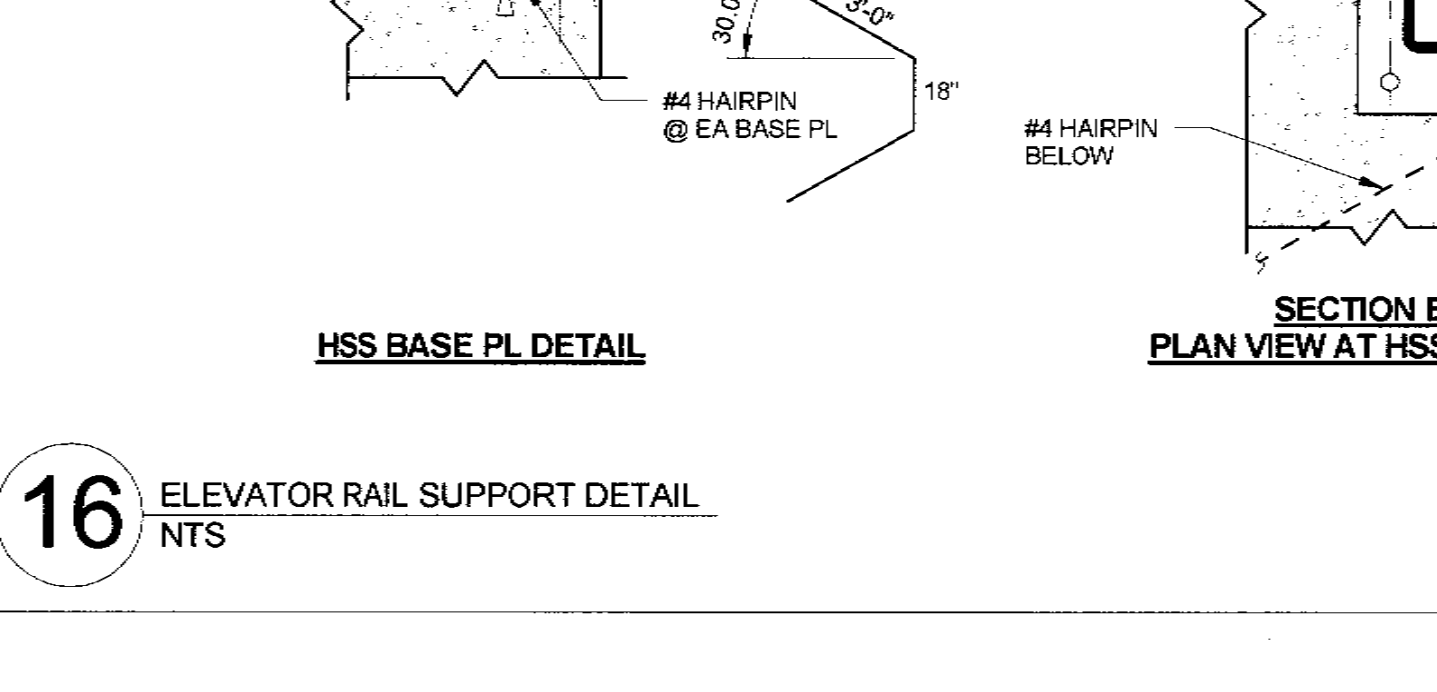
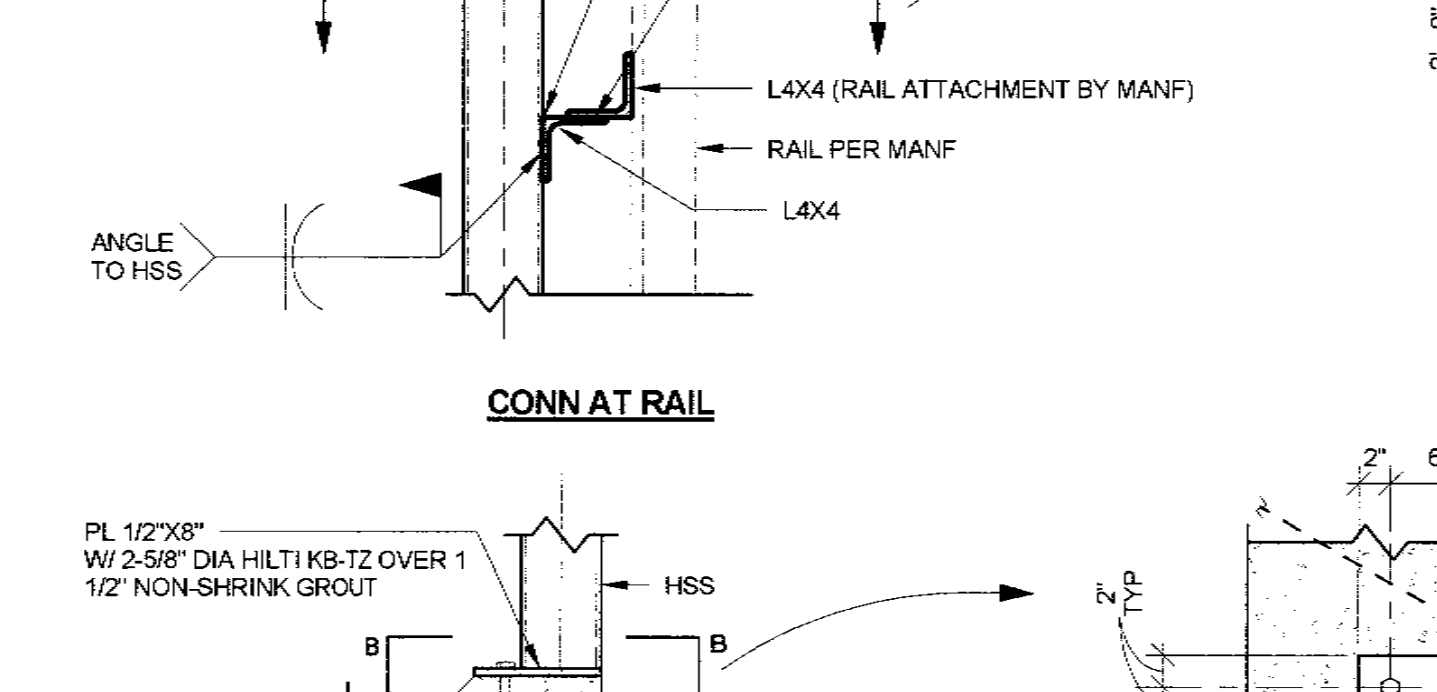
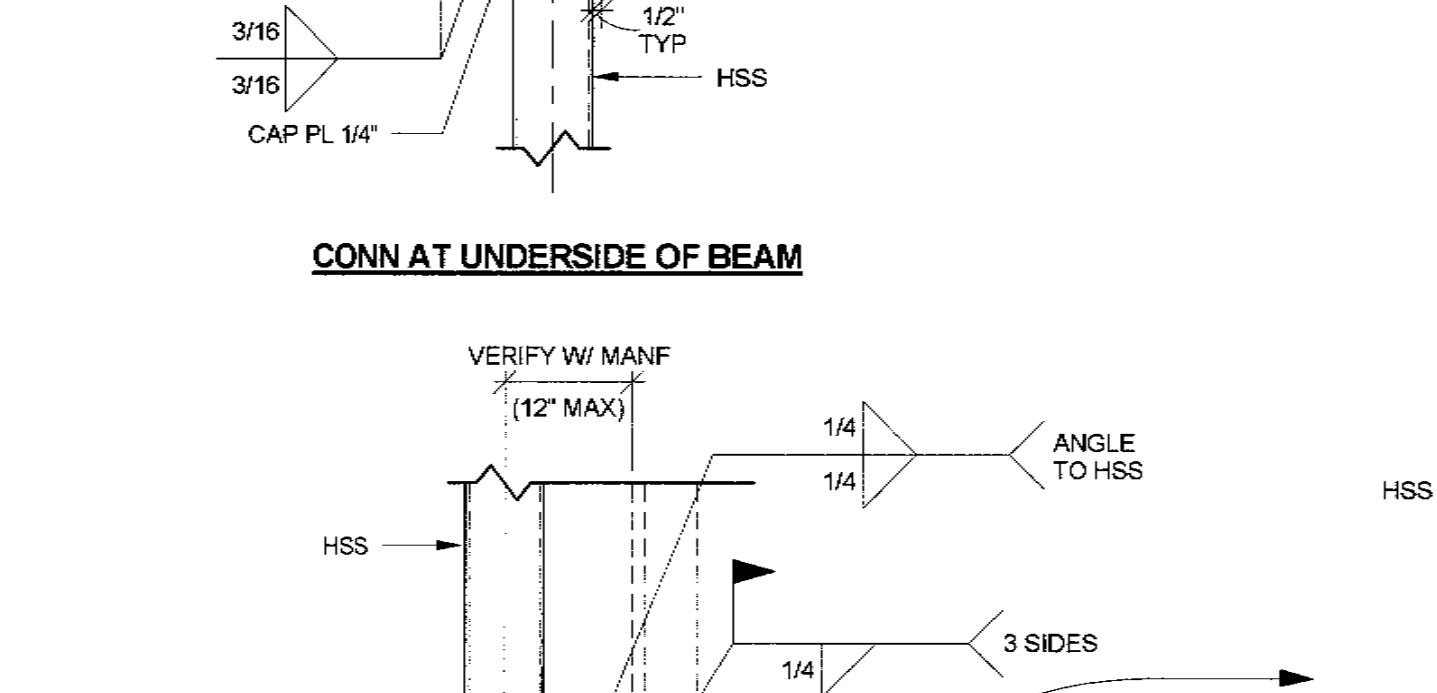
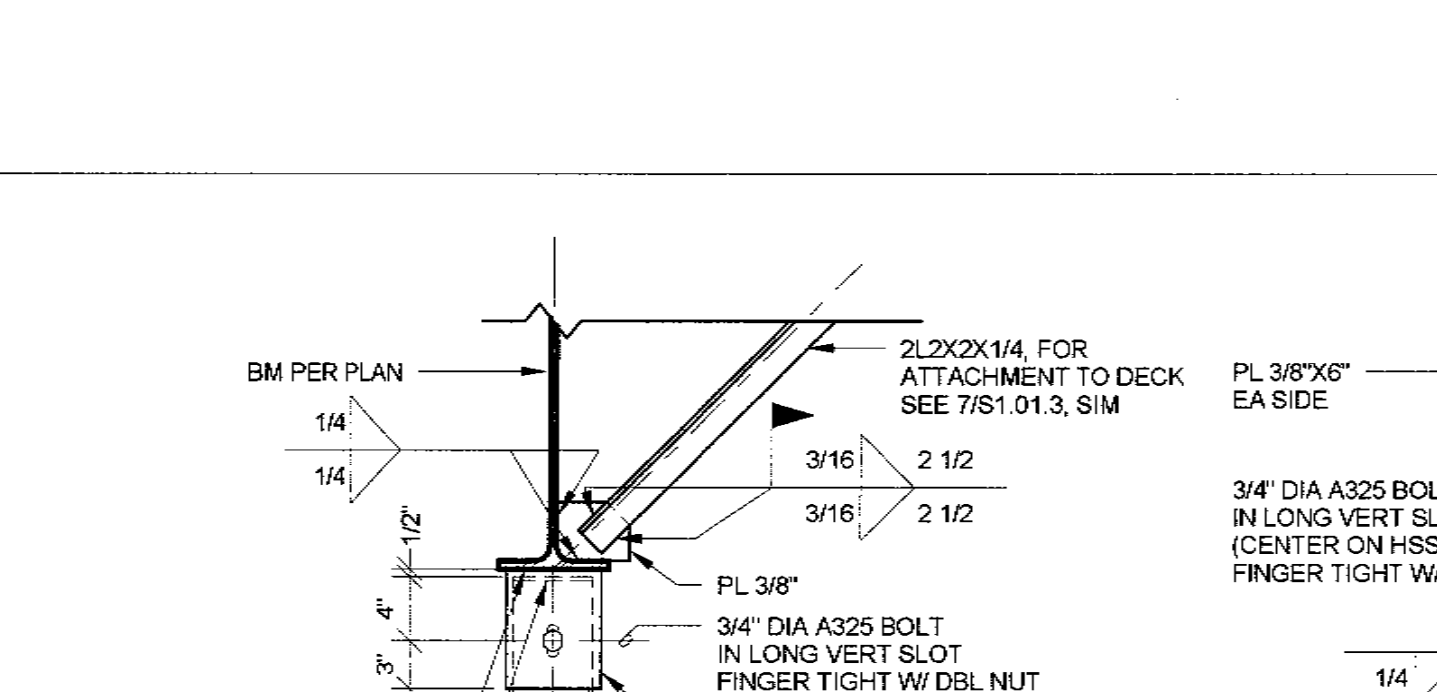
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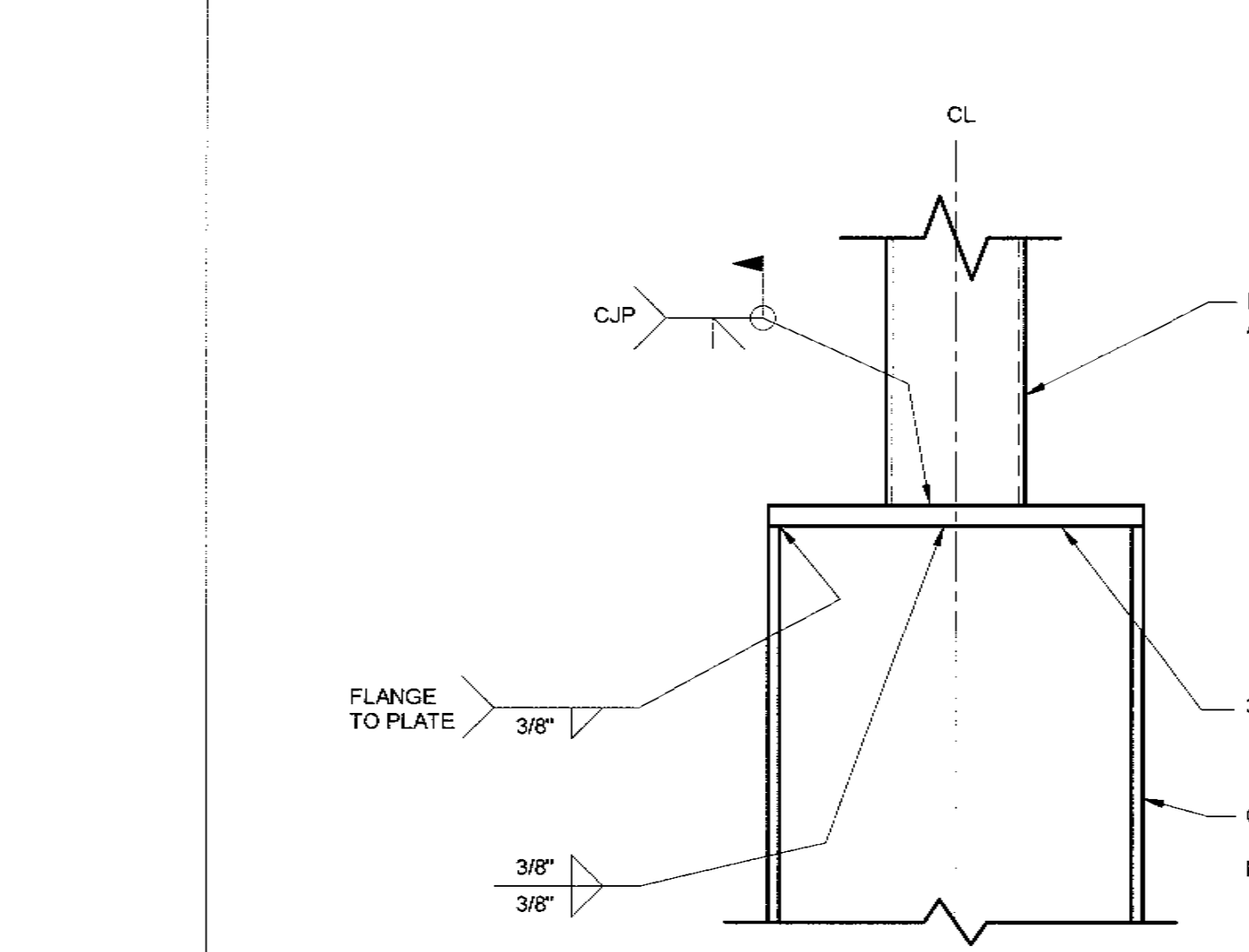
**3 MOMENT CONN - WxMB TO WxCOL AT ROOF
NTS**



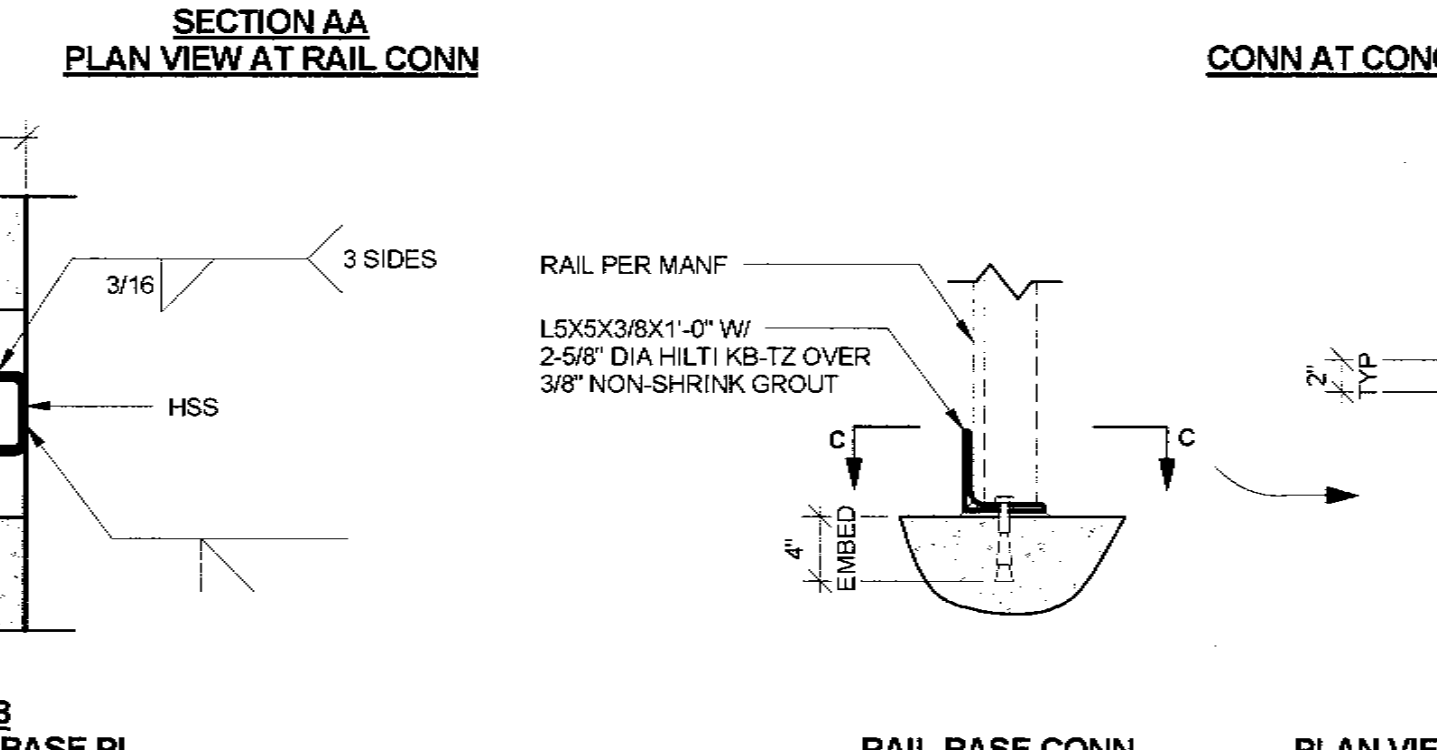
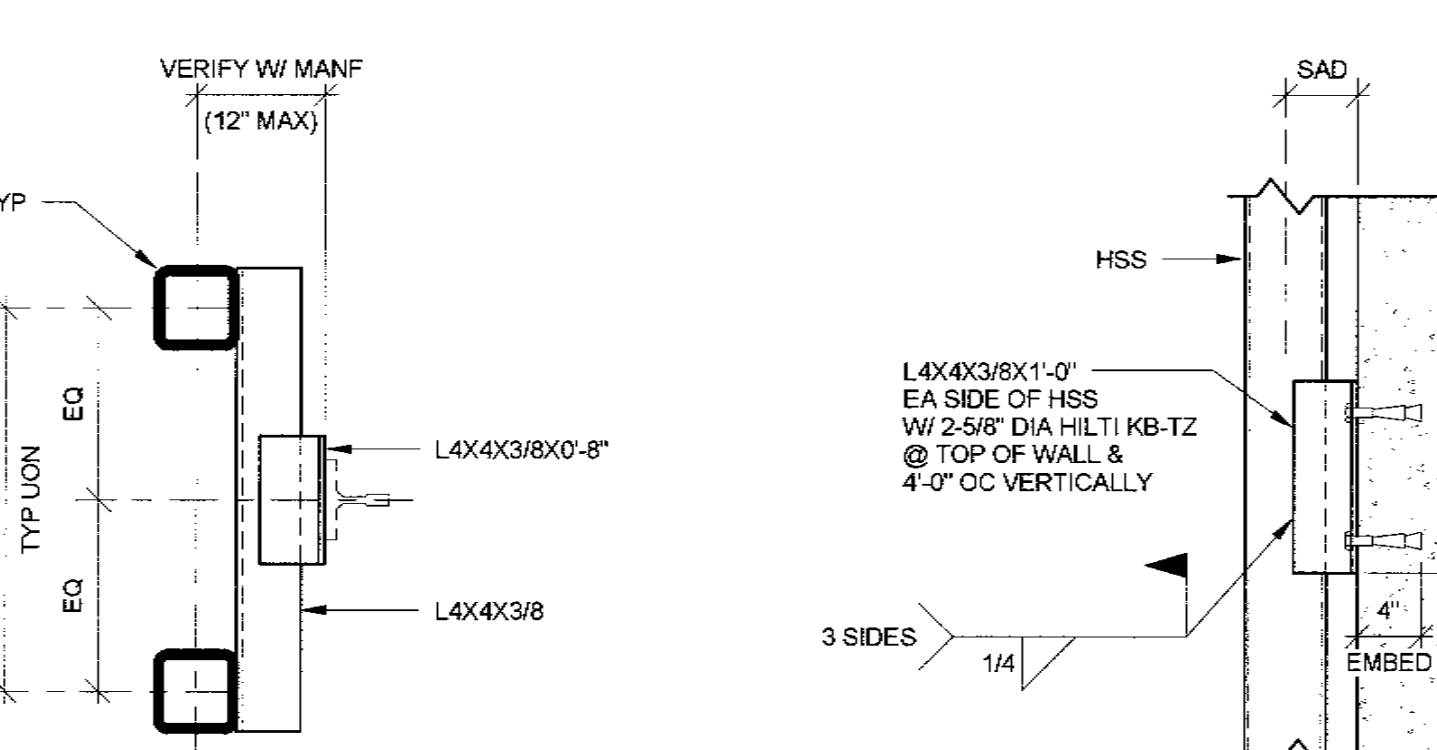
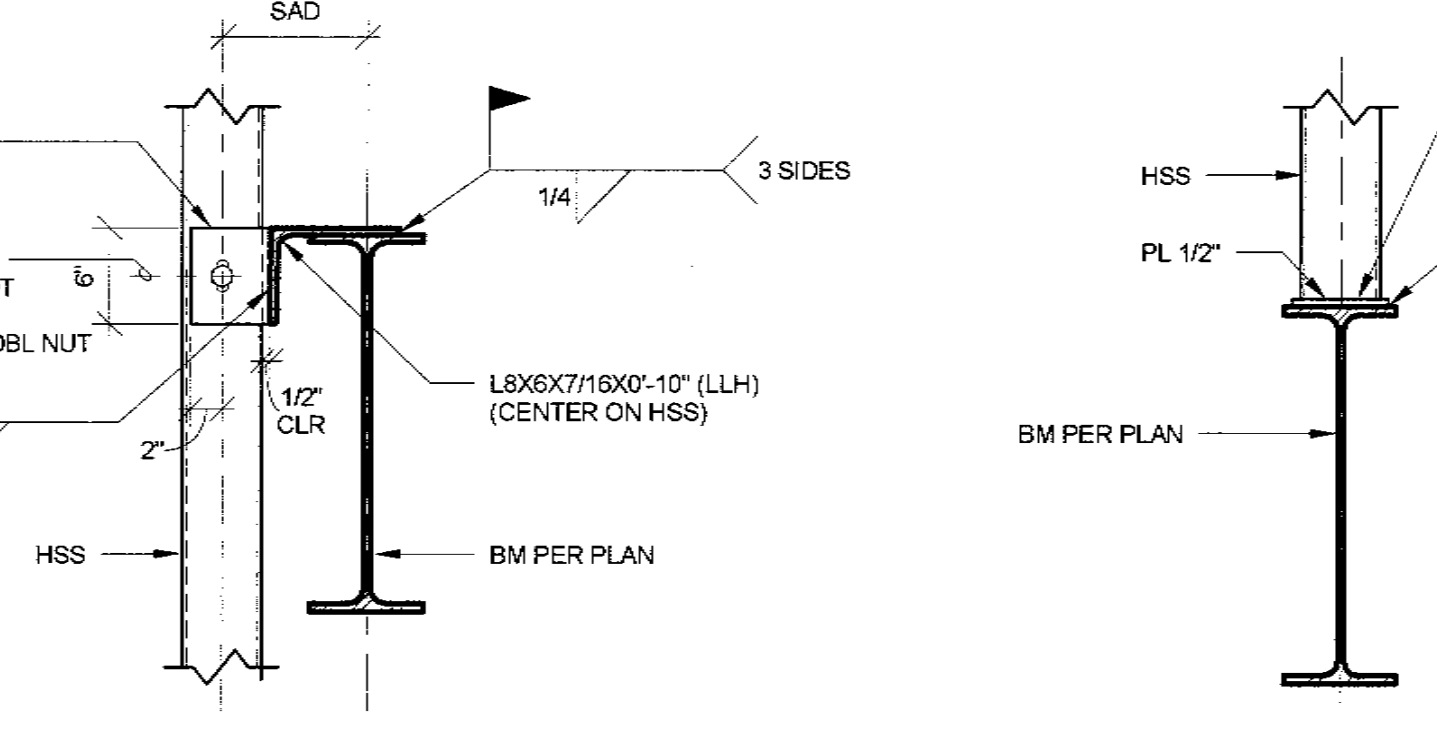
**16 ELEVATOR RAIL SUPPORT DETAIL
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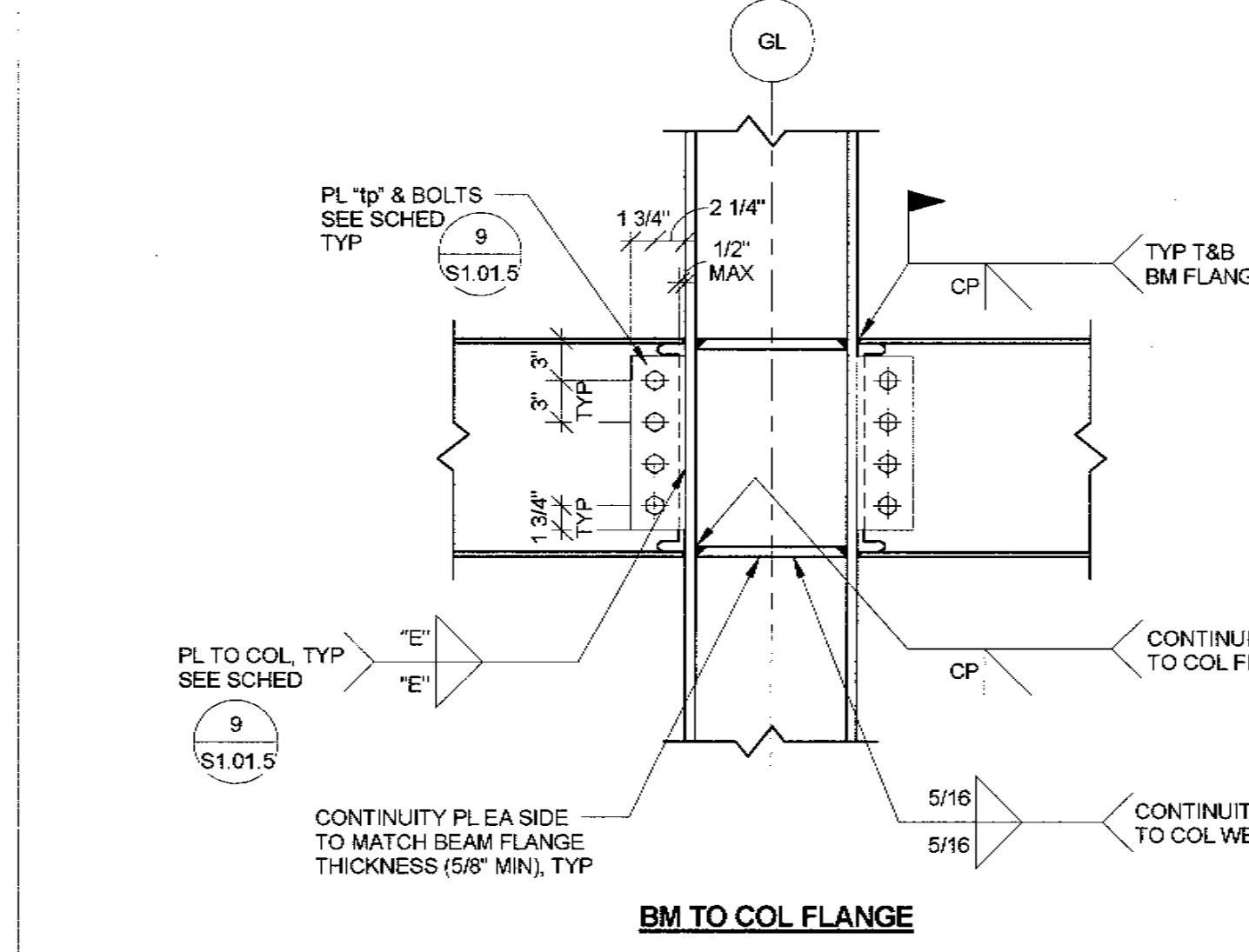
**9 FALL ARREST DETAIL
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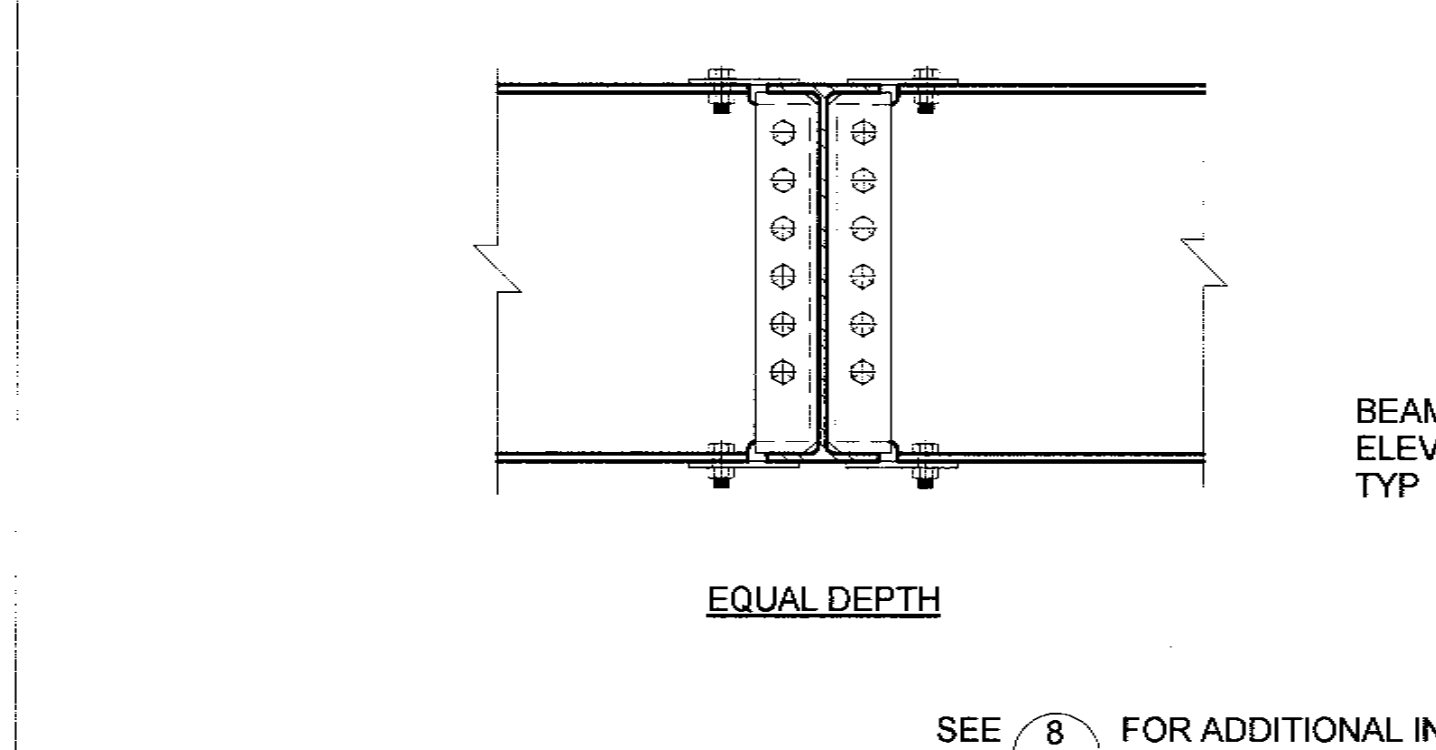
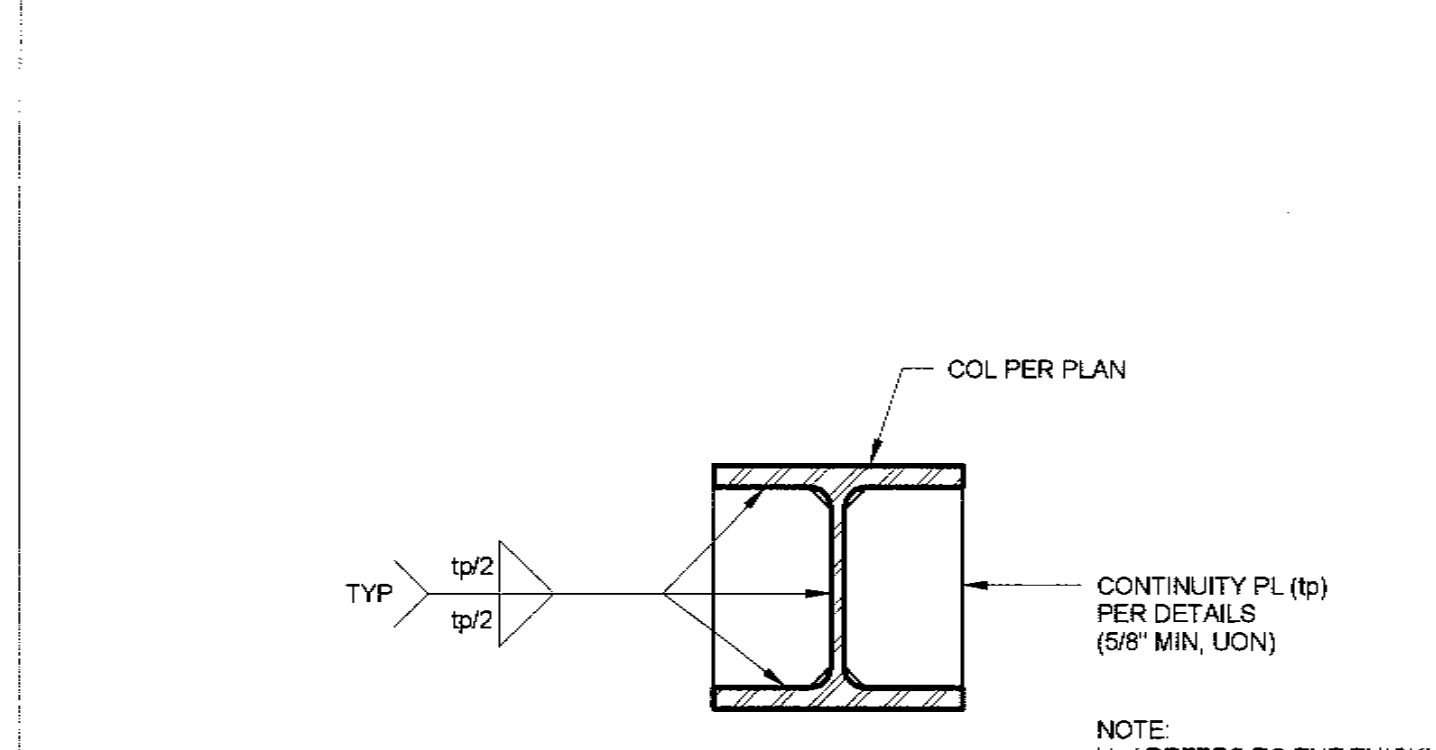
**10 PIPE TO COL
NTS**



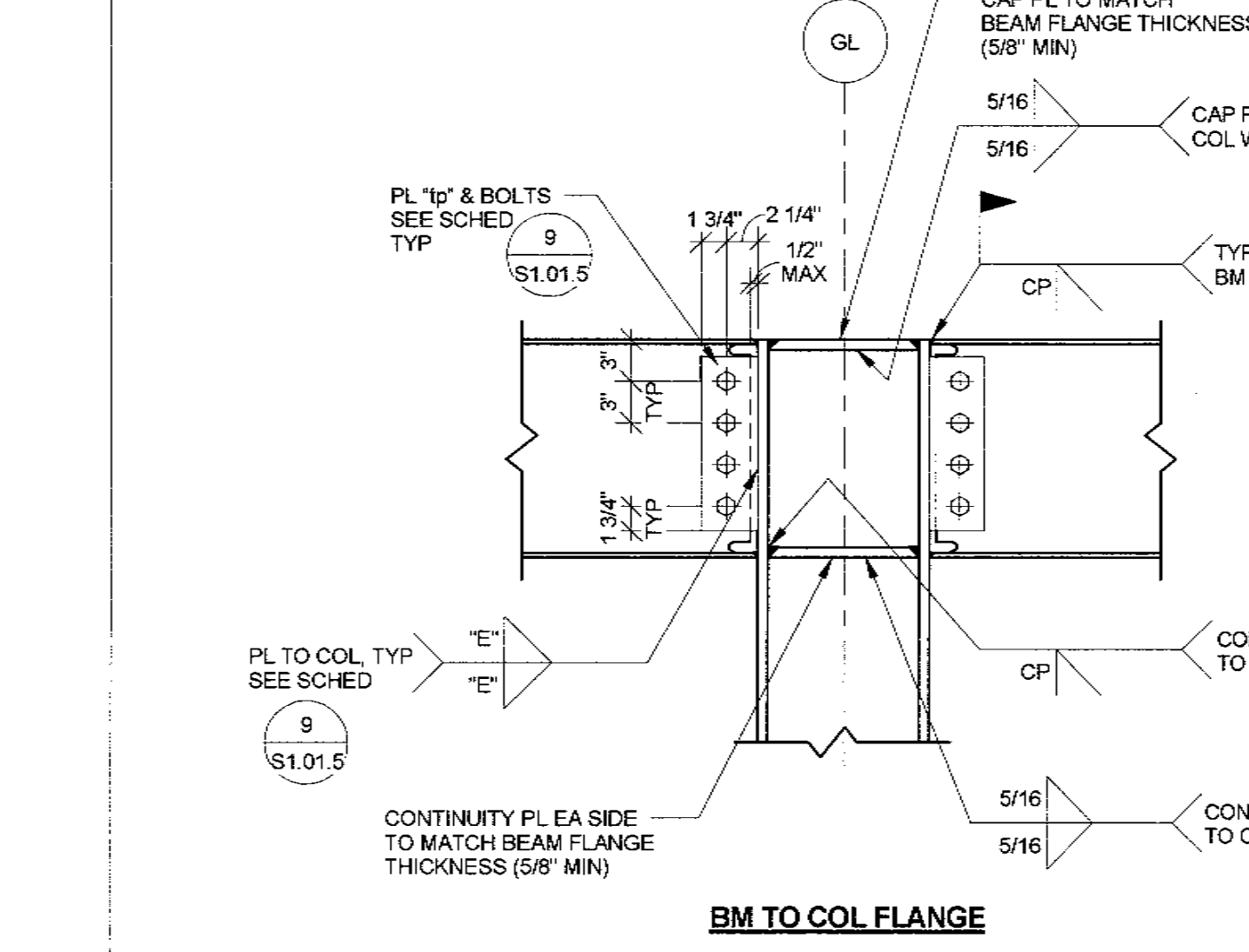
**6 BM TO COL WEB
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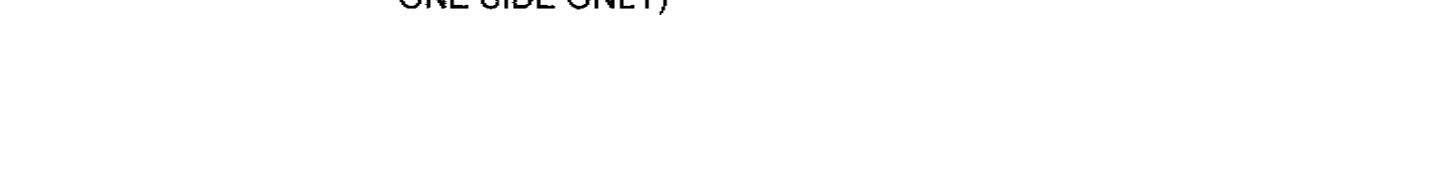
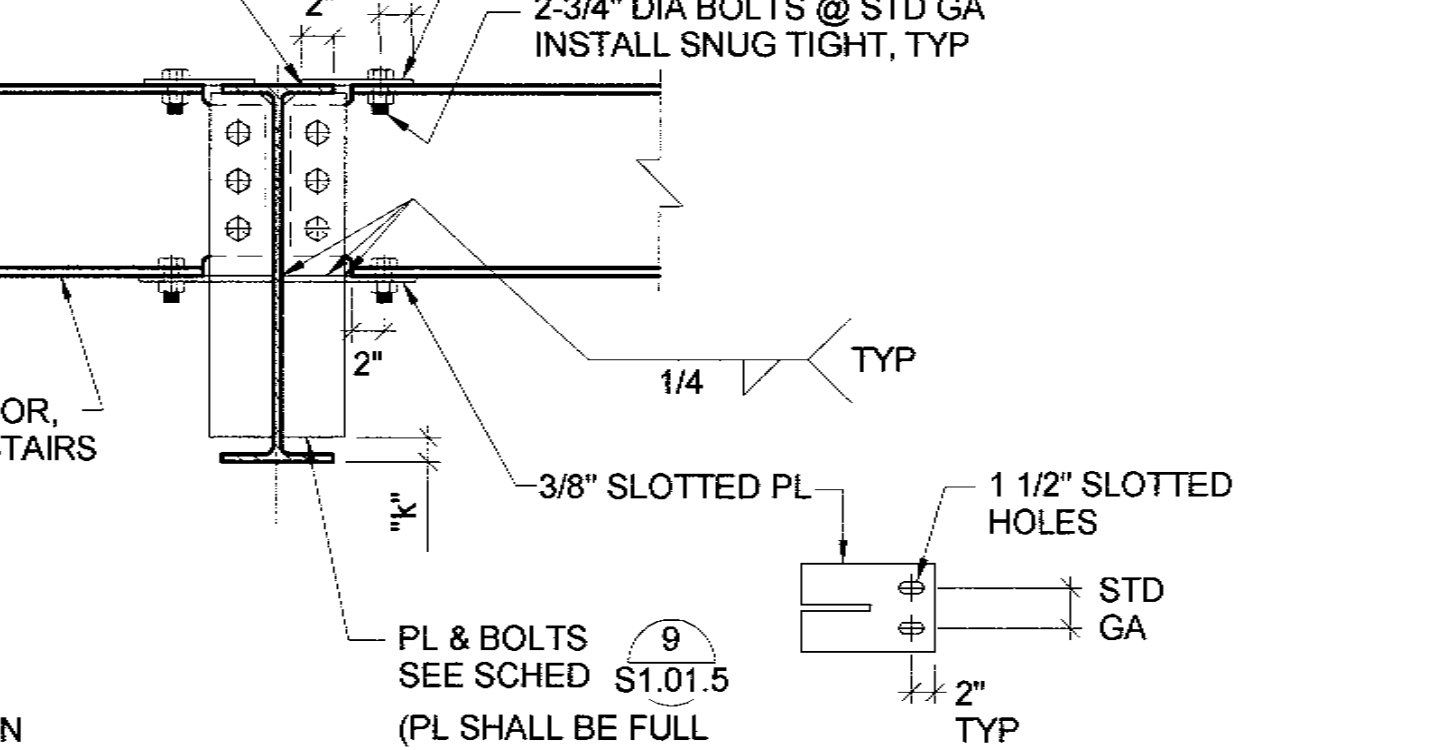
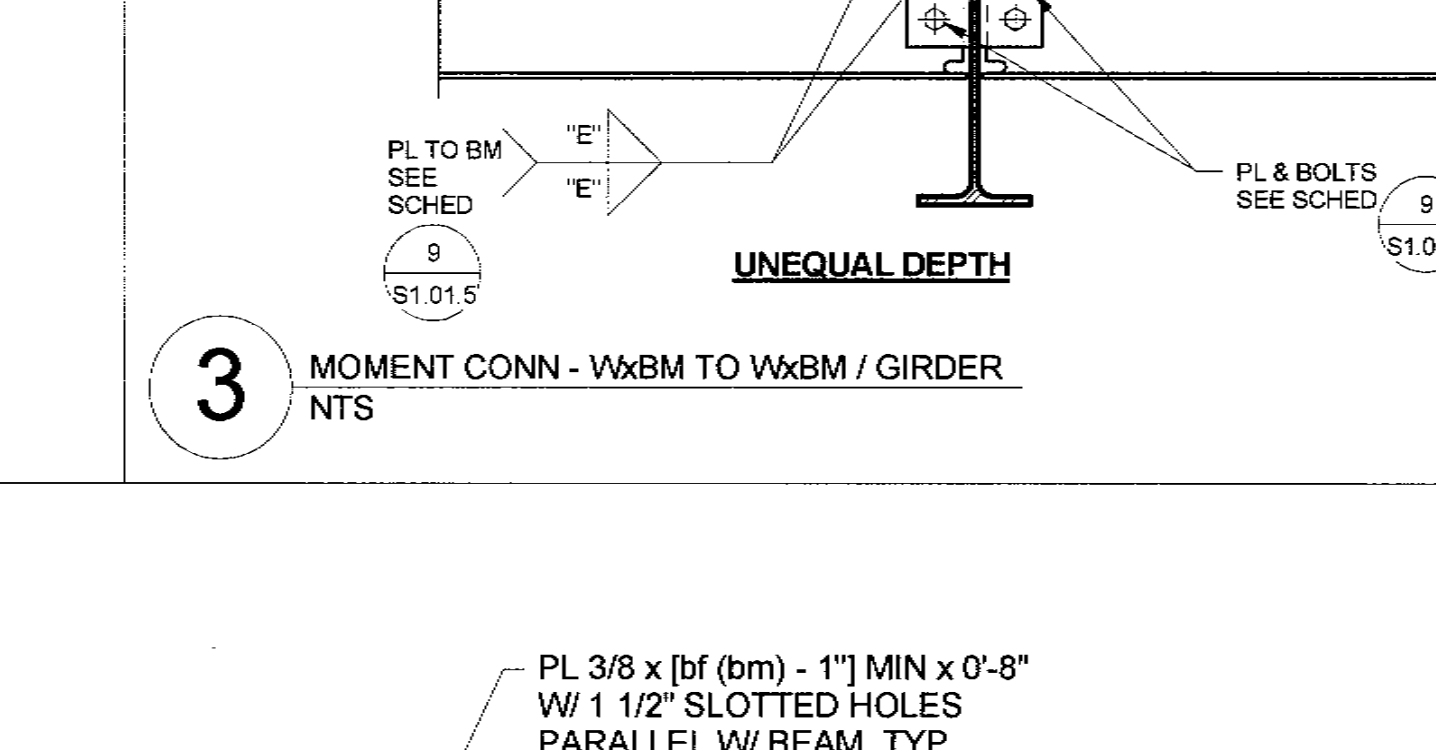
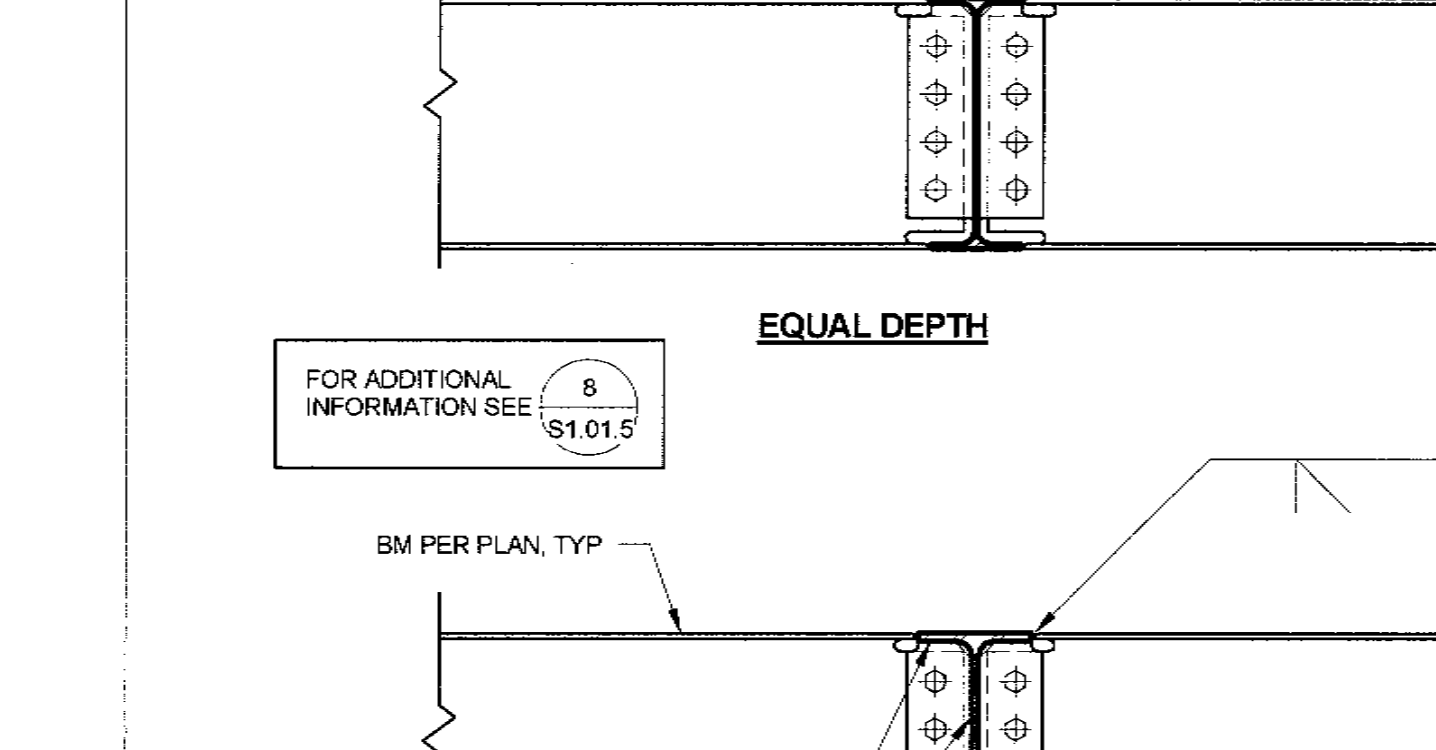
**7 WxCOL CONTINUITY PLATE
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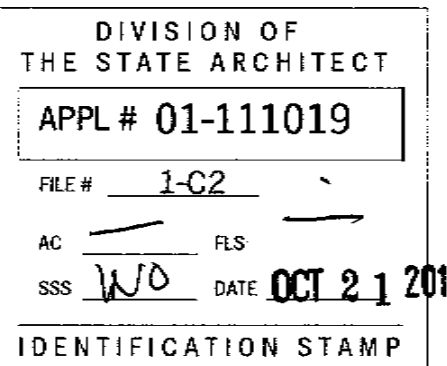


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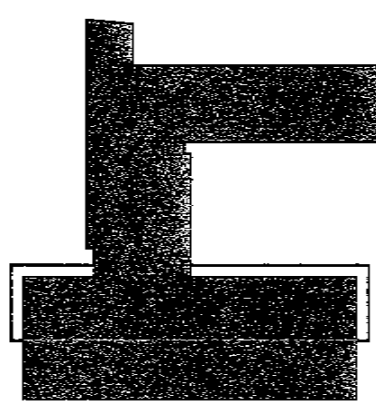
**3 MOMENT CONN - WxMB TO WxCOL AT ROOF
NTS**





09	10.14.2010	DSA BACKCHECK
08	07.15.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD - DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% SD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

REV NO DATE ISSUE



Student Services and Administration Building

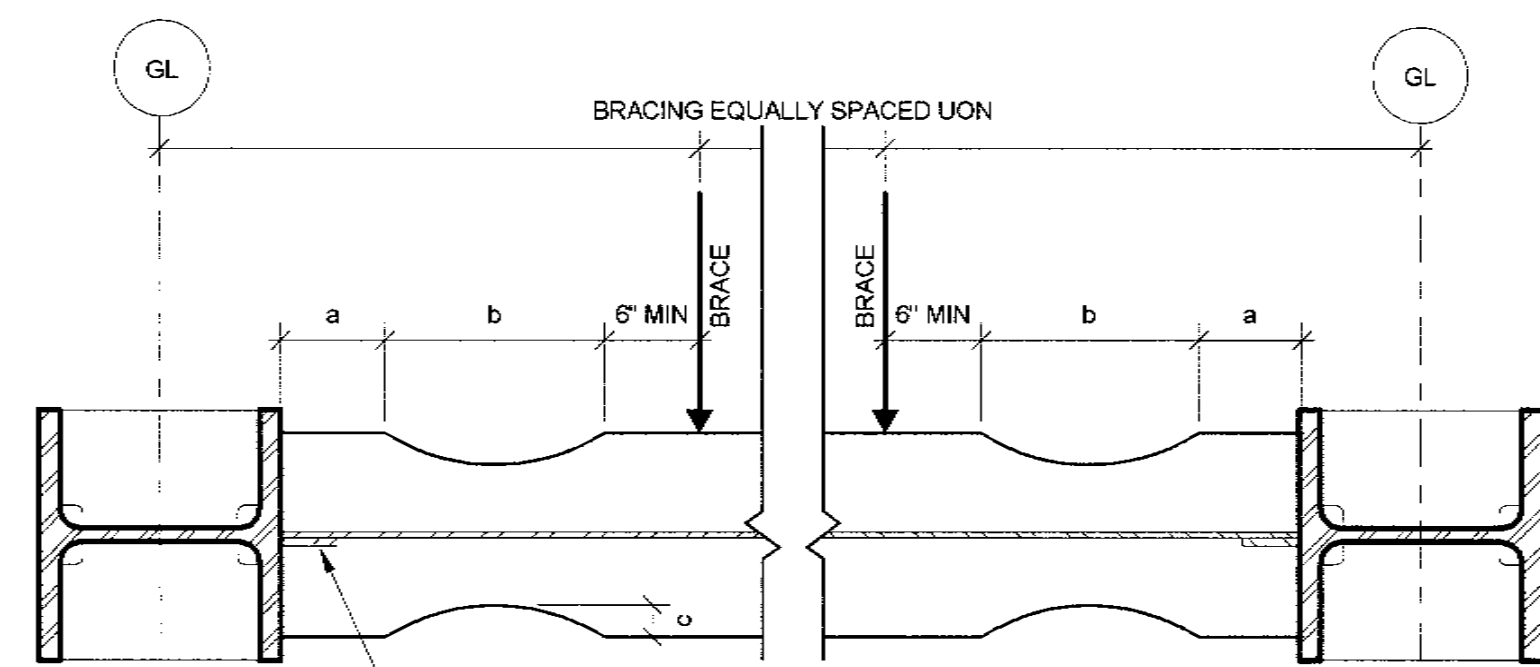
Las Positas College

3000 Campus Hill Dr.,
Livermore, CA 94551

TYPICAL STEEL DETAILS

PROJECT #: 06050.000
DATE: 02/10/2010
SCALE: NTS

S1.01.8

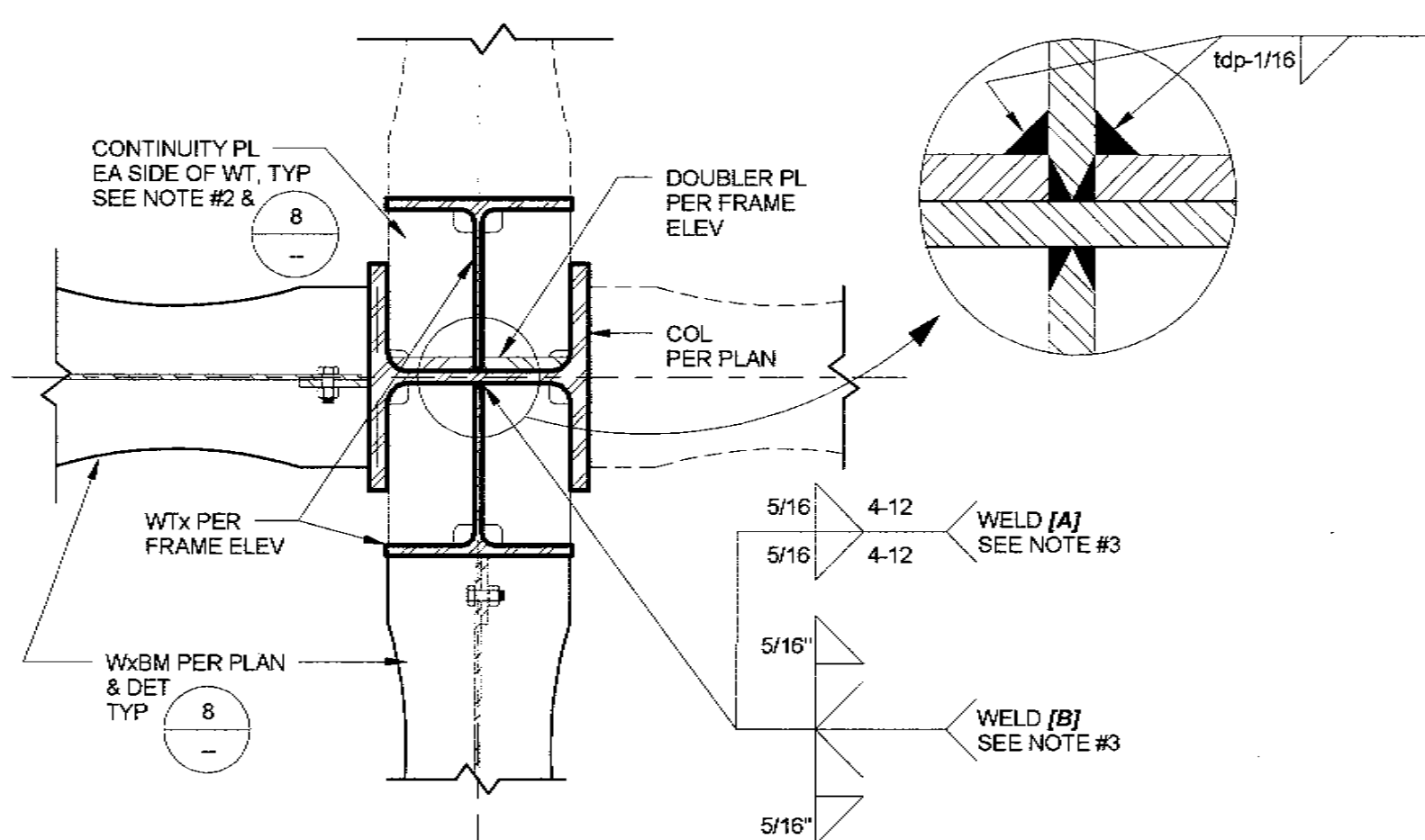


BEAM	MAX BRACE SPACING
W24x76	8'-0" OC
W27x54	8'-0" OC
W30x108	9'-0" OC

NOTES:

1. BEAM BRACING PROVIDED BY BEAMS FRAMING INTO SMRF BEAM DETAILED PER S1.01.5 & S1.01.6.

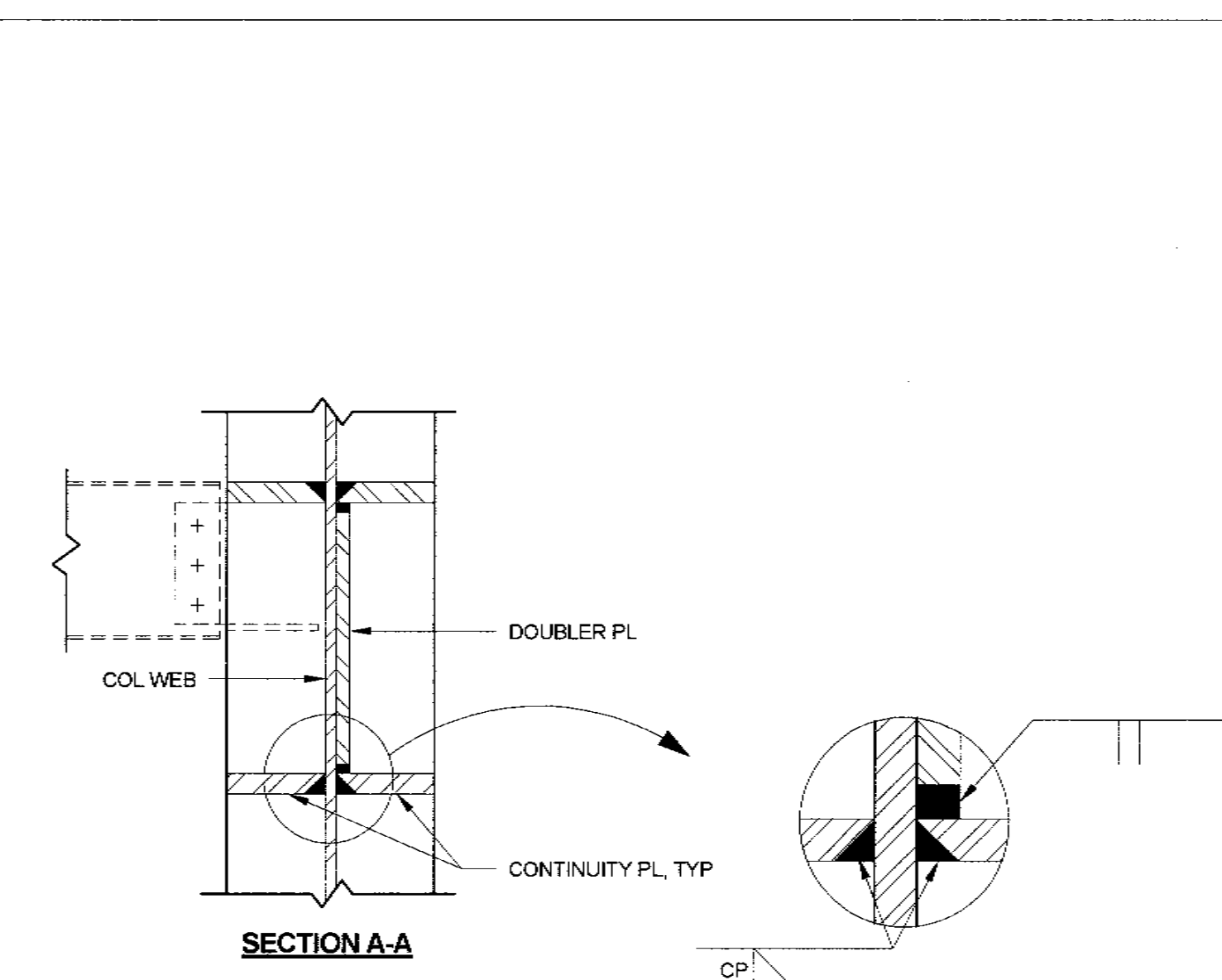
9 RBS BRACING LAYOUT & SPACING NTS



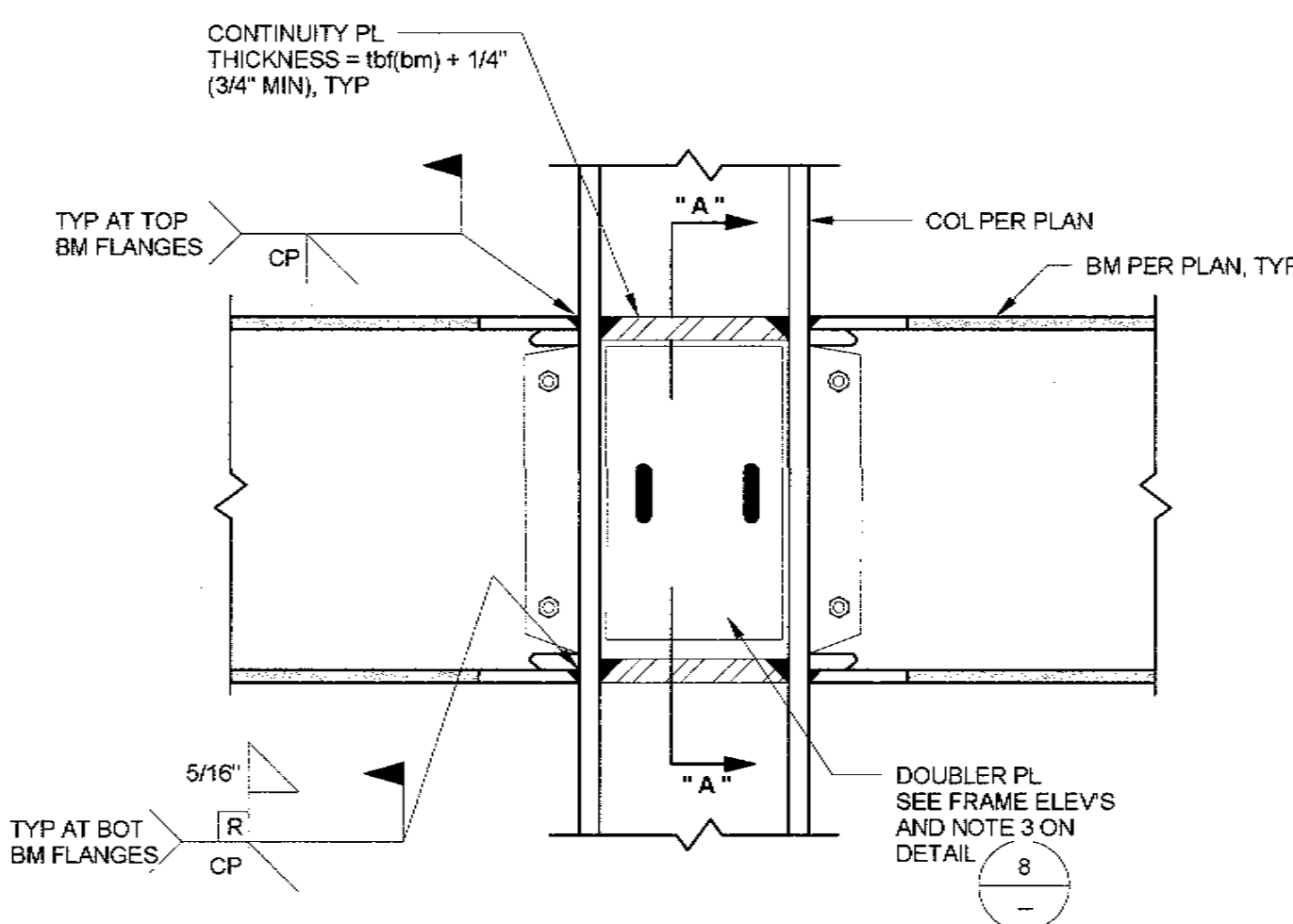
NOTES:

1. PROVIDE CONNECTION PER AISC 358-05.
2. PROVIDE CONTINUITY PLATE AT BEAM TO COL CONNECTION PER DET 8A AND @ 2'-0" OC MAX SPACING ALONG THE LENGTH OF THE WTX TO WK COL.
3. PROVIDE WELD (B) WITHIN THE ZONE FROM 12" ABOVE UPPER BEAM FLANGE TO 12" BELOW LOWER BEAM FLANGE. PROVIDE WELD (A) ELSEWHERE.
4. FOR ADDITIONAL INFORMATION SEE 8A.

10 BIAXIAL SMRF RBS W/ WTX CONN NTS

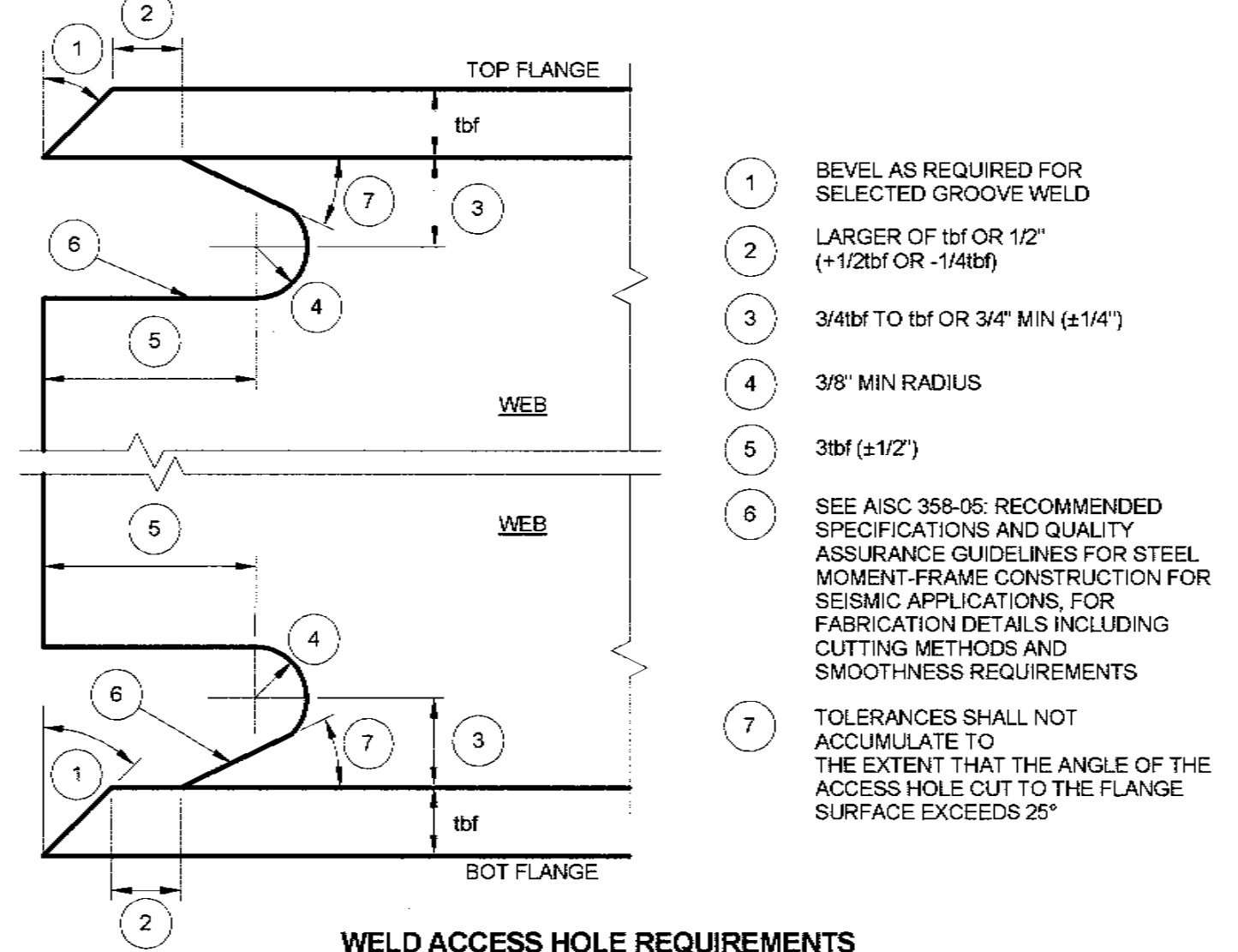


SECTION A-A

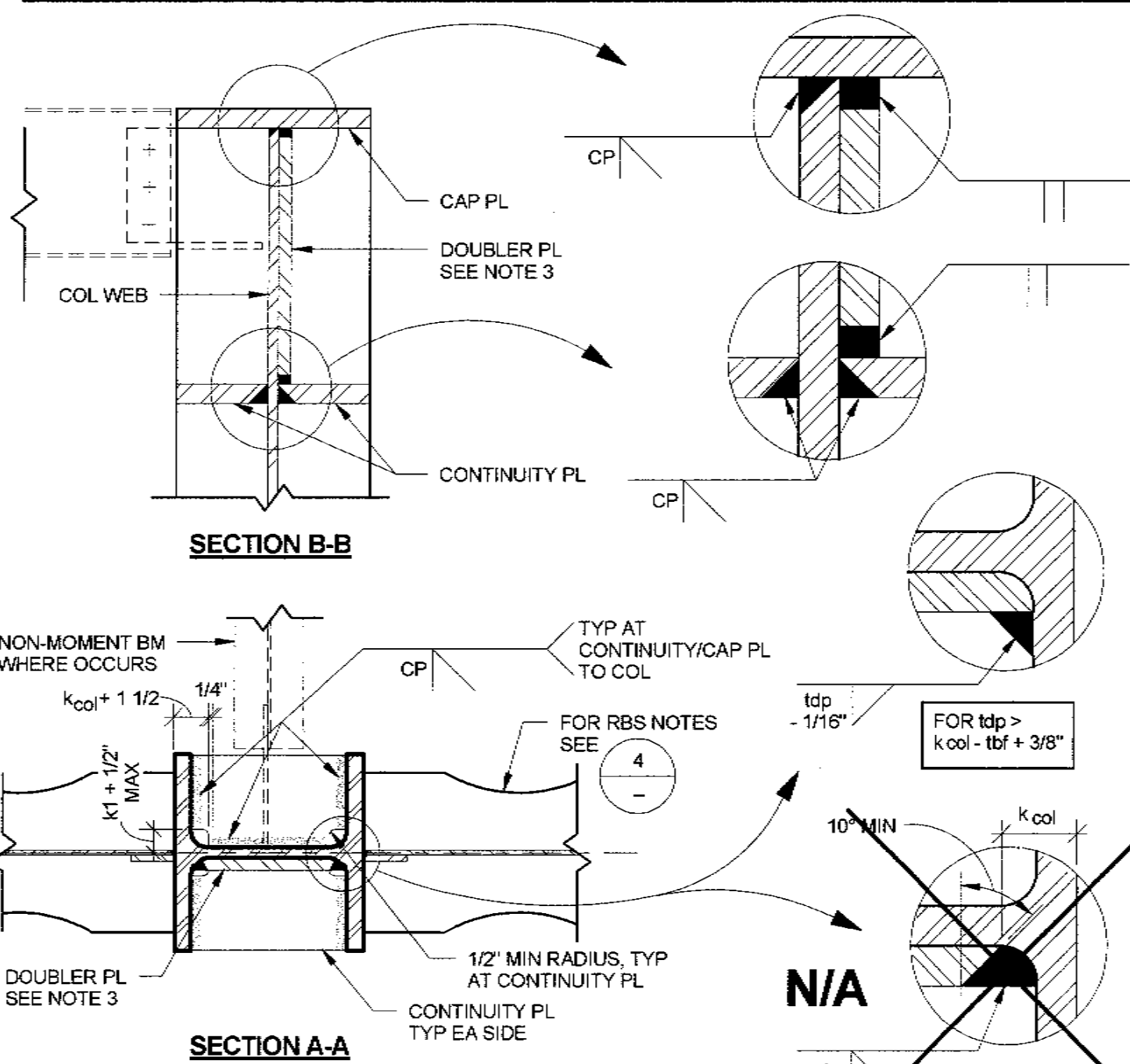


NOTE: FOR ADDITIONAL INFORMATION NOT SHOWN, SEE DETAIL 8.

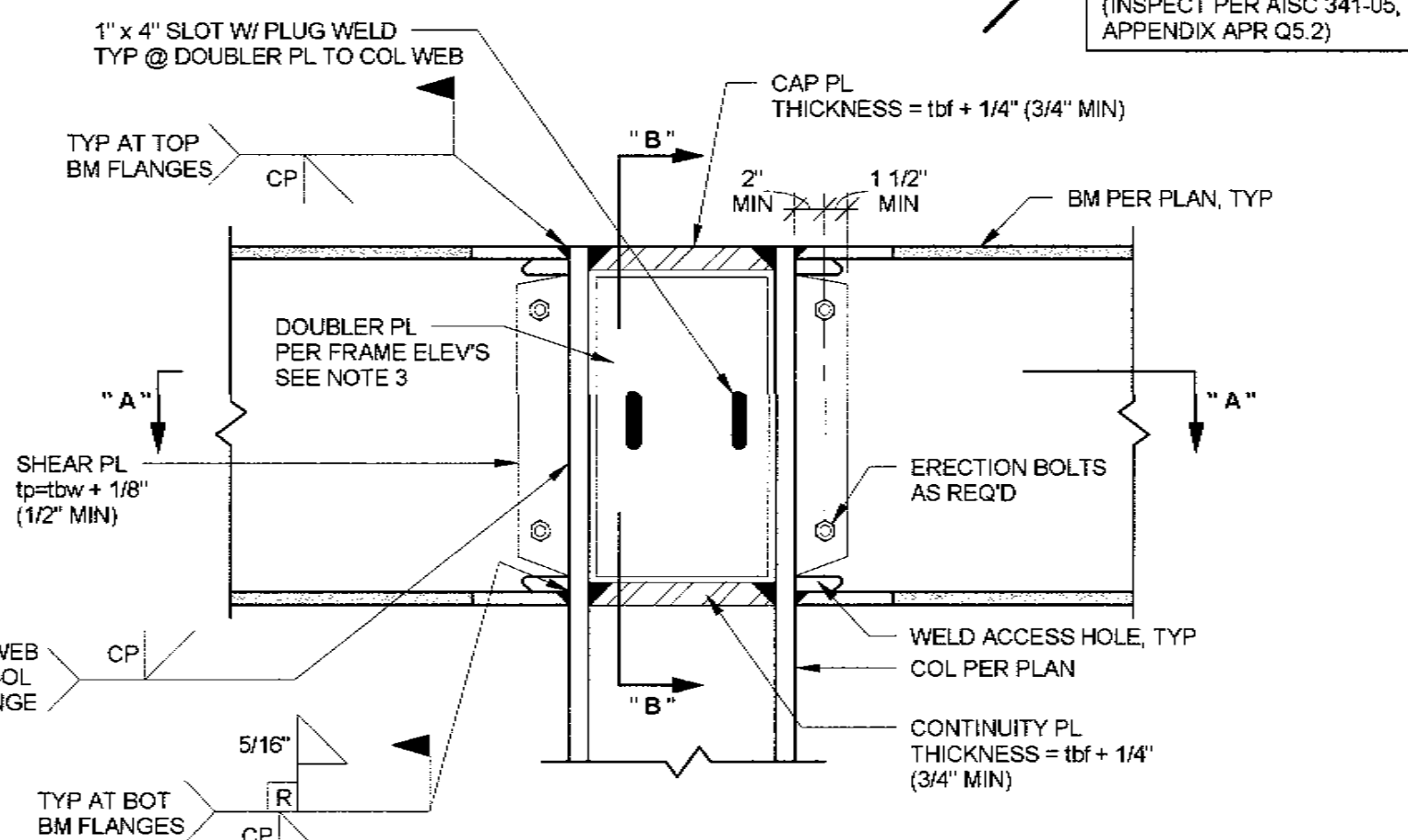
12 SMRF RBS-FLOOR BEAM TO COLUMN FLANGE NTS



WELD ACCESS HOLE REQUIREMENTS



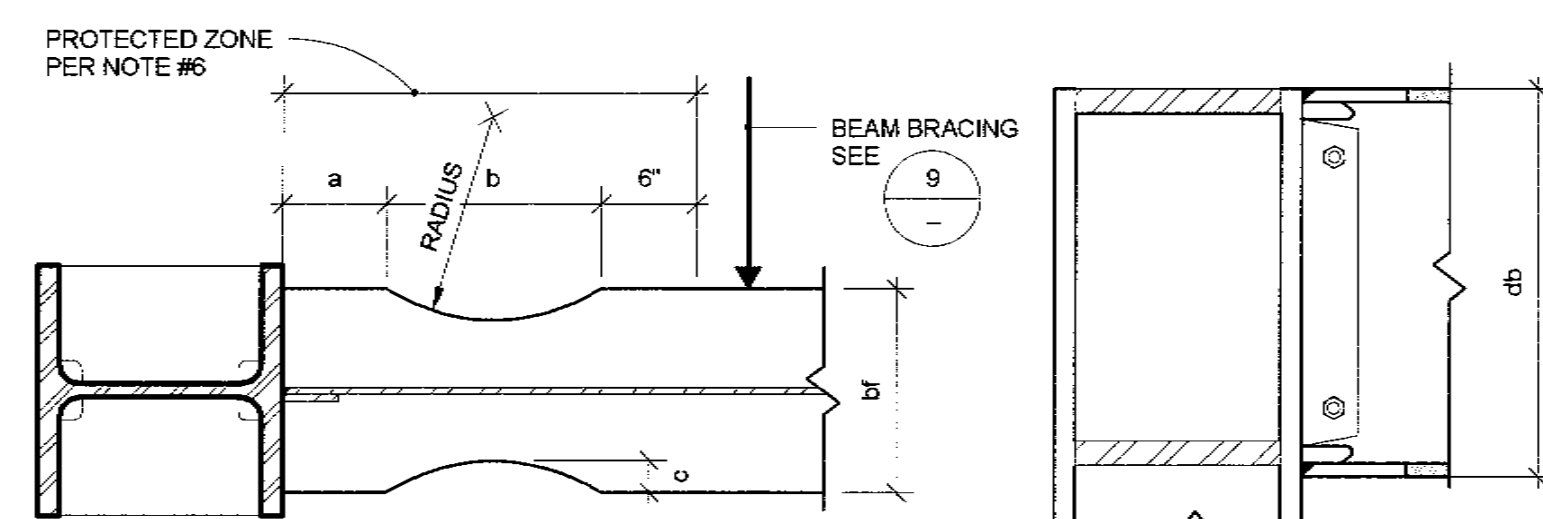
SECTION A-A



1. 'tbf' REFERS TO BM FLANGE THICKNESS. 'tdp' REFERS TO DOUBLER PLATE THICKNESS. 'tp' REFERS TO PLATE THICKNESS. 'tdw' REFERS TO BM WEB THICKNESS.
2. REFER TO AISC FOR VALUES FOR Kcol, K1 & tbf.
3. LOCATE DOUBLER PL ON SIDE OF COLUMN WHERE NO BEAM OR SMALLER BEAM FRAMES INTO COLUMN WEB.
4. SEE PLAN AND FRAMING ELEV.
5. SHEAR PL SHALL EXTEND VERTICALLY BETWEEN WELD ACCESS HOLES AND BE USED AS BACKING FOR CJP WELD BETWEEN BM WEB AND COL FLANGE.
6. ALL PLATES TO BE ASTM A572 GR-50.
7. FOR CONDITION AT FLOOR, SEE DETAIL 12L.
8. FOR ADDITIONAL NOTES AND REQUIREMENTS, SEE DETAIL 2A.

8 SMRF RBS-ROOF BEAM TO COLUMN FLANGE NTS

2 STEEL MOMENT RESISTING FRAME (SMRF) CONN NOTES NTS

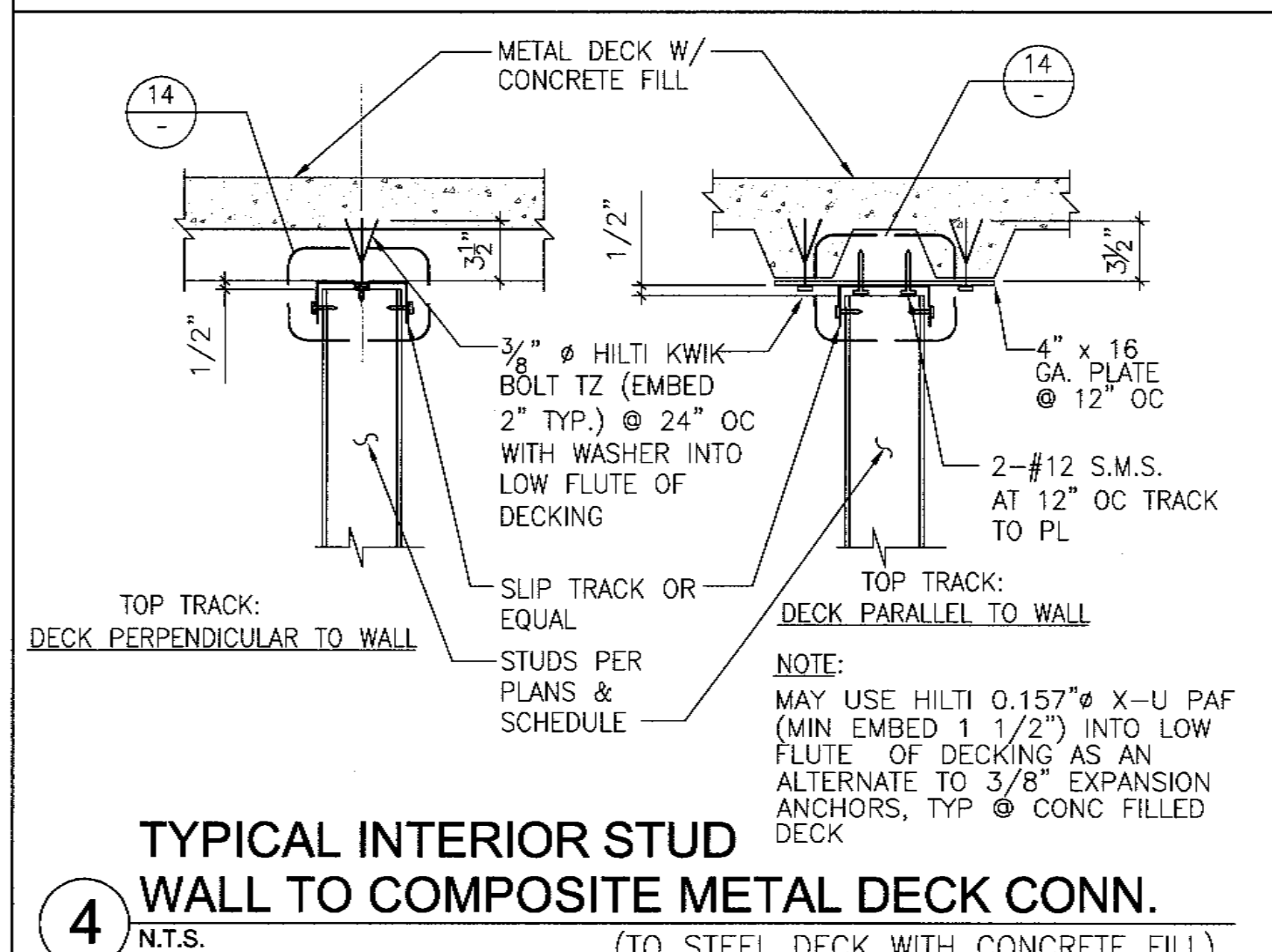
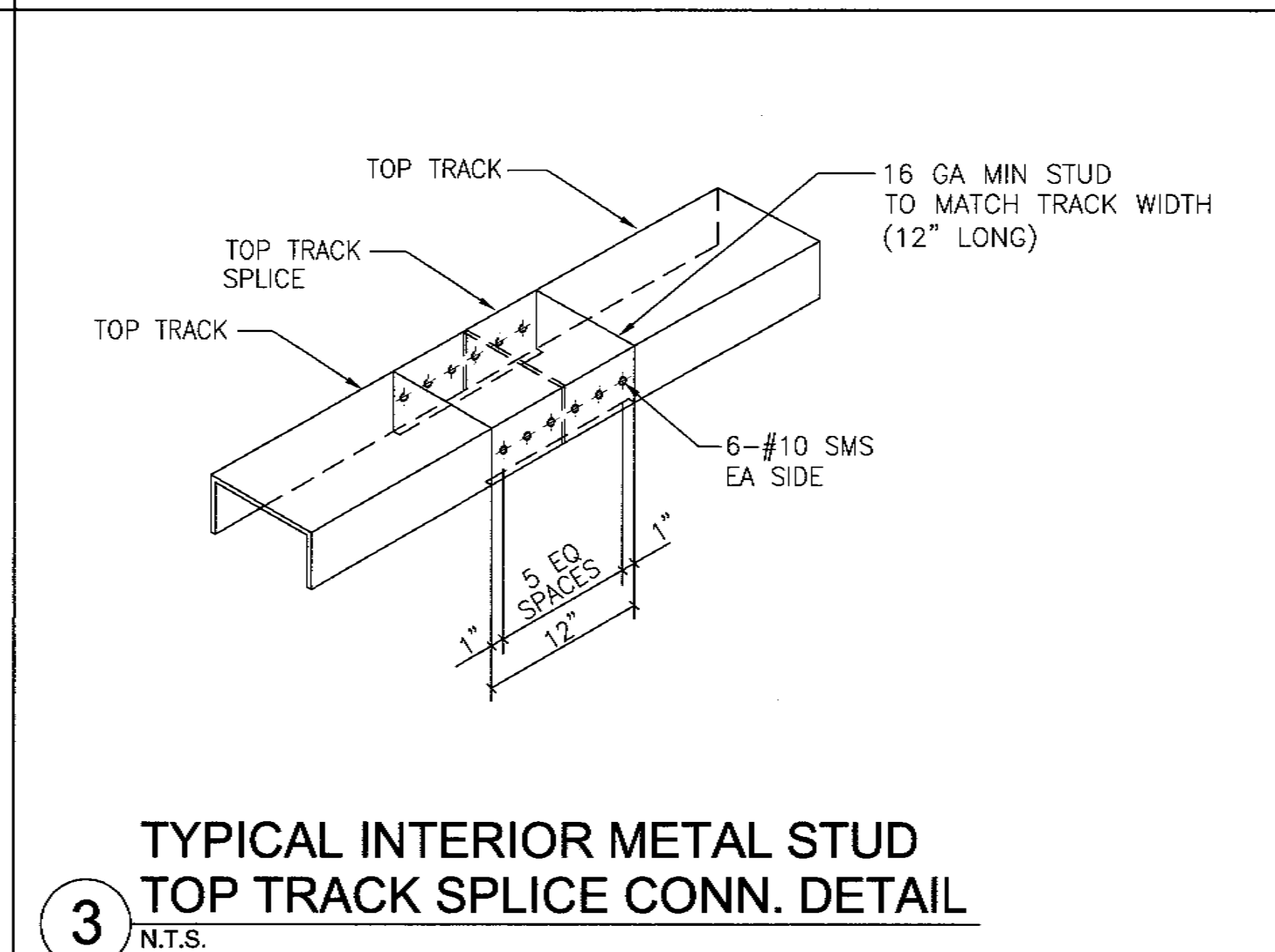
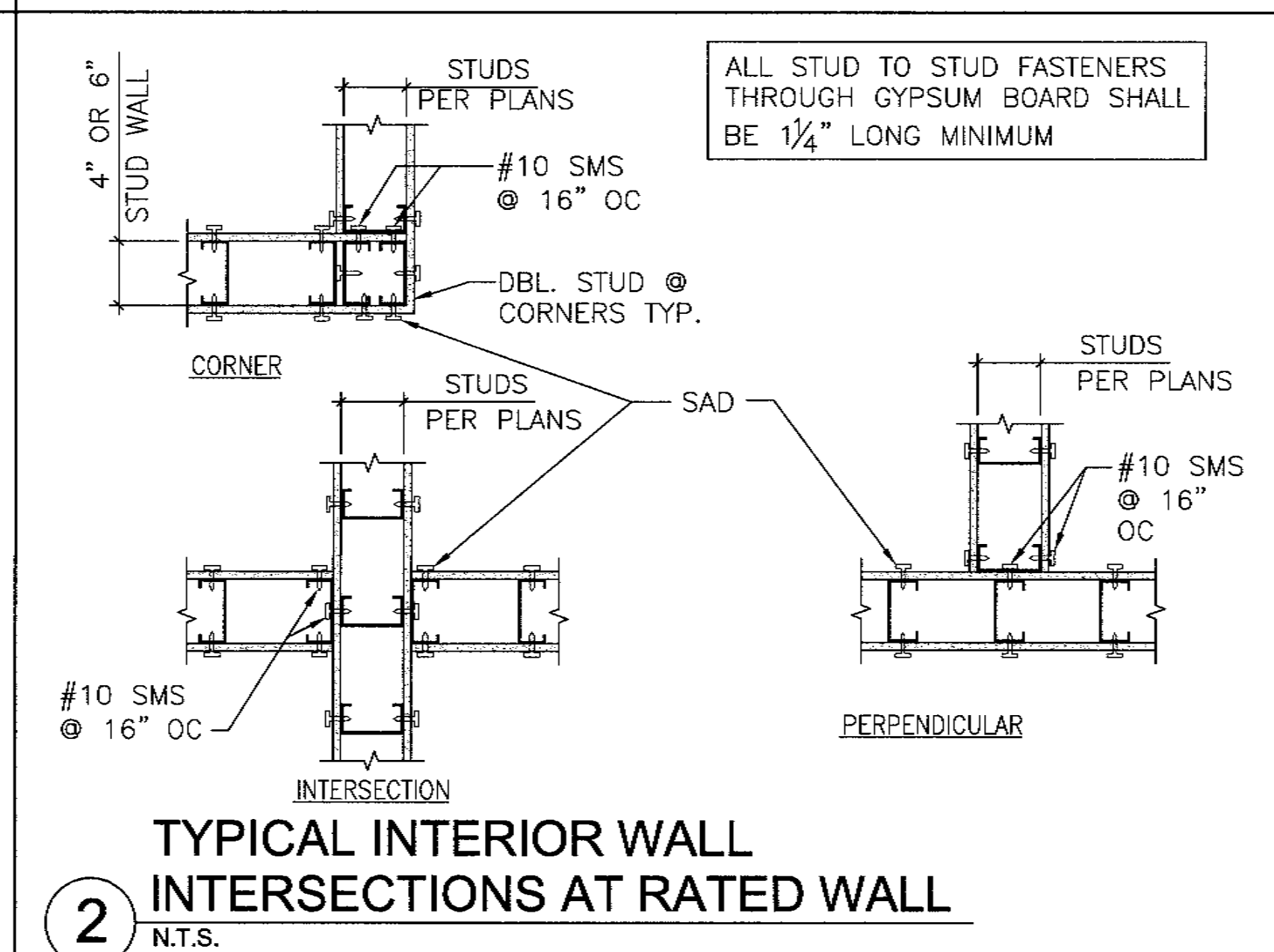
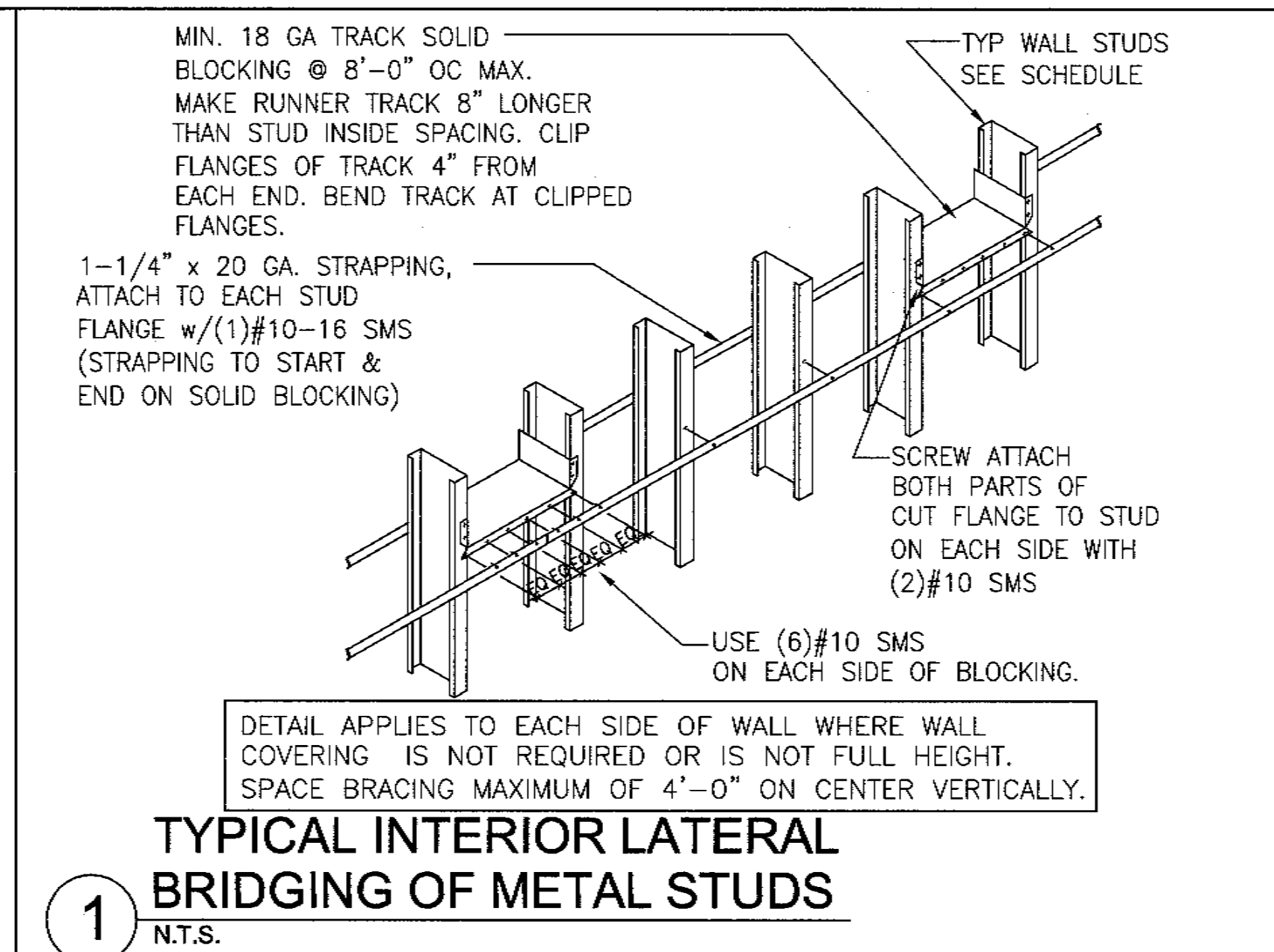
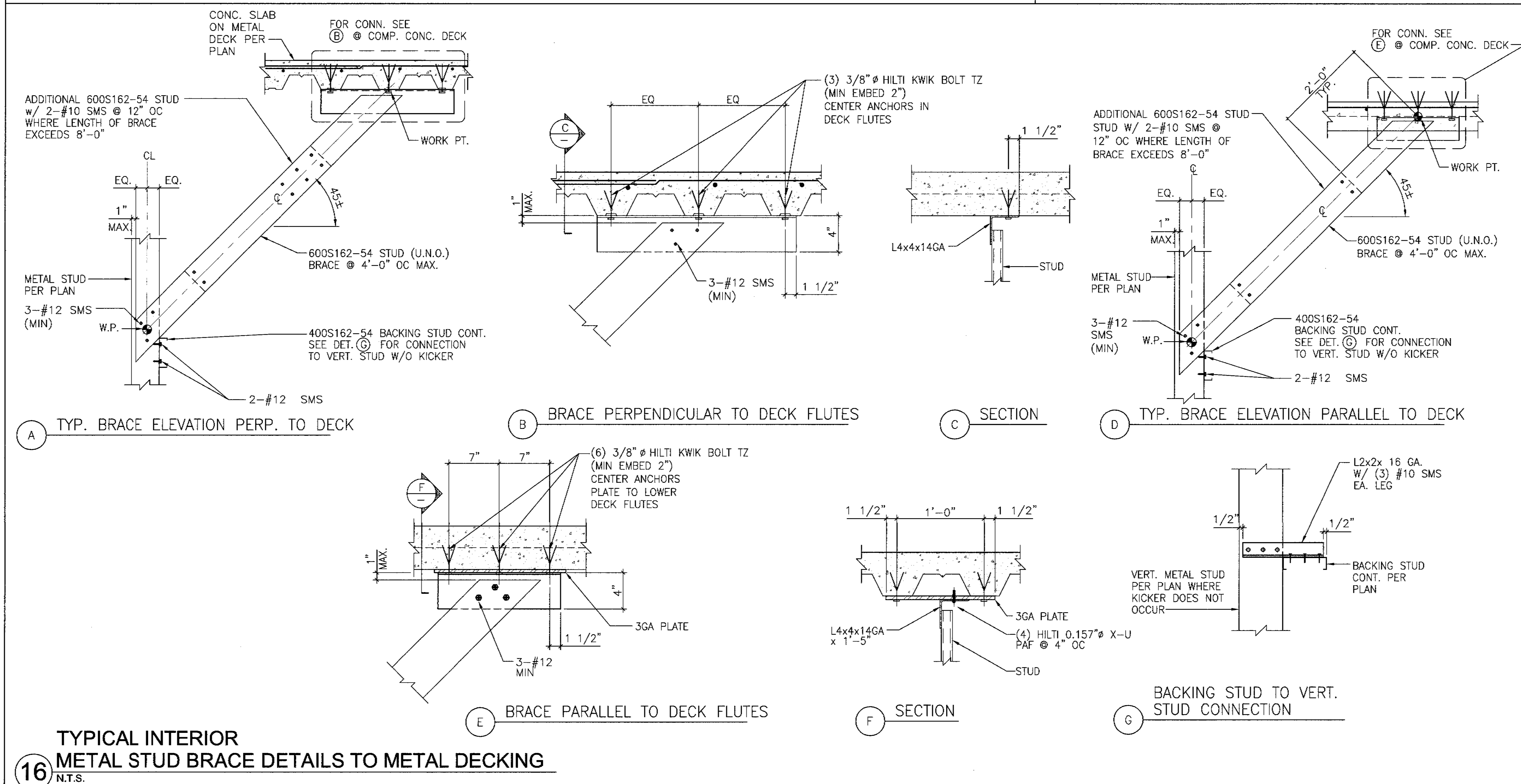
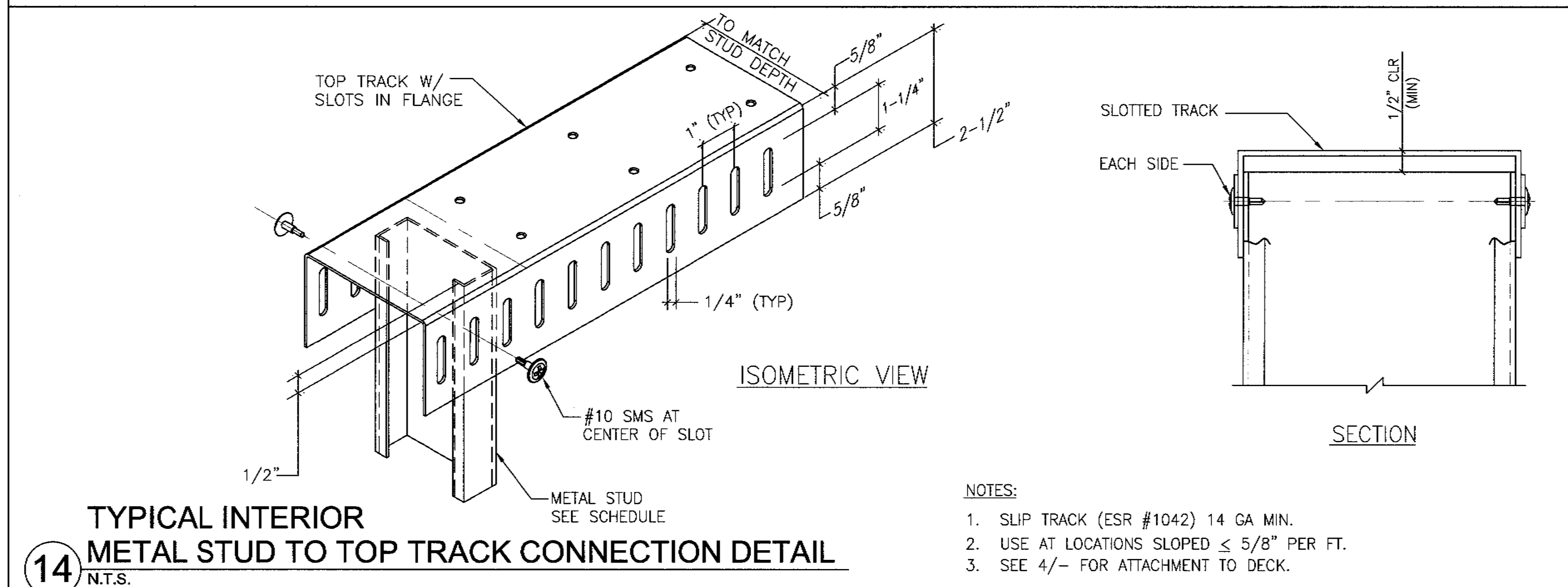
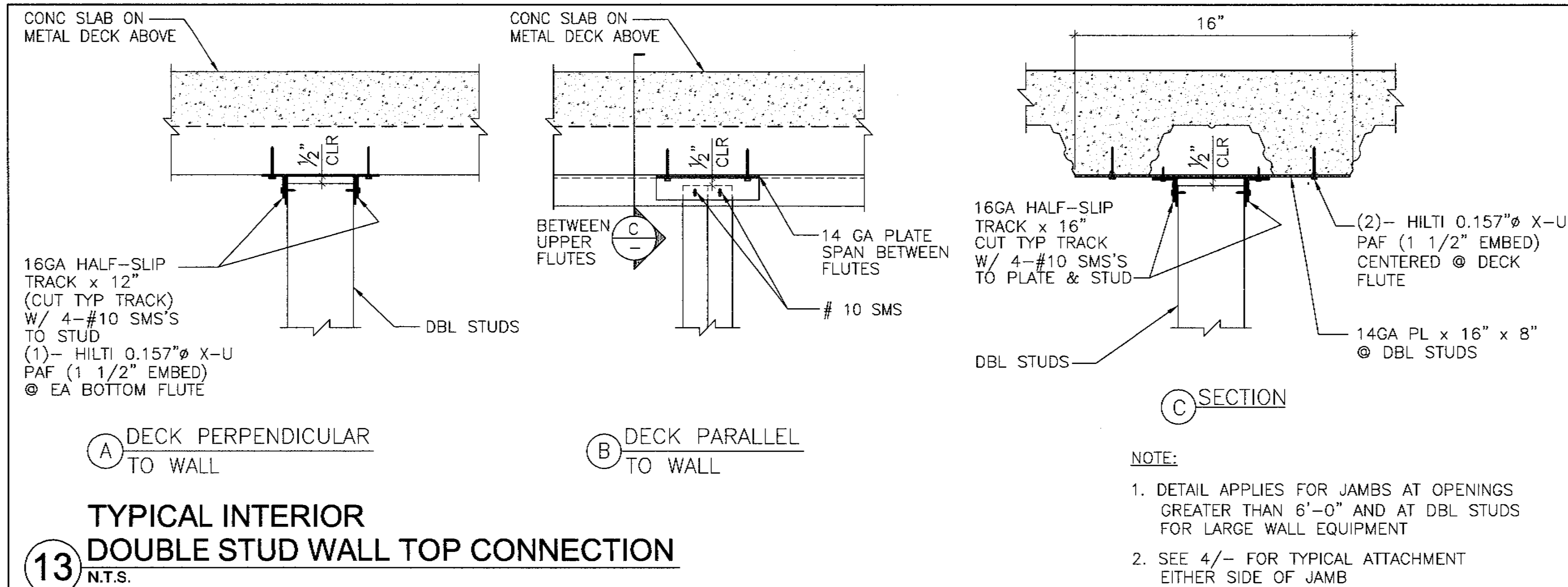


BEAM	a	b	c	RADIUS
W24x76	5.62'	17.94'	1.80'	23.25'
W27x54	6.24'	20.19'	2.00'	26.48'
W30x108	6.55'	22.37'	2.10'	30.84'

NOTES:

1. NO HOLES MAY BE DRILLED OR PUNCHED IN EITHER FLANGE OF THE BEAM WITHIN THE LENGTH THAT HAS RECEIVED THE RADIUS CUT OR BETWEEN THE REDUCED BEAM SECTION (RBS) CUT AND THE COLUMN. SHEAR STUDS AND MECHANICAL DECK FASTENERS TO THE BEAM FLANGE WITHIN THE LENGTH OF THE RADIUS CUT ARE PROHIBITED. SPOT WELDS FOR THE ATTACHMENT OF METAL DECKING ARE PERMITTED.
2. THE TOLERANCE ON THE DEPTH OF EACH RBS CUT IS PLUS OR MINUS 1/4 INCH. THE LENGTH OF THE CUT SHALL BE WITHIN PLUS OR MINUS 15% OF THE SPECIFIED LENGTH. THE DEPTH OF CUT ON EACH SIDE SHALL BE BALANCED, WITH NO MORE THAN 3/8 INCH TOTAL VARIANCE IN THE DEPTH OF CUT FROM ONE SIDE TO THE OTHER (EXAMPLE: PLUS 1/8 INCH ON ONE SIDE, MINUS 1/4 INCH ON THE OTHER SIDE). THE BALANCE OF REMAINING FLANGE WIDTH ABOUT THE WEB OF THE BEAM IS NOT A CONSIDERATION.
3. AFTER THERMAL CUTTING, THE RBS SURFACE SHALL HAVE A SURFACE ROUGHNESS OF NO MORE THAN 500 MICRONS (AWS SAMPLE 3). GRINDING OF THERMALLY CUT EDGES SHALL BE PROVIDED ONLY AS NECESSARY TO MEET THIS CRITERIA. ALL CORNERS SHALL BE ROUNDED TO MINIMIZE NOTCH EFFECTS. GRINDING SHALL ONLY BE IN THE DIRECTION OF THE FLANGE LENGTH. CORNERS BETWEEN THE CUT RBS SURFACE AND THE TOP AND BOTTOM OF THE FLANGES SHALL BE ROUNDED TO REMOVE SHARP EDGES, BUT A MINIMUM RADIUS OR CHAMFER IS NOT REQUIRED.
4. GOUGES AND NOTCHES THAT OCCUR IN THERMAL CUT RBS SURFACE MAY BE REPAIRED BY GRINDING NOT MORE THAN 1/4 INCH DEEP. THE GOUGED OR NOTCHED AREA SHALL BE REPAIRED BY WELDING. NOTCHES AND GOUGES EXCEEDING 1/2 INCH IN DEPTH MAY BE REPAIRED ONLY WITH THE APPROVAL OF THE ENGINEER. THE NOTCH OR GOUGE SHALL BE REMOVED AND GROUND TO PROVIDE A SMOOTH RADIUS OF NOT LESS THAN 3/8 INCH FOR WELDING. THE REPAIR AREA SHALL BE PREHEATED TO A TEMPERATURE BETWEEN 400 DEGREES FAHRENHEIT AND 500 DEGREES FAHRENHEIT, MEASURED AT THE LOCATION OF THE WELD REPAIR APPROXIMATELY ONE MINUTE AFTER REMOVAL OF THE HEATING SOURCE. REPAIR WELDING SHALL BE DONE WITH NOTCH-TOUGH ELECTRODES MEETING THE REQUIREMENTS IN FEMA 353 (JULY 2000) PART 1, SECTION 2.4. IF A REPAIR WELD IS REQUIRED, FOLLOWING WELDING, THE REPAIR WELD SHALL BE GROUND TO A SMOOTH CONTOUR MEETING THE RBS REQUIREMENTS, WITH A SURFACE ROUGHNESS NOT TO EXCEED 500 MICRONS. THE WELDED REPAIR AREA SHALL BE INSPECTED USING MAGNETIC PARTICLE TESTING.
5. GOUGES AND NOTCHES THAT EXCEED 1/4 INCH IN DEPTH, BUT NOT TO EXCEED 1/2 INCH IN DEPTH, AND THOSE NOTCHES AND GOUGES WHERE REPAIR BY GRINDING WOULD INCREASE THE EFFECTIVE DEPTH OF THE RBS CUT BEYOND TOLERANCE, MAY BE REPAIRED BY WELDING. NOTCHES AND GOUGES EXCEEDING 1/2 INCH IN DEPTH MAY BE REPAIRED ONLY WITH THE APPROVAL OF THE ENGINEER. THE NOTCH OR GOUGE SHALL BE REMOVED AND GROUND TO PROVIDE A SMOOTH RADIUS OF NOT LESS THAN 3/8 INCH FOR WELDING. THE REPAIR AREA SHALL BE PREHEATED TO A TEMPERATURE BETWEEN 400 DEGREES FAHRENHEIT AND 500 DEGREES FAHRENHEIT, MEASURED AT THE LOCATION OF THE WELD REPAIR APPROXIMATELY ONE MINUTE AFTER REMOVAL OF THE HEATING SOURCE. REPAIR WELDING SHALL BE DONE WITH NOTCH-TOUGH ELECTRODES MEETING THE REQUIREMENTS IN FEMA 353 (JULY 2000) PART 1, SECTION 2.4. IF A REPAIR WELD IS REQUIRED, FOLLOWING WELDING, THE REPAIR WELD SHALL BE GROUND TO A SMOOTH CONTOUR MEETING THE RBS REQUIREMENTS, WITH A SURFACE ROUGHNESS NOT TO EXCEED 500 MICRONS. THE WELDED REPAIR AREA SHALL BE INSPECTED USING MAGNETIC PARTICLE TESTING.
6. PROTECTED ZONE, NO ATTACHMENT TO BEAM, INCLUDING WSA, BEAM PIPE HANGERS, EXTERIOR SUPPORT, ETC., SHALL OCCUR WITHIN THE AREA BETWEEN THE COLUMN FACE AND 6\"/>

4 REDUCED BEAM SECTION (RBS) NOTES NTS

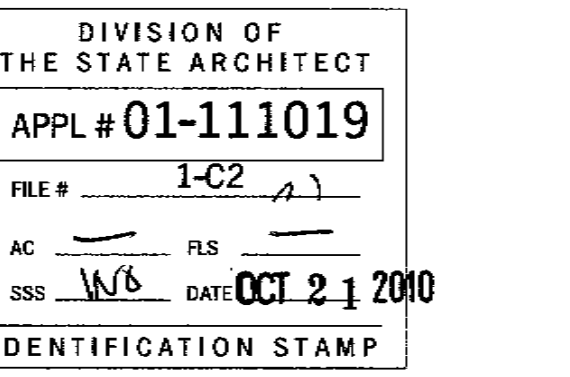


Steinberg Architects

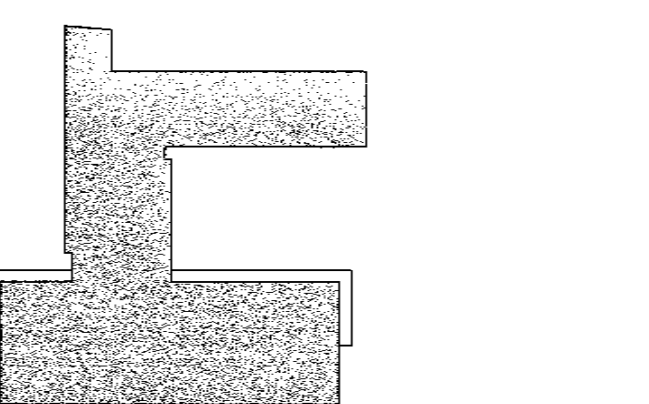
CLIENT
Chabot-Las Positas Community College District
5020 Franklin Dr.
Pleasanton, CA 94588

ARCHITECT
Steinberg Architects
60 Pierce Avenue
San Jose, CA 95110

STRUCTURAL
Crosby Group Structural Engineering
2200 Bridge Parkway, Suite 104
Redwood City, CA 94065
(650) 367-8100
(650) 598-0277 (fax)



REV	DATE	REASON
09	10.14.2010	DSA BACKCHECK
08	07.19.2010	100% CD SUBMITTAL TO DSA
05	06.03.2010	75% CD - DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% DD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

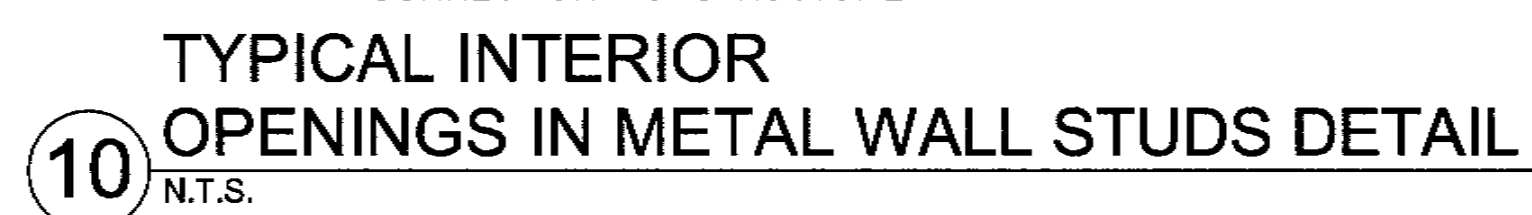
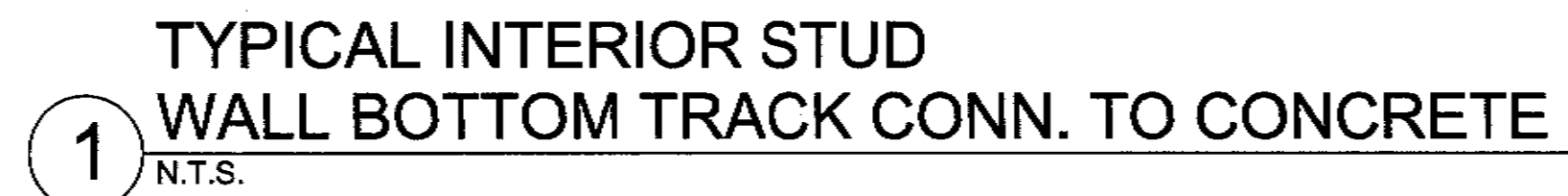
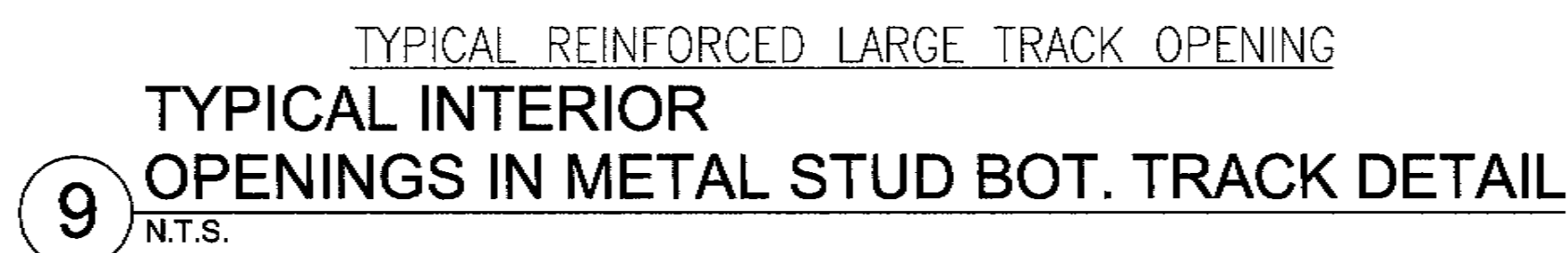
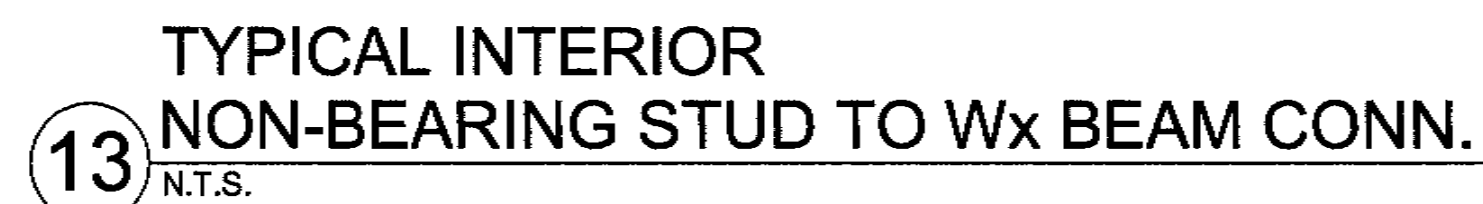


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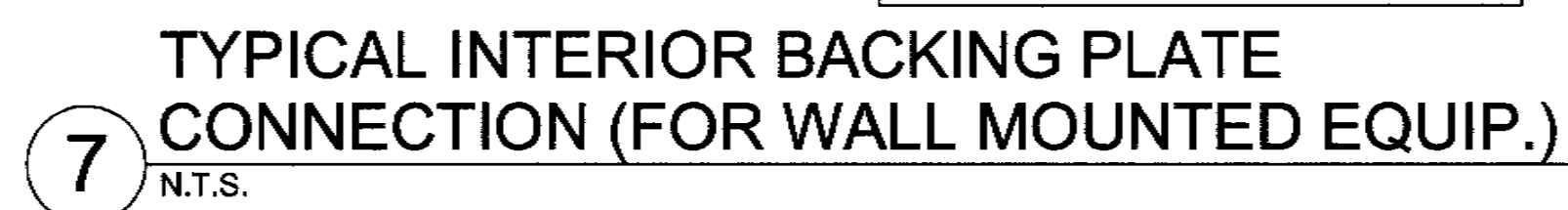
TYPICAL INTERIOR METAL STUD DETAILS

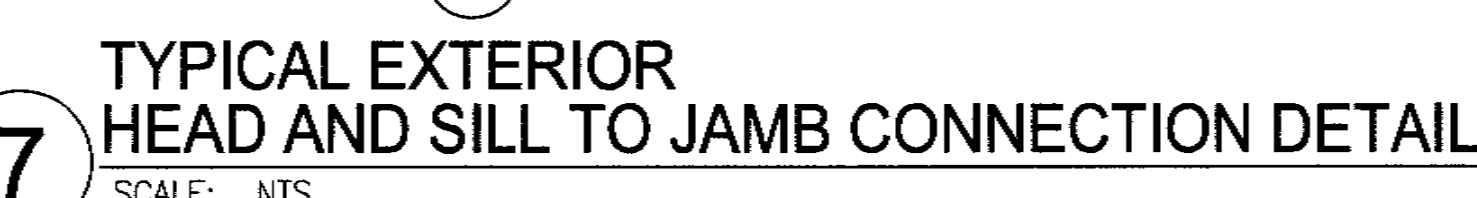
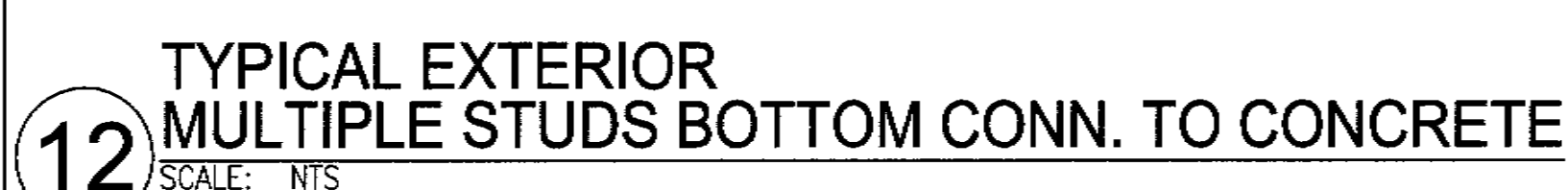
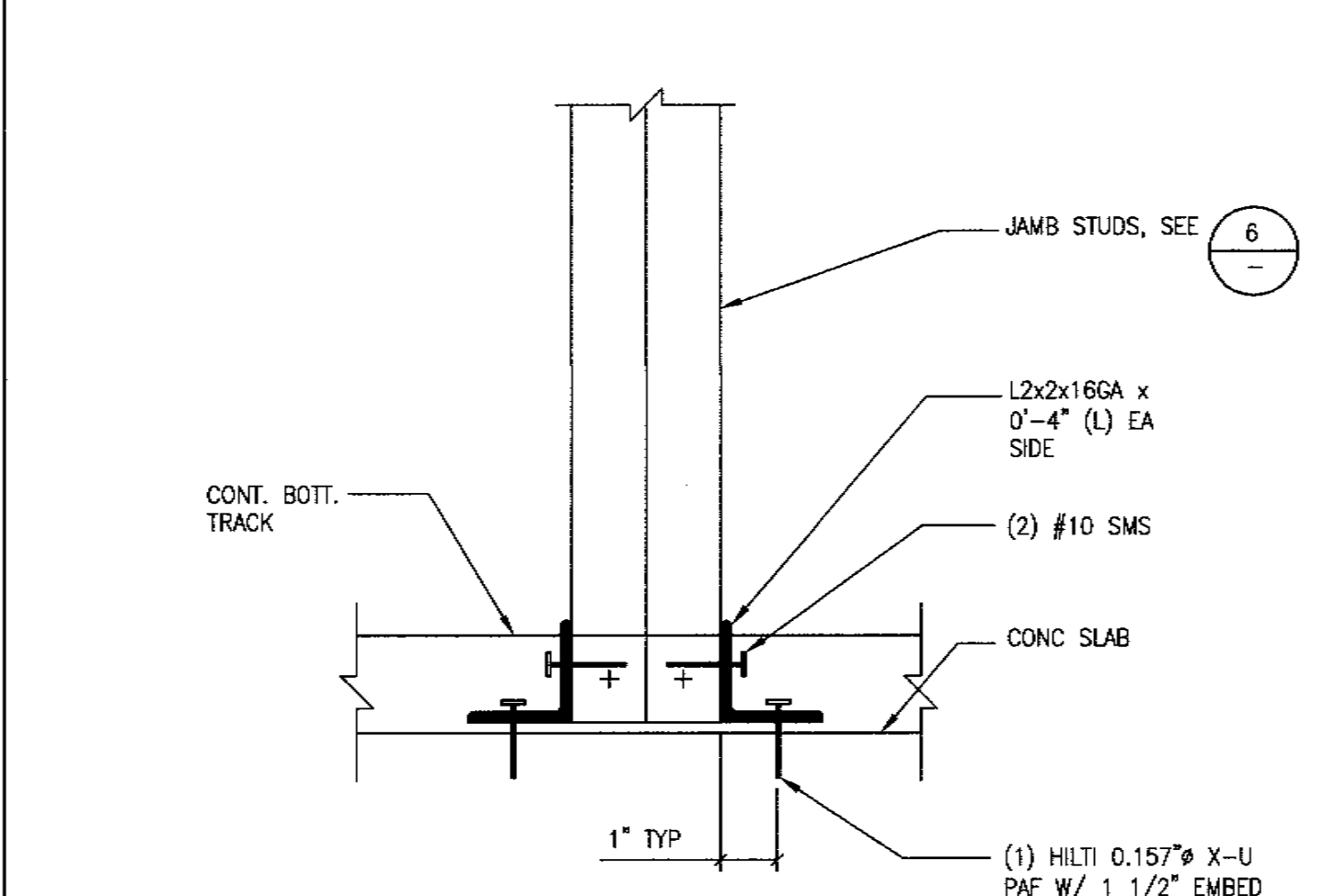
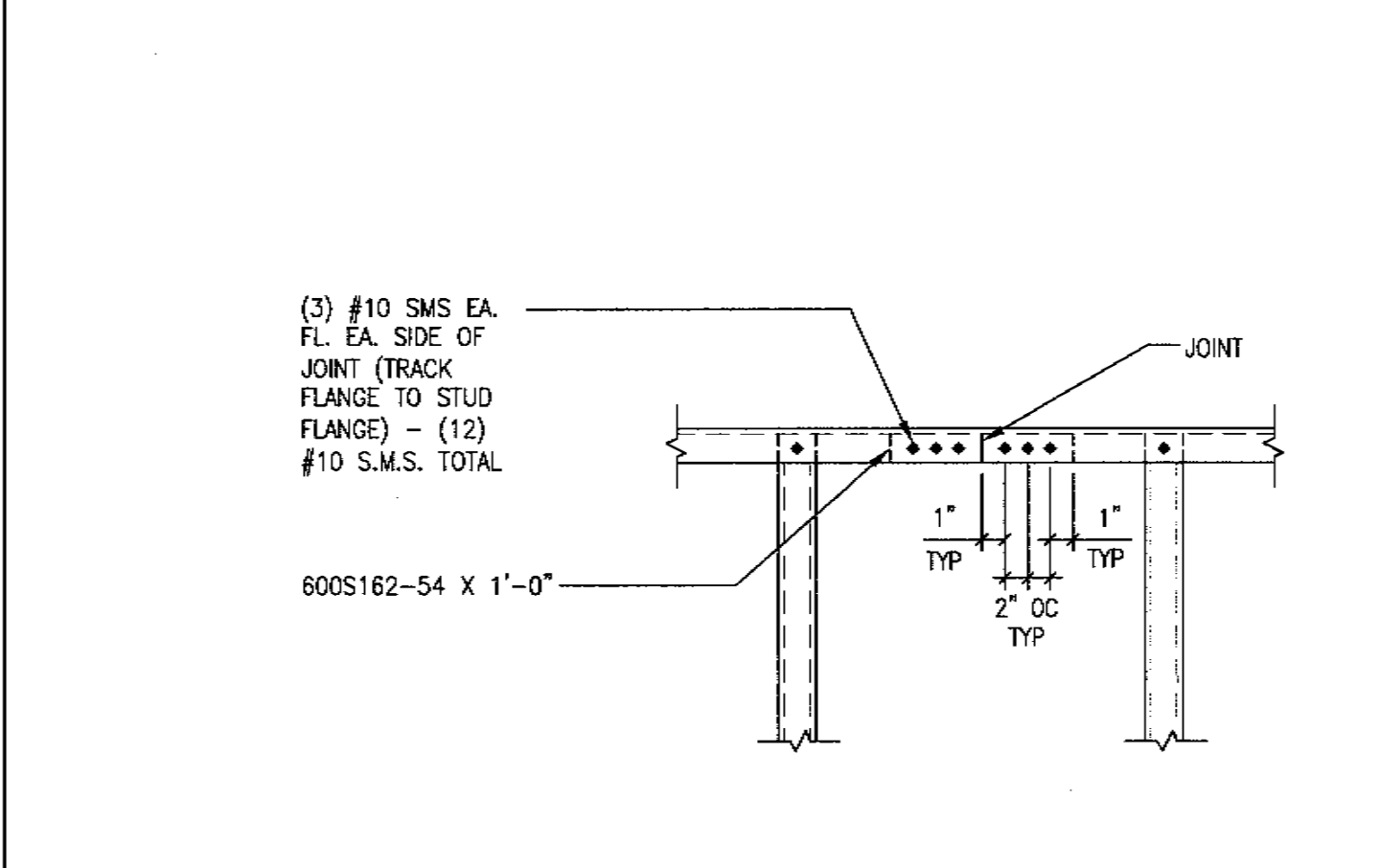
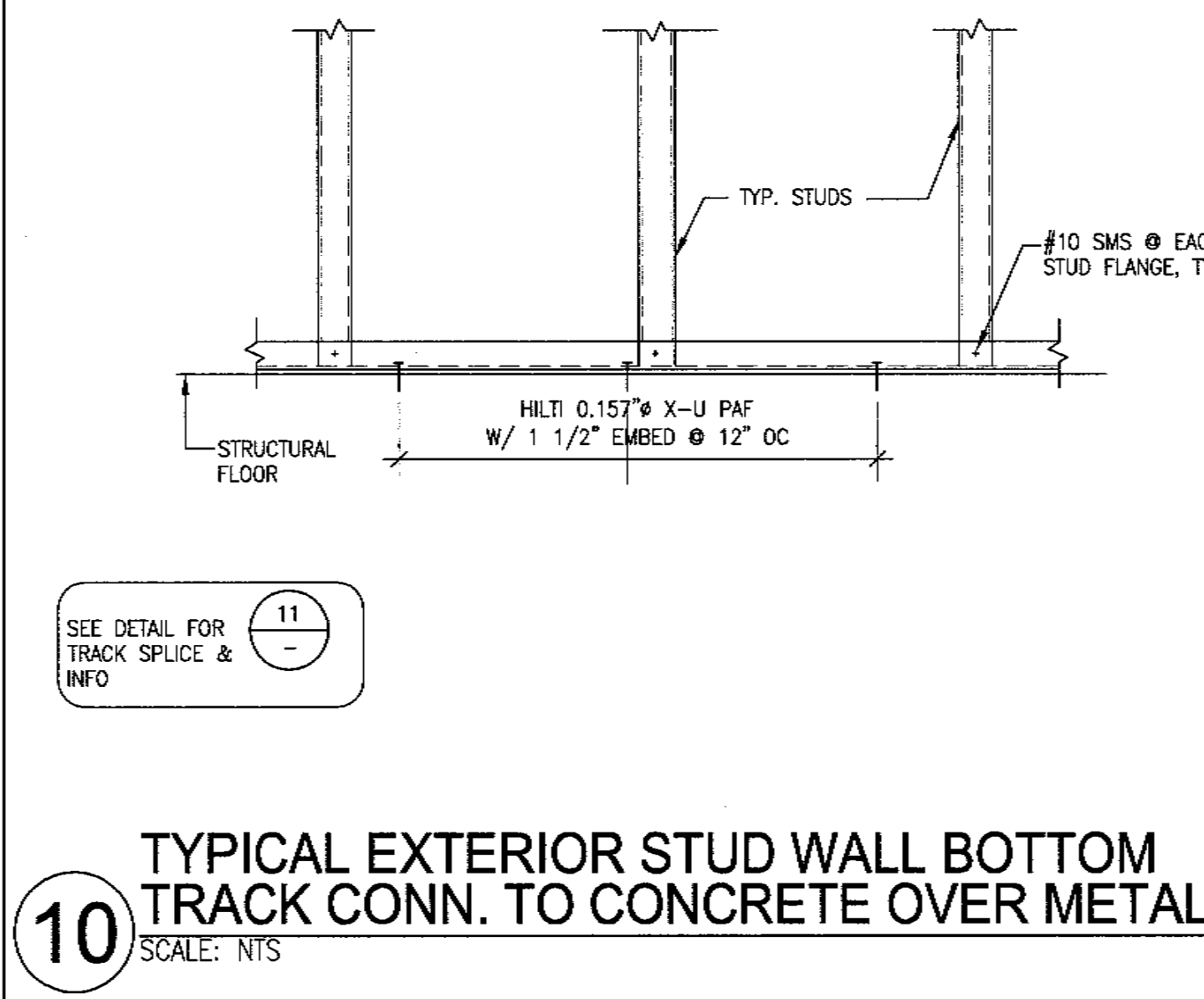
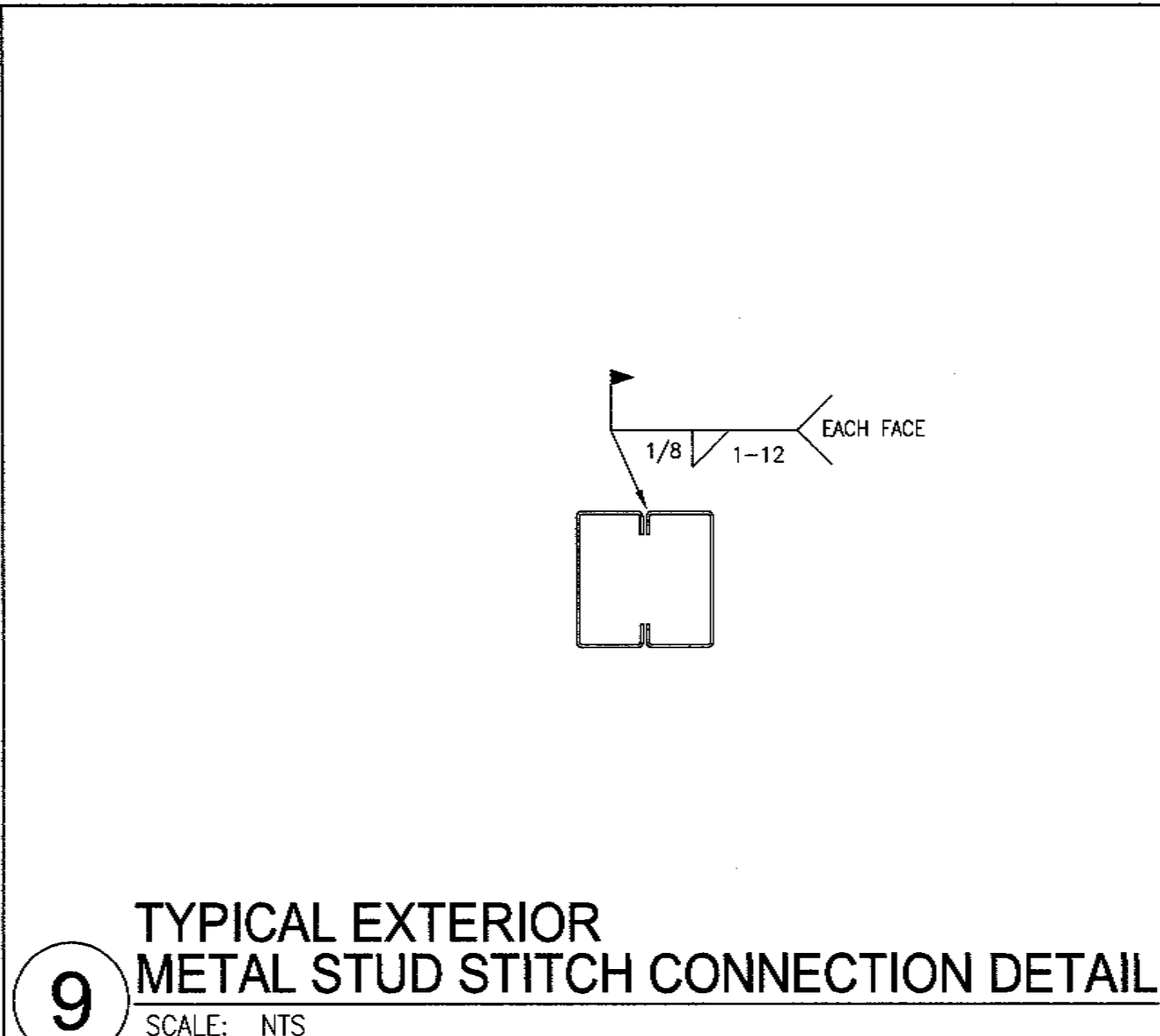
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DATE: 2010.06.06.14
SCALE: AS NOTED

S1.01.10

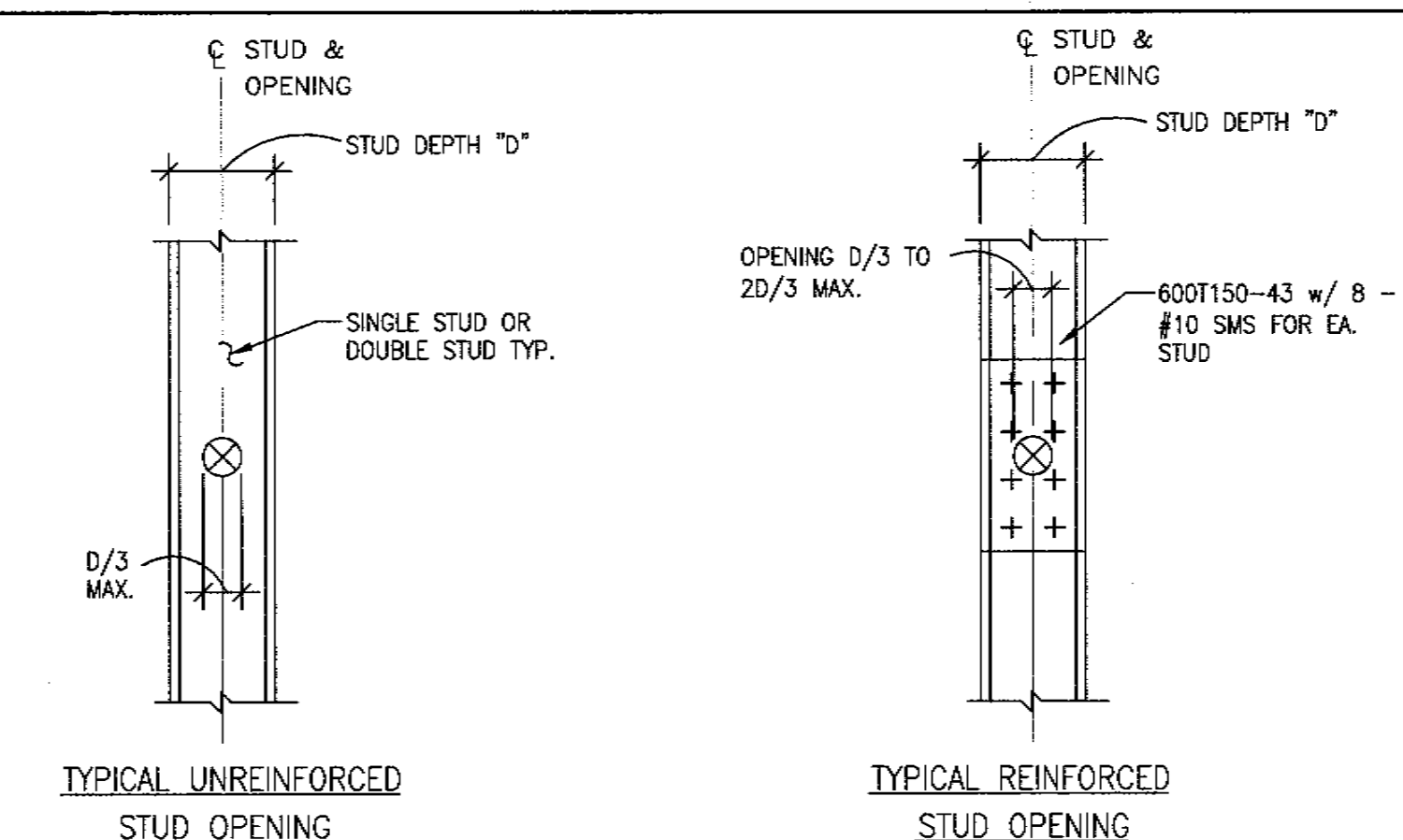


11 TYPICAL INTERIOR
BACKING PLATE CONNECTION SCHEDULE
N.T.S.





S1.01.12

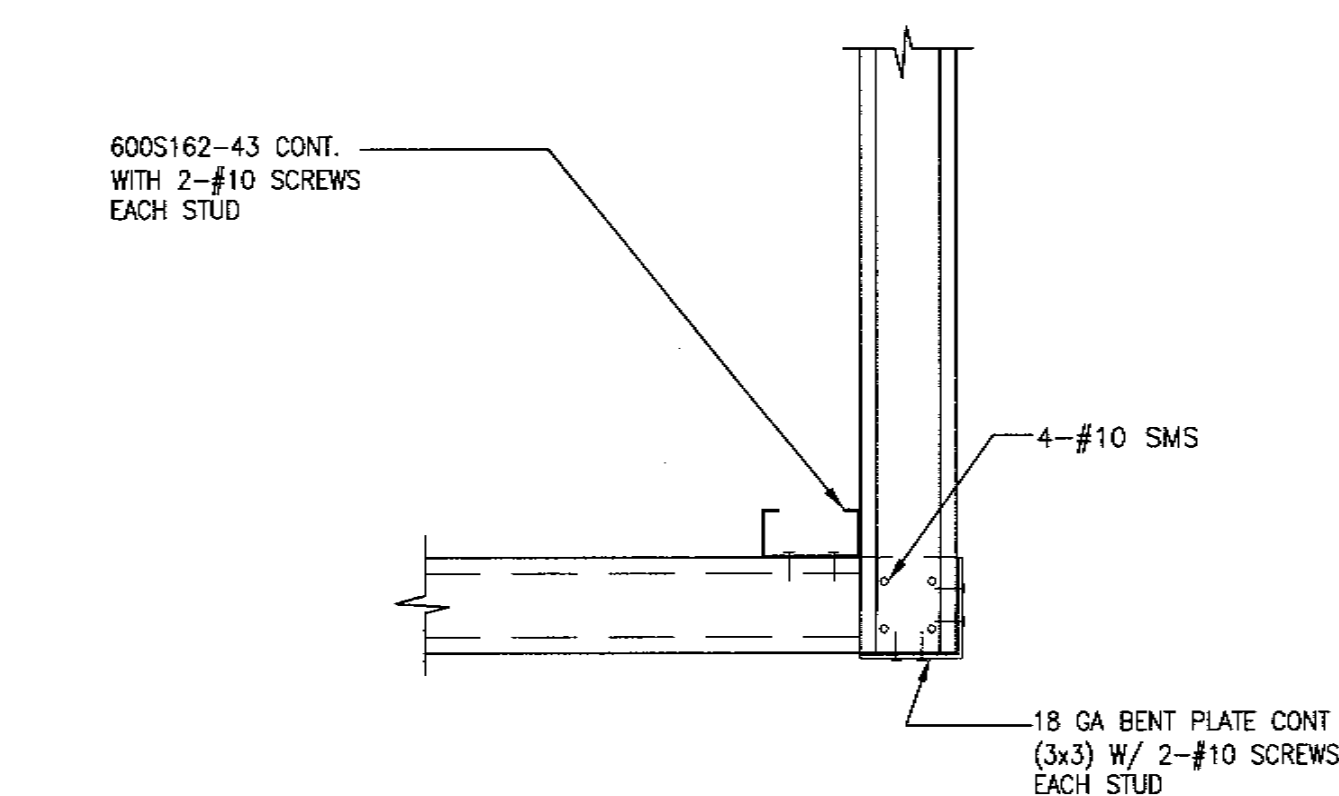


NOTES:

1. NO OPENINGS IN METAL STUD GREATER THAN 2D/3 ALLOWED.
2. DO NOT LOCATE OPENINGS AT BACKING PLATE OR WITHIN 10" OF BEARING OR ATTACHMENT TO STRUCTURE.

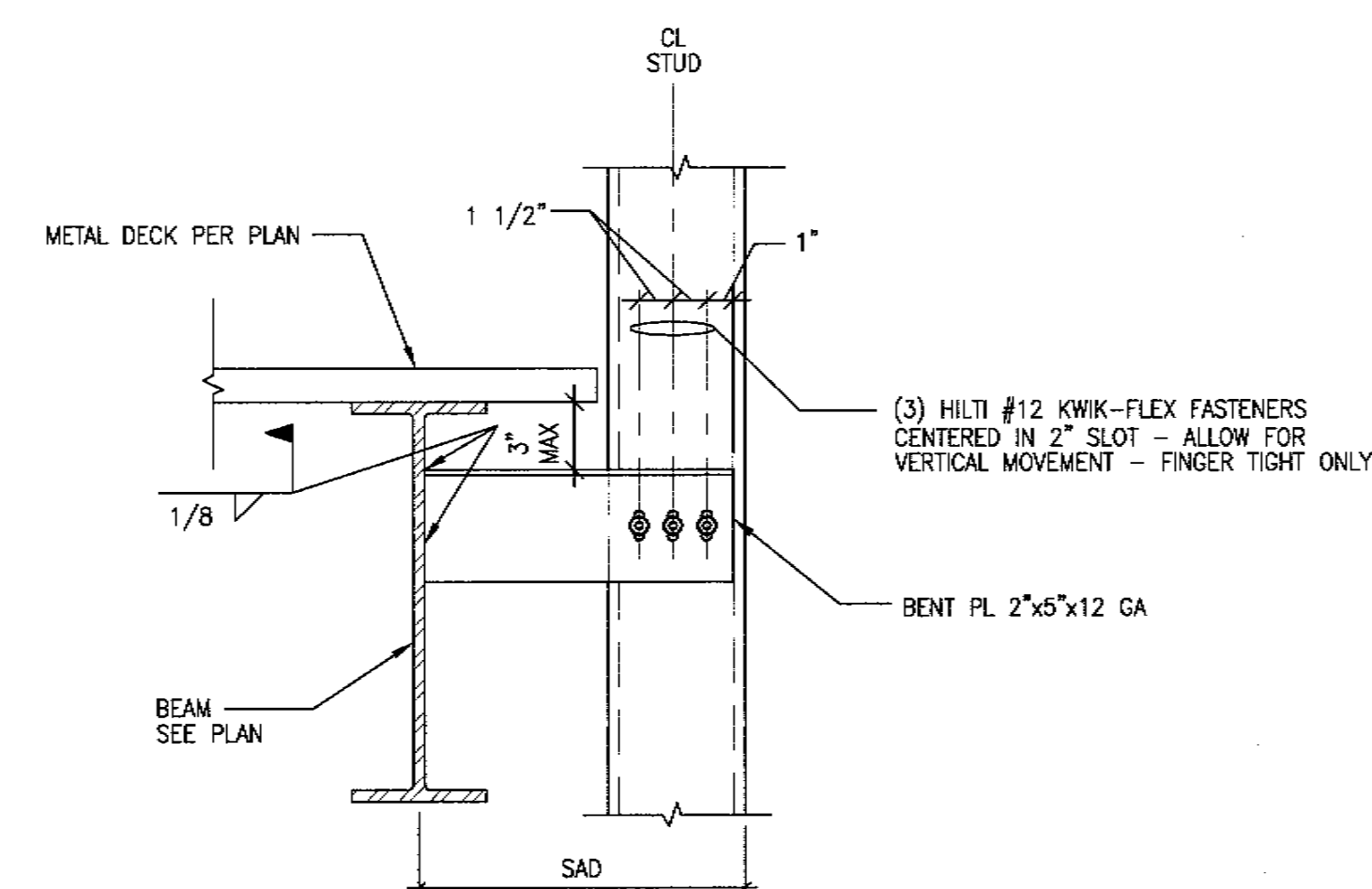
1 TYPICAL EXTERIOR
OPENINGS IN METAL WALL STUDS DETAIL

SCALE: NTS



2 TYPICAL EXTERIOR
HORIZ. METAL STUD TO VERT. METAL STUD CONN.

SCALE: NTS



2 TYPICAL EXTERIOR
METAL STUD TO STRUCTURE AT HIGH ROOF

SCALE: NTS

Steinberg Architects

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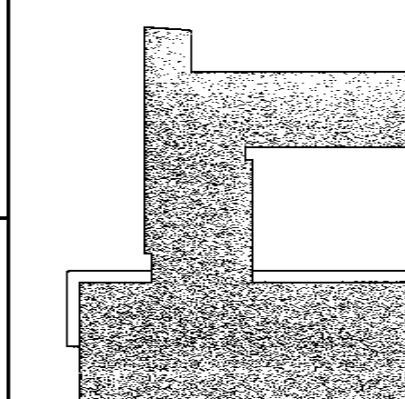
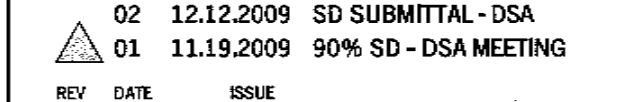
STRUCTURAL
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Redwood City, CA 94065
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(650) 598-0277 (fax)



Crosby
group



09	10.14.2010	DSA BACKCHECK
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01	11.19.2009	90% SD - DSA MEETING



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TYPICAL EXTERIOR METAL STUDS DETAILS

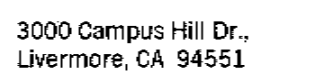
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DATE: 2010.October.14
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S1.01.13

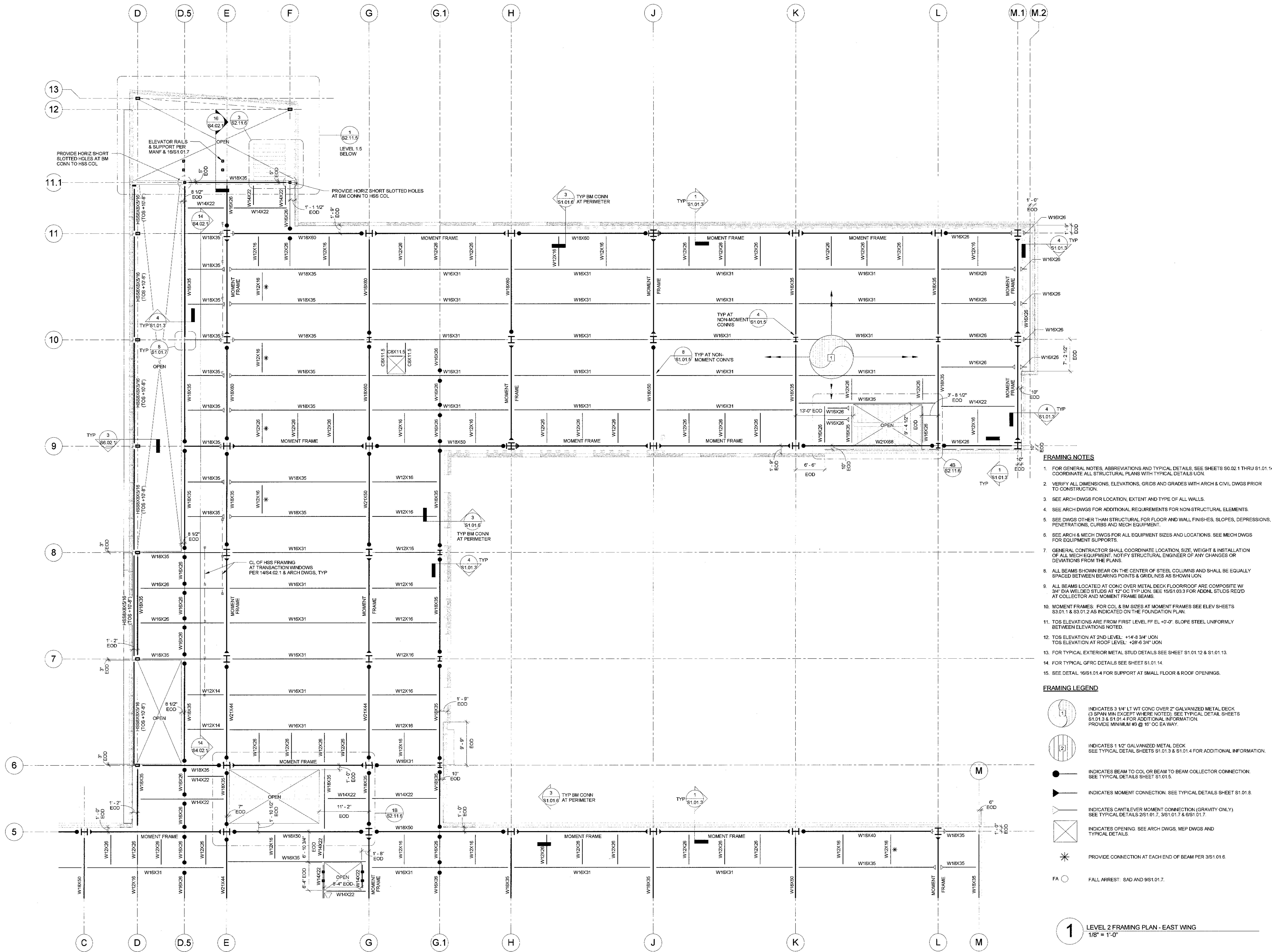
5 TYPICAL BEARING PLATE
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PROJECT #: 09050.000
DATE: 2010.October.14
SCALE: AS NOTED

02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING
REV	NO	DATE
		ISSUE



2.11.1



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Redwood City, CA 94065
(650) 367-8100
(650) 599-0277 (fax)

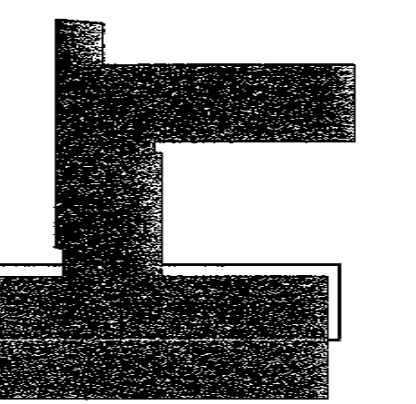


DIVISION OF
THE STATE ARCHITECT
APPL # 01-111019
FILE # 1-02
AC W.D. FLS
SSS W.D. DATE OCT 21 2010
IDENTIFICATION STAMP



09	10.14.2010	DSA BACKCHECK
06	07.19.2010	100% CD SUBMITTAL TO DSA
06	06.03.2010	75% CD - DSA MEETING
05	05.20.2010	75% STRUCTURE - DSA COLLABORATIVE REVIEW
04	03.25.2010	100% DD SUBMITTAL TO LPC
03	03.11.2010	80% DD - DSA MEETING
02	12.12.2009	SD SUBMITTAL - DSA
01	11.19.2009	90% SD - DSA MEETING

REV NO DATE ISSUE

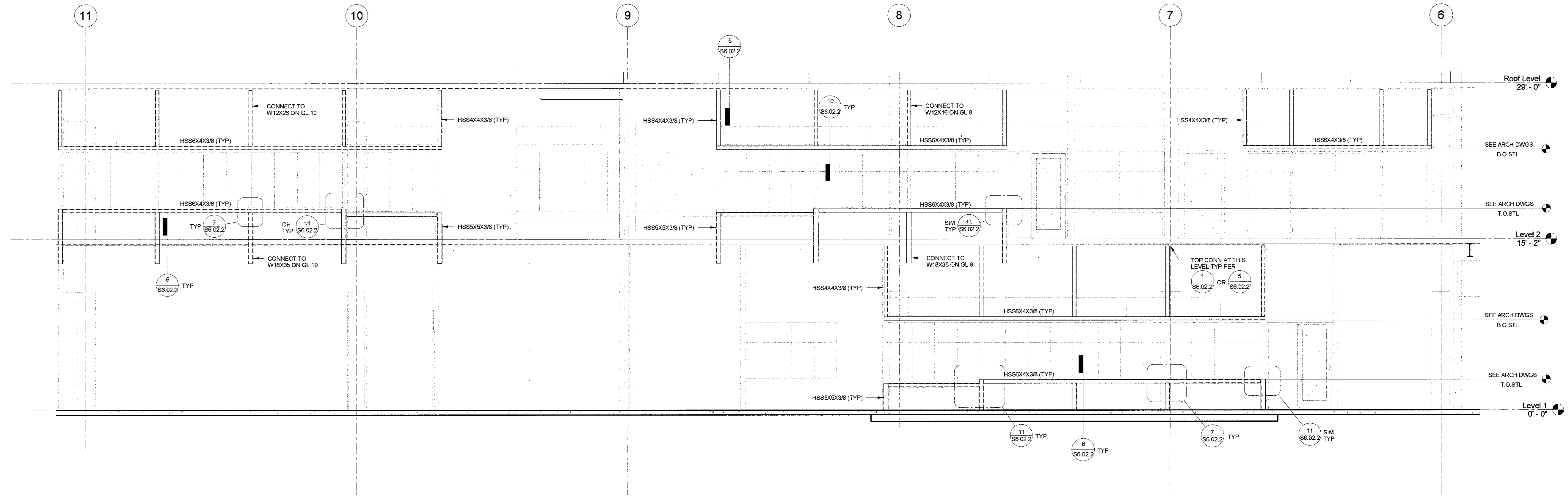


Student Services and
Administration
Building

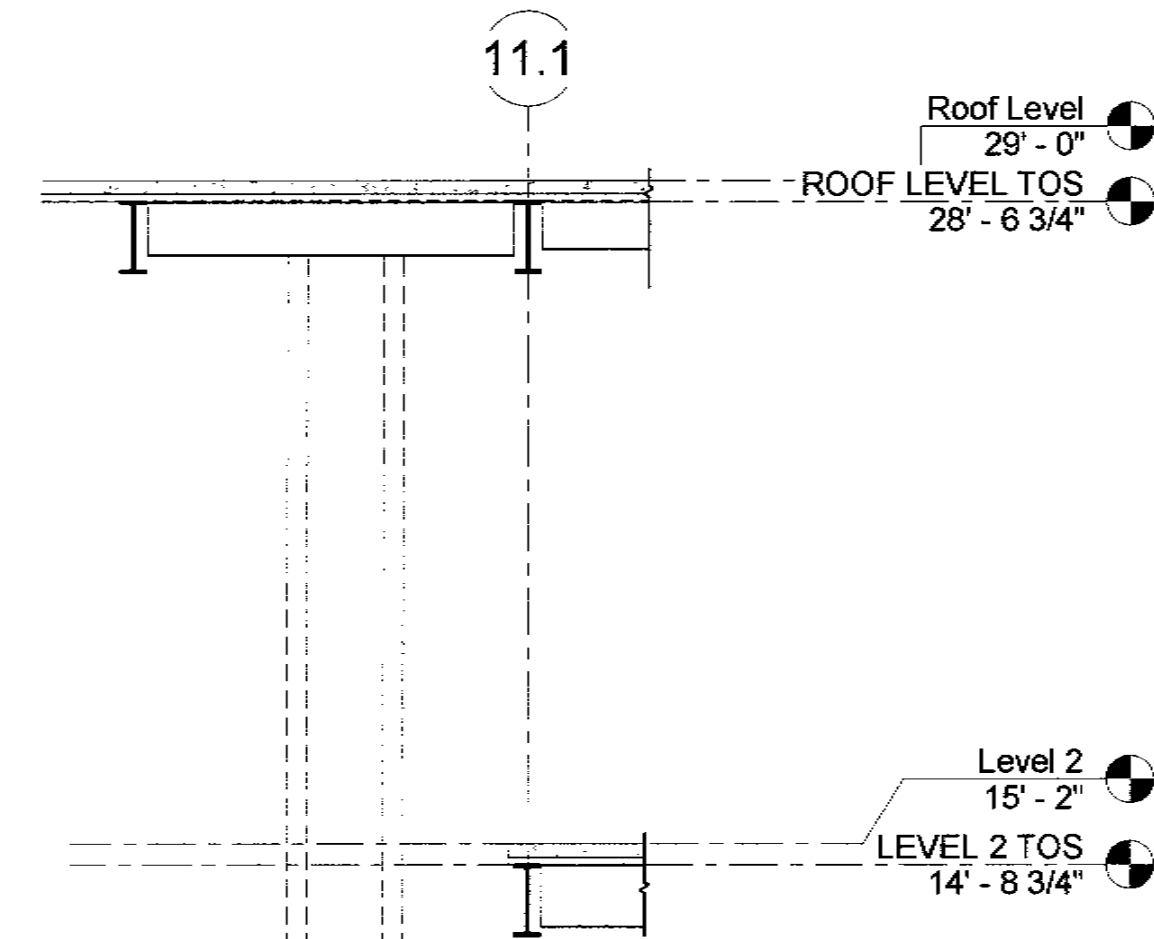
Las Positas College
3000 Campus Hill Dr.,
Livermore, CA 94551

WALL SECTIONS & ELEVATIONS

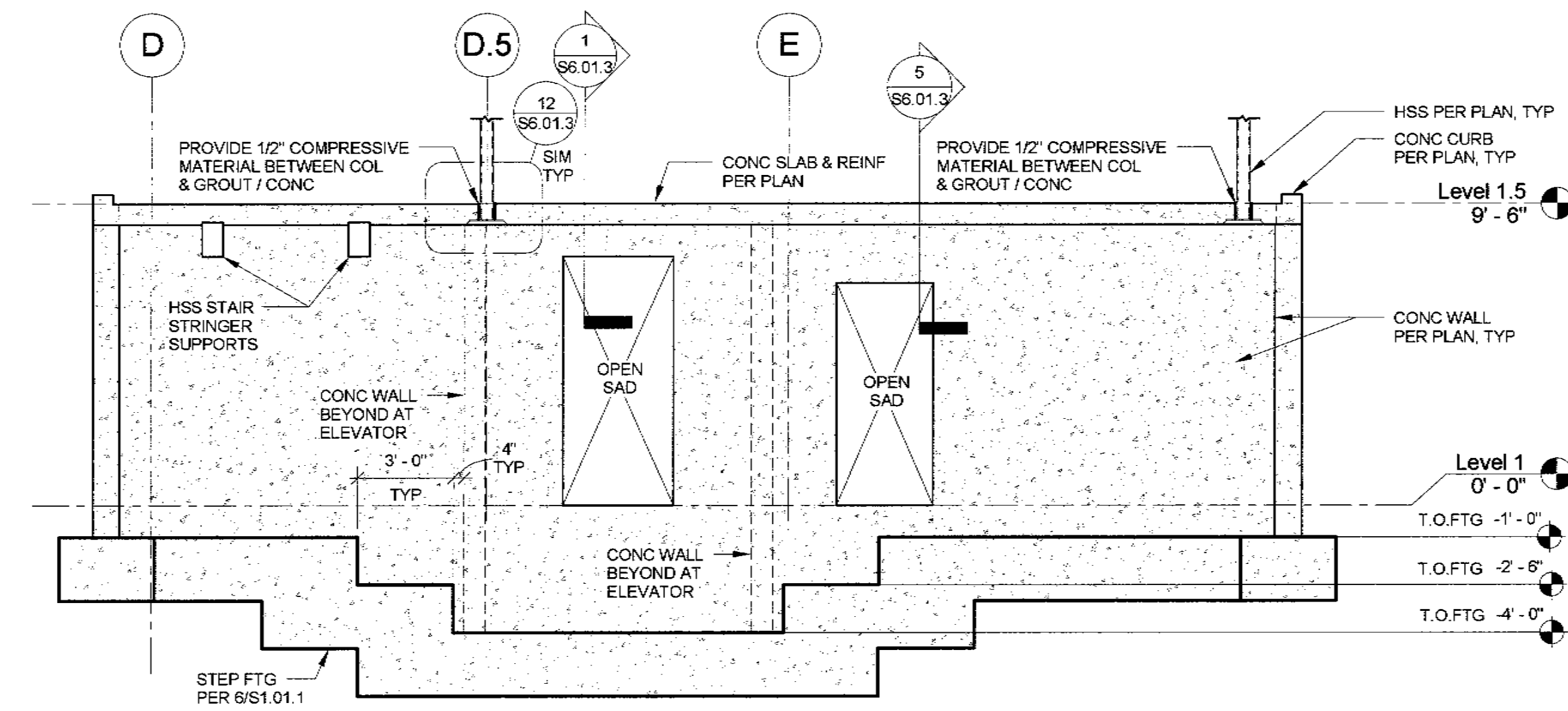
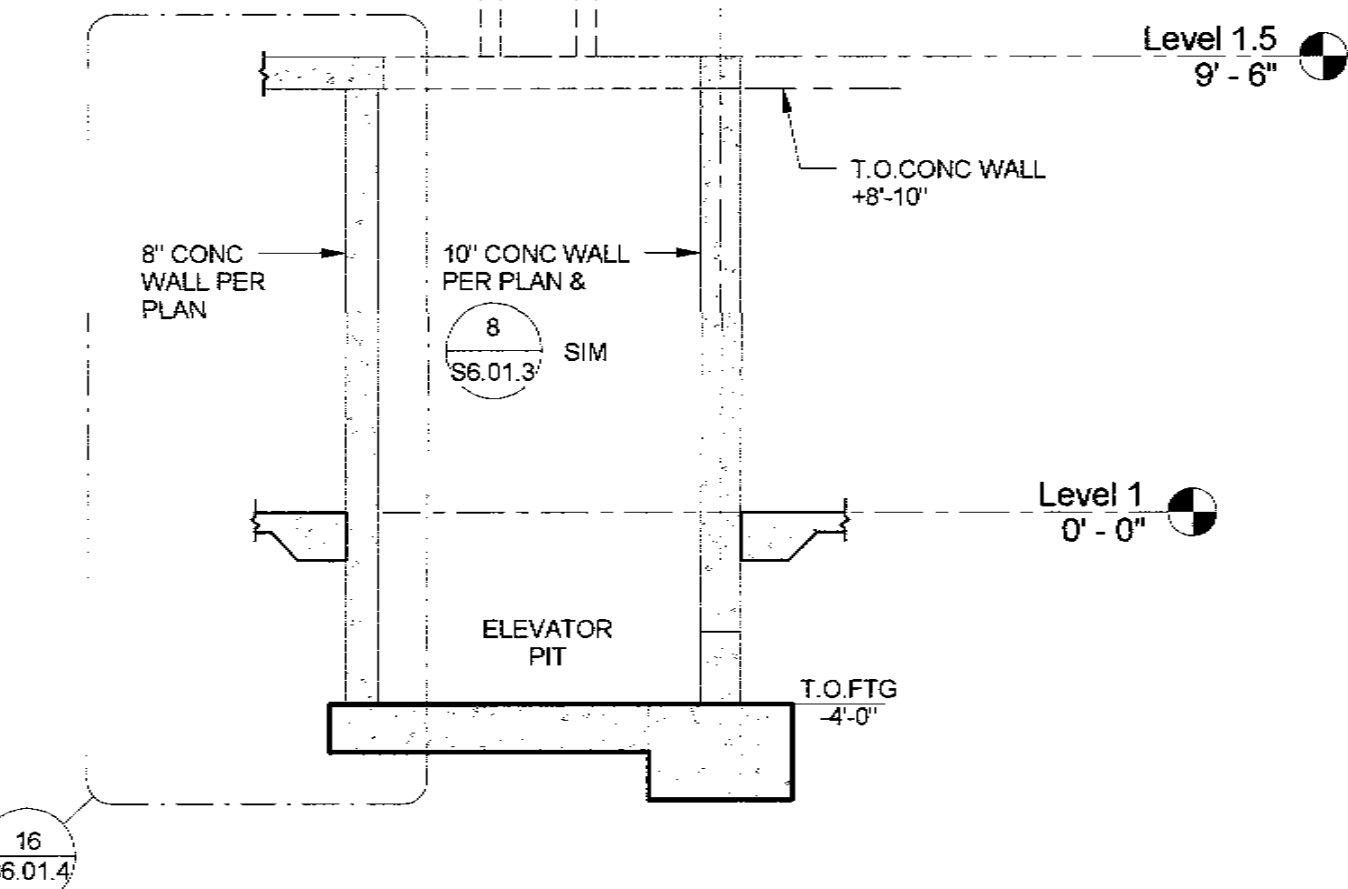
PROJECT # 08050.000
DATE: 2010.04.04.14
SCALE: 1/4" = 1'-0"



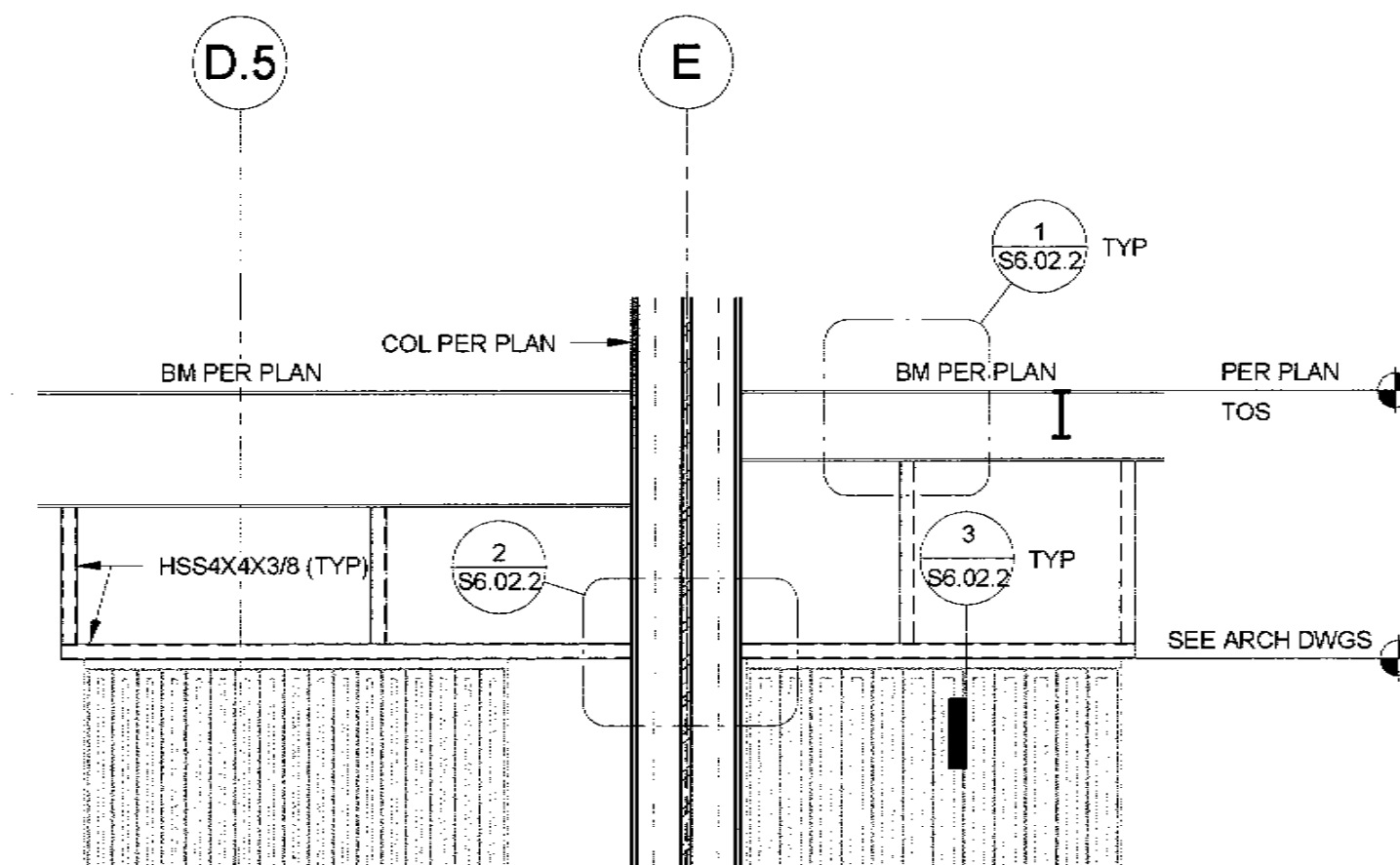
14 BLDG SECTION AT TRANSACTION WINDOWS
1/4" = 1'-0"



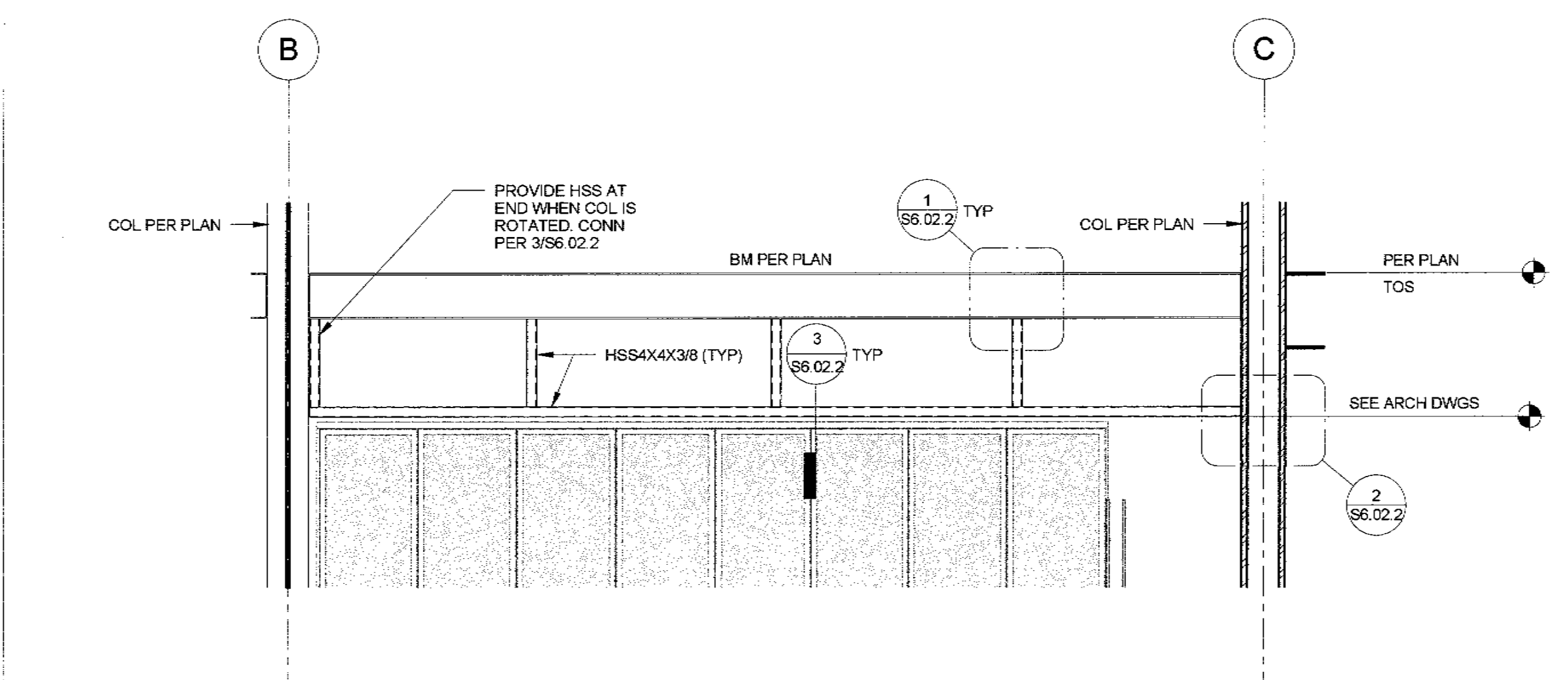
16 BLDG SECTION AT EAST ELEVATOR
1/4" = 1'-0"



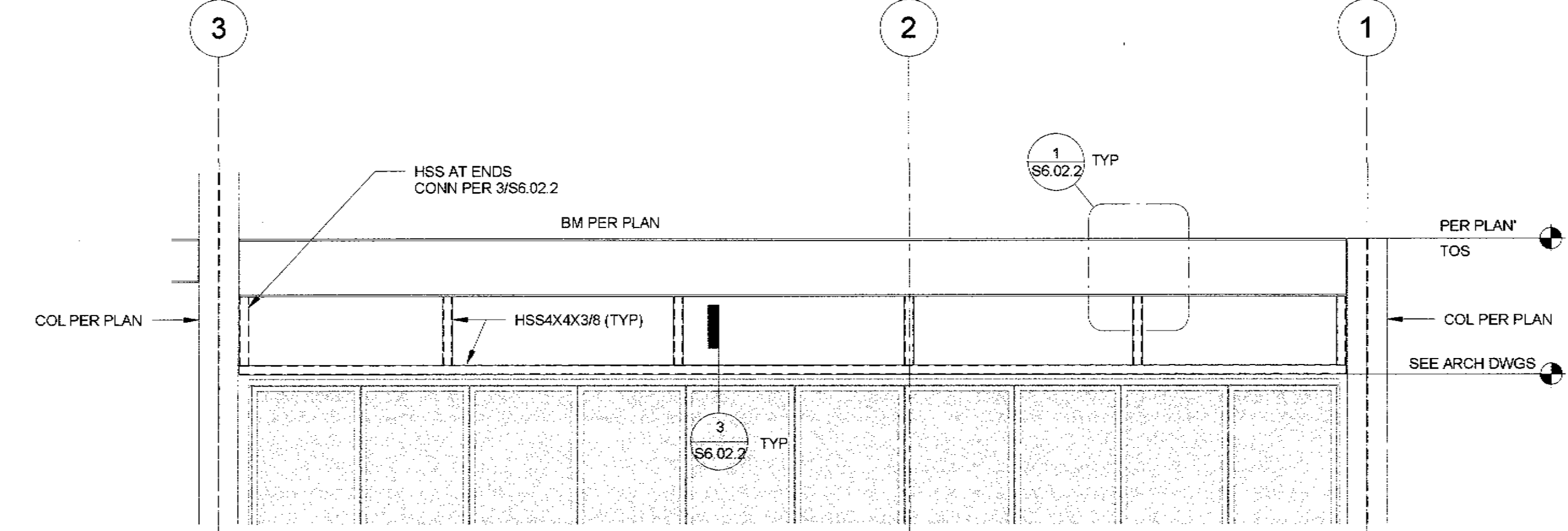
11 11-CONC WALL ELEV
1/4" = 1'-0"



12 SECURITY GRILL SUPPORT ALONG GRID 3
1/4" = 1'-0"



7 PARTITION SUPPORT ALONG GRID 3
1/4" = 1'-0"



8 PARTITION SUPPORT ALONG GRID C
1/4" = 1'-0"